

## Inspection Report with SI&A Data

**Structure Description:** 121.06 Foot - Single Span Steel Truss - Thru

**2 District:** 09      **3 County:** Mason      **16 Latitude:** 38°31'50.00"      **7 Longitude:** 83°38'51.00"

**7 Facility Carried:** DAVIS LN

**Milepoint:** 0.020

**6A Feature Intersected:** N.FK.LICKING RVR

**9 Location:** NE @ JCT CR 5120

NBI	X
Element	
Fracture Critical	
Underwater	
Special	X

NBI CONDITION RATINGS	
<b>58 Deck:</b> 6	<b>61 Channel:</b> 4
<b>59 Superstructure:</b> 4	<b>62 Culvert:</b> N
<b>60 Substructure:</b> 6	<b>Sufficiency Rating:</b> 22.5

GEOMETRIC DATA	
<b>48 Max Length Span:</b>	118.110 ft
<b>49 Structure Length:</b>	121.063 ft
<b>32 Approach Roadway:</b>	9.843 ft
<b>33 Median:</b>	(0) No Median
<b>34 Skew:</b>	0°
<b>35 Flare:</b>	No Flare
<b>50A Curb/Sidewalk Width L:</b>	0.000 ft
<b>50B Curb/Sidewalk Width R:</b>	0.000 ft
<b>47 Horiz. Clearance:</b>	13.123 ft
<b>51 Width Curb to Curb:</b>	13.123 ft
<b>52 Width Out to Out:</b>	13.451 ft

DESIGN	
<b>Substandard:</b>	Weight
<b>43A Main Span Material:</b>	(3) Steel
<b>43B Main Span Design:</b>	(10) Truss-Thru
<b>45 Number of Spans Main:</b>	1
<b>44A Approach Span Material:</b>	Not Applicable (0)
<b>44B Approach Span Design:</b>	Not Applicable (00)
<b>46 Number of Approach Spans:</b>	0
<b>107 Deck Type:</b>	(1) Concrete-Cast-in-Place
<b>108A Wearing Surface:</b>	(1) Monolithic Concrete
<b>108B Membrane:</b>	(0) None
<b>108C Deck Protection:</b>	(0) None
<b>Overlay Y/N:</b>	No
<b>Overlay Type:</b>	None
<b>Overlay Thickness:</b>	in
<b>Overlay Date:</b>	

ADMINISTRATIVE	
<b>27 Year Built:</b>	1918
<b>106 Year Reconstructed:</b>	0
<b>42A Type of Service On:</b>	(1) Highway
<b>42B Type of Service Under:</b>	(5) Waterway
<b>37 Historical Significance:</b>	(3) Possibly Eligible for NRHP
<b>21 Maintenance Responsibility:</b>	(02) County Hwy Agency
<b>22 Owner:</b>	(02) County Hwy Agency
<b>101 Parallel Structure:</b>	(N) No II Structure Exists

APPRAISAL	
<b>36A Bridge Railings:</b>	(0) Substandard
<b>36B Transitions</b>	(0) Substandard
<b>36C Approach Guardrail:</b>	(0) Substandard
<b>36D Approach Guardrail Ends:</b>	(0) Substandard
<b>71 Waterway Adequacy:</b>	(8) Equal Desirable
<b>72 Approach Alignment:</b>	(4) Minimum Tolerable
<b>92A Fracture Critical Inspection:</b>	Yes
<b>92B Under Water Inspection:</b>	No
<b>113 Scour Critical:</b>	(8) Stable above footing
<b>Recommended Scour Critical:</b>	(2) SC- Extensive Scour

CLEARANCES	
<b>10 Vert. Clearance:</b>	15.322 ft
<b>53 Min. Vert. Clearance Over:</b>	15.322 ft
<b>54A Vert. Under Reference:</b>	(N) Feature not hwy or RR
<b>54B Min. Vert. Underclearance:</b>	0.000 ft
<b>55A Lateral Under Reference:</b>	(N) Feature not hwy or RR
<b>55B Min. Lat. Underclearance R:</b>	0.000 ft
<b>56 Min. Lat. Underclearance L:</b>	0.000 ft

LOAD RATINGS	
<b>63 Operating Type:</b>	(1) Load Factor (LF)
<b>64 Operating Rating:</b>	9.6 tons
<b>65 Inventory Type:</b>	(1) Load Factor (LF)
<b>66 Inventory Rating:</b>	5.8 tons
<b>Truck Capacity Type I:</b>	4 tons
<b>Truck Capacity Type II:</b>	4 tons
<b>Truck Capacity Type III:</b>	5 tons
<b>Truck Capacity Type IV:</b>	8 tons

POSTINGS	
<b>41 Posting Status:</b>	(P) Posted For Load
<b>Signs Posted Cardinal:</b>	Yes
<b>Signs Posted Non-Cardinal:</b>	Yes
<b>Field Postings Gross:</b>	4 tons
<b>Field Postings Type I:</b>	tons
<b>Field Postings Type II:</b>	tons
<b>Field Postings Type III:</b>	tons
<b>Field Postings Type IV:</b>	tons

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:									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
			%		%		%		%

STRUCTURE NOTES
<p>40.9                      6/17/2016 Controlling member is floorbeam at L2. Critical point is 1.5 (midspan) for all trucks. Gross post at 4 tons due to rating of superstructure. DGA</p>

INSPECTION NOTES
<p>This is a special NBI inspection to only verify that the proper posting signs have been put back in place where they were missing and to change item (41) to P posted for load. Both ends of the bridge are posted as recommended. Inspection by A. Greiner &amp; W. K. Shugars.</p>

WORK	
Action:	



View of the west 9 tons posting.



View of moderate impact damage to the downstream rail at the west abutment.



View of minor sized transverse cracking in the wearing surface near the west abutment.



The wearing surface is rough along the centerline of the bridge. This is in Amish country and the bridge sees alot of horseshoes.



View of a bend in the lower section of the upstream L1 vertical.



View of a minor sized full width transverse crack in the wearing surface between the L1 and L2 connections.



View of a minor sized full width transverse crack in the wearing surface between the L4 and L5 connections.



View of a more moderate sized (~1/16") full width transverse crack in the wearing surface near the L5 connection.



View of a more moderate sized (~1/16") full width transverse crack in the wearing surface near the L6 connection.



View of a minor sized full width transverse crack in the wearing surface between the L6 and L7 connections.



View of 2 minor (there is actually 3, one is not visible in photo) sized diagonal trending cracks in the wearing surface between the L7 and L8 connections



View of the east 9 tons posting.





View of the upstream east bearing.



View of ~ 3.25" measured along the forge in the interior eyebar at the upstream L2 connection. There is a retrofit at this location.



View of retrofit at upstream L2.



View of heavy vegetation growth at the east abutments downstream bearing.



View from the upstream east end of the bridge. Notice the heavy vegetation growth.



View of heavy vegetation growth at the upstream end of the east abutment.



Typical view of the east abutment. Notice the minor sized horizontal cracking in the concrete cap.



View of heavy vegetation growth at the downstream end of the east abutment.



View of heavy vegetation growth at the east end of the bridge.



Typical view of heavy corrosion at the upstream end of the floorbeam at L7.



View looking west.



View of heavy drift blocking the channel~ 75' upstream of the bridge.



View of the floorbeam at L2. Notice that the retrofitted floorbeam that was welded to the bottom flange of the original floorbeam has fallen off. Note the floorbeam at L1.



View of the retrofitted floorbeam that WAS welded to the bottom flange of the original floorbeam at L2. It has fallen off.



View of the floorbeam at L2. Notice that the retrofitted floorbeam that was welded to the bottom flange of the original floorbeam has fallen off. Note the floorbeam at L1.



View of the floorbeam near the upstream L2 connection. Notice that the retrofitted floorbeam that was welded to the bottom flange of the original floorbeam has fallen off. Note the heavy corrosion.





Typical view of moderate to heavy corrosion of the stringer ends at the west abutment.



Typical view of the upstream bearing at the west abutment.