FINAL REPORT

I-69 Strategic Planning Corridor Study

Overview of Existing Conditions

Julian M. Carroll Purchase Parkway and Interstate 24

Fulton to Eddyville, Kentucky

December 2011

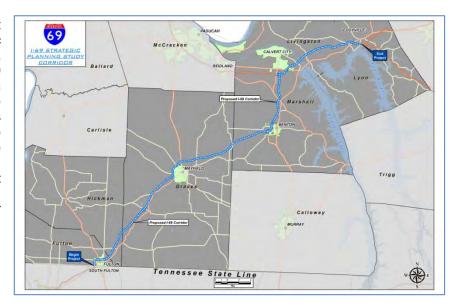




I-69: FULTON TO EDDYVILLE STRATEGIC CORRIDOR PLANNING STUDY EXECUTIVE SUMMARY

Kentucky Transportation Cabinet – Division of Planning July 2011

The Kentucky Transportation Cabinet (KYTC) has undertaken a strategic corridor planning study for a portion of a proposed interstate route. Interstate 69 (I-69), which is proposed to travel from Tennessee through Kentucky and into Indiana. The project corridor extends along the Julian M. Carroll Purchase Parkway north from the Tennessee state border to the I-24 interchange, and then travel east along I-24 to west of the Wendell H. Ford (Western Kentucky) Parkway. The corridor passes through Fulton, Hickman. Graves, Marshall, Livingston and Lyon Counties.



STUDY PURPOSE

The primary purposes of the strategic corridor study is to review the existing conditions along the Purchase Parkway and I-24 to identify locations that do not meet current AASHTO and Federal Highway Administration (FHWA) highway design guidelines and related criteria. Evaluations include the degree to which these criteria are not met, there impact on safety and capacity, identification of options for making improvements to address identified deficiencies, and make recommendations regarding suitability of routing I-69 along the Purchase Parkway and I-24.

PROJECT BACKGROUND

The federal Intermodal Surface Transportation Efficiency Act (ISTEA) of 1991 identified the I-69 (Corridor 18) as a Priority Corridor. The results from a 1995 FHWA *Corridor 18 Feasibility Study* concluded that the future construction of I-69 from Canada to Mexico was economically feasible. The *Corridor 18 Special Issues Study* completed in 1997 identified a Representative Corridor which best served the purposes of Corridor 18 and yielded the most benefits relative to facility costs. The initial national goals of I-69 included the enhanced movement of goods, creating greater employment opportunities and improved system linkage. In Kentucky these national goals are consistent with the regional and local goals of providing improved mobility and serving local connectivity needs. Utilizing the existing Parkway system for I-69 also is consistent with the national and local goals.

STUDY ACTIVITIES

The study activities for the I-69 Strategic Corridor Planning Study included the following:

- Identify criteria and standards per AASHTO and the FHWA for designation as an interstate route;
- Collect data from the KYTC's Highway Information System, as-built plans, crash data, field observation and measurement, and other information provided by local Highway District office:
- Compare and analyze data collected with criteria and identify conditions and locations on the Purchase Parkway that do not meet interstate criteria and standards;

• Develop potential alternatives and costs associated with improving these areas with identified deficiencies to meet criteria and standards for designation as an interstate highway.

KEY FINDINGS

The Purchase Parkway operates similar to an interstate. With exception of one location on the Mayfield Bypass, it possess two travel lanes in each direction, a design speed of 70 mile-per-hour for rural conditions and 50 mile-per hour for urban conditions, and is a fully controlled access facility. However, some of the physical features do not meet the criteria of an interstate facility. Attached to the end of this summary are figures identifying deficiencies.

The following findings are based on available data and limited field reviews.

Operational Considerations and Safety

- <u>Crash Analysis:</u> For the crash analysis, a high crash segment was defined as having a critical crash rate factor greater than or equal to one. Crash segments with a critical crash rate factor between 0.9 and 0.99 are identified in the report.
- <u>Crash Analysis Purchase Parkway:</u> When compared to other Kentucky parkways, there is one high crash segment in Graves County (MP 25.1 MP 27.452) where the crash rate exceeds the statewide average for all parkways. There also is one segment in Graves and Marshall Counties (MP 27.452 MP 41.035) with a critical crash rate factor between 0.9 and 0.99.
- <u>Crash Analysis I-24:</u> When compared to other interstates within Kentucky, there is one high crash segment located near the Purchase Parkway interchange in Marshall County (MP 24.941-MP 26.558) where the crash rate exceeds the statewide average for all interstates.
- <u>Crash Analysis Purchase Parkway as an Interstate:</u> When compared to Kentucky interstates, rather than state parkways, two additional high crash segments were identified along the Purchase Parkway located in Graves and Marshall Counties (MP 27.452 MP 41.035 and MP 42.555 MP 46.942).
- <u>Crash Segment Purchase Parkway as an Interstate:</u> There also are three segments with a critical crash rate factor between 0.9 and 0.99. These segments are: MP 24.747 MP 25.1, MP 41.035 MP 42.555, and MP 46.942 MP 51.398.
- Additional Findings Related to Crash Analysis: There were six crashes coded as median crossover or head-on collisions for the Purchase Parkway and I-24 during the study period (2005-2009). Two crashes occurred on the Purchase Parkway and the remaining four happened on I-24. There were seven fatal crashes on the Purchase Parkway and six fatal crashes on I-24 during the study period (2005-2009).
- <u>Current Traffic (2010):</u> The current Purchase Parkway traffic volumes range from 7,060 vehicles per day (vpd) in Fulton County to 19,200 vpd near I-24 interchange in Marshall County. The current I-24 traffic volumes range from 21,900 vpd near the Purchase Parkway interchange to 28,200 vpd near Calvert City in Marshall County.
- <u>Truck Percentages (2010):</u> The existing truck percentages on the Purchase Parkway range from 24.9% at Mayfield, Kentucky in Graves County to 34.5% near Benton, Kentucky in Marshall County. On I-24, the truck percentage is 24.9%.
- <u>Future Traffic (2040) without I-69:</u> The projected annual growth rate along the Purchase Parkway and I-24 is 2%. This rate results in traffic volumes ranging from 12,800 vpd to 34,800 vpd on the Purchase Parkway and from 39,700 vpd to 51,100 vpd on I-24.
- <u>Future Traffic (2040) with I-69:</u> Assuming I-69 will travel along the Purchase Parkway and I-24, an annual growth rate of 2.5% was used to forecast 2040 traffic volumes. This rate results in traffic volumes ranging from 14,800 vpd to 40,300 vpd on the Purchase Parkway and from 45,900 vpd to 53,900 vpd on I-24.
- <u>Truck Percentages (2040):</u> Future truck volumes were not forecasted for this project. However, truck traffic is expected to increase if the national goals of I-69 are met.
- <u>Level of Service (2010)</u>: All evaluated segments of I-24 and Purchase Parkway operate at LOS C or better in the current year.
- <u>Level of Service (2040):</u> All segments of I-24 and Purchase Parkway in the study area are expected to operate at LOS C or better in the future year 2040.

Mainline Geometry/Typical Section

- <u>Design Speed:</u> The Purchase Parkway meets or exceeds the minimum design speed guidelines for interstate highways in rural and urban areas.
- <u>Lane Width:</u> The lane width on the Purchase Parkway meets the minimum AASHTO guidelines for interstate design.
- <u>Outside Shoulder Width:</u> The Purchase Parkway meets minimum criteria for outside shoulder width based on the current truck DDHV.
- Inside Shoulder Width: The Purchase Parkway does not comply with the minimum design guidelines for inside paved shoulder widths. The section of Purchase Parkway at Mayfield, KY, also referred to as the Mayfield Bypass, has a raised median and no inside shoulder (MP 21.887 MP 24.901). The remainder of the Purchase Parkway has a 3 foot paved inside shoulder, while the minimum criteria requires a 4 foot paved shoulder.
- Median Width: The Purchase Parkway meets the rural 36 foot AASHTO minimum median width in rural areas and the 10 foot AASHTO minimum median width in urban areas.
- <u>Clear Zones:</u> Based on the available data, it was not possible to fully evaluate the clear zone without detailed field study. The fill and cut slopes provided in the typical sections vary from 1V:2H to 1V:4H, the median ditch slope is 1V:4H, and the outside ditch slope is between 1V:3H and 1V:4H. Inference can be made regarding available clear zone from review of the as-built plans. However, it can be assumed that those sections not already with guardrail installed meet clear zone requirements.
- <u>Sign Installations:</u> A field review of roadside signs showed all signs within the apparent clear zone were crash worthy (break away).
- Guardrail Placement and Condition: As-built plans do not provide sufficient information to
 evaluate the placement of guardrail (length of need) along the I-69 corridor. However, a field
 review of the corridor showed that the guardrail end treatments on the Purchase Parkway meet
 current criteria and standards.
- <u>Superelevation:</u> From the review of as-built plans, horizontal curves along the Purchase Parkway appear to comply with the AASHTO criteria of 10% maximum superelevation.
- <u>Horizontal Alignment:</u> Horizontal curvature for the Purchase Parkway meets the minimum criteria of current design criteria and guidelines.
- Vertical Alignment: The majority of the vertical curves along the Purchase Parkway meet the current criteria and guidelines. Eight vertical curves do not meet the guideline for the minimum length of vertical curves.
- <u>Stopping Sight Distance:</u> The minimum stopping sight distance guideline is not met for three vertical curves: MP 14.965, MP 18.727, and MP 25.320

Bridges and Overpasses

- <u>Lateral Clearance Purchase Parkway:</u> Of the 46 mainline bridges on the Purchase Parkway, 10 fail to meet the minimum lateral clearance requirement.
- <u>Vertical Clearance Purchase Parkway and I-24:</u> Of the 35 overpass bridges on the Purchase Parkway, 4 do not meet the minimum 16 foot vertical clearance requirement. The five overpass bridges on I-24 meet the minimum vertical clearance regulation.
- Functional Adequacy: One bridge (MP 21.285) is identified as functionally obsolete.
- <u>Sufficiency Rating:</u> All Purchase Parkway mainline and overpass bridges have a sufficiency rating greater than 60.0.

Interchanges and Ramps (Purchase Parkway)

- <u>Design Speed:</u> Design speed for ramps were not provided on the as-built plans and were not evaluated.
- <u>Lane Width:</u> Ramp lane widths range from 15 feet to 18 feet, which is greater than the 15 foot minimum width per current criteria for lane width.
- <u>Shoulder Width:</u> A majority of the interchange ramps on the Purchase Parkway do not meet the AASHTO guidelines for shoulder width. 10 of the 13 interchanges have ramp shoulder widths that do not meet criteria.
- <u>Horizontal Alignment:</u> With the exception of one loop ramp (Exit 14), all horizontal curvature at interchanges meet minimum criteria and requirements. The loop ramp has a 130 foot radius which does not meet the minimum loop ramp radius of 134 feet for a 25 mph design speed.

- <u>Vertical Alignment-Vertical Grade:</u> The minimum vertical grade is met on all interchange ramps that were provided on the as-built plans.
- <u>Vertical Alignment-Vertical Length of Curve:</u> Three vertical curves on ramps did not meet the requirements for minimum length of curve that were calculated based on the ramp design speed. These ramps are located at the US 51 interchange (Exit 1) and KY 80 interchange (Exit 22).
- <u>Vertical Alignment-Stopping Sight Distance:</u> Two vertical curves on ramps did not meet the minimum stopping sight distance requirement that were calculated based on the ramp design speed. These ramps are located at the US 51 interchange (Exit 1) and KY 80 interchange (Exit 22).
- <u>Superelevation:</u> Based on review of as-built plans, existing ramps appear to satisfy the AASHTO criteria for 10% maximum superelevation.
- <u>Speed-Change Lanes:</u> Many of the existing ramps on the Purchase Parkway do not meet the minimum criteria for acceleration and deceleration lengths.
- <u>Weaving Characteristics:</u> The one location with an existing weaving situation between interchanges will operate at a LOS B with future I-69 traffic projections. The interchanges at Exits 14 and 43 are previous toll plaza interchanges. Exit 52 is a cloverleaf interchange with weaving within the interchanges.
- <u>Interchange Spacing:</u> On the Purchase Parkway, there are two locations where the minimum interchange spacing requirements are not met. Interchange spacing was measured from intersecting routes along the Purchase Parkway. The three interchanges (Exits 0, 1, 2) in Fulton are within three miles of each other. The two interchanges (Exit 41 and Exit 43) in Benton are within three miles of each other.
- <u>Interchange Control of Access:</u> The Purchase Parkway has four interchanges that do not meet the recommended criteria for control of access.
- <u>Interchange Configuration:</u> Currently, the Purchase Parkway has four service interchanges that do not meet the recommended interstate interchange configuration. They are located at Exit 0, Exit 14, Exit 21, and Exit 43. The interchange configuration at I-24 and the Purchase Parkway is not recommended for a systems interchange.

POTENTIAL IMPROVEMENT ALTERNATIVES

For this study, the range of alternatives under consideration is No Build, Necessary Upgrades and Spot Safety Improvements, and Fully Compliant Reconstruction. These alternatives represent incremental levels of infrastructure investment needed to implement I-69 along the Purchase Parkway from Tennessee to I-24.

- No Build This alternate would leave a gap in the nationally proposed I-69 route. However, the Purchase Parkway would provide the connectivity for the I-69 traffic to travel from Tennessee to I-24
- Necessary Upgrades and Spot Safety Improvements Key safety and operational concerns
 would be addressed. Design exceptions or variances would be obtained for the existing
 conditions that do not meet current AASHTO or KYTC guidelines that are deemed appropriate by
 the KYTC and the FHWA.
- Fully Compliant Reconstruction This alternate would involve improvements within existing right of way or with minimum right of way acquisition necessary for making the existing Purchase Parkway meet minimum AASHTO criteria for interstate routes.

The following table represents preliminary cost estimates for the potential improvement alternatives.

Alternative	Meet Current Standards	Impact on Environment	Cost (million)	Cost per Mile ¹ (million)
1. No Build	No	Least	\$0.00 2	\$0.00
2. Necessary Upgrades / Spot Safety Improvements	Yes ³	Minimal	\$131.95	\$2.57
3. Fully Compliant Reconstruction	Yes	More Significant	\$218.94 ⁴	\$4.26

Table 8-5 Cost Comparison of Potential Alternatives

RECOMMENDATIONS

It is recommended that the Necessary Upgrades and Spot Safety Improvements alternative be chosen for initial advancement based on the following:

- The Purchase Parkway adequately meets AASHTO guidelines for most design elements of an interstate. Of the design element deficiencies, others may be accepted as design exception/variance with agreement by the KYTC and the FHWA.
- Based on the operational and crash analysis included in this study, addressing those repairs identified for Needed Upgrades and Spot Safety Improvements will appropriately address any crash history concerns identified. The entire length of the Purchase Parkway meets the level of service required and only a few locations exhibit potential safety problems.

If the intention is to utilize the Purchase Parkway for future I-69 designation, it is recommended to develop a strategy for future improvements based on operational characteristics, safety, routine maintenance and Federal Highway Administration guidance. The strategy of improvements will insure an efficient and coordinated implementation of future projects and designation of I-69. Additional data and analysis are recommended for project development:

- Operational Considerations There may be roadway conditions not shown in crash data contributing to crash history. Additional analyses during preliminary engineering may provide additional insight which could refine the scope of needed improvements at a given location.
- Mainline Geometry and Typical Section Analyses for mainline geometry and typical section were evaluated using as-built plans supplemented with field reviews of existing conditions. Actual design features may require further verification with non-detailed field reviews of the roadway cross-section during preliminary engineering for implementing improvement strategies.
- Interchanges and Ramps Most of the interchange ramps are deficient and some design features were illegible on the as-built plans. Therefore, as interchanges are identified for improvement, geometric features (i.e. superelevation rate, horizontal and vertical alignments, design speed, etc.) should be further analyzed.

CONCLUSIONS

Based on the findings of this study, it can generally be concluded that the Purchase Parkway is currently providing motorists efficient and safe travel from US 51 in Tennessee to I-24 with operating conditions similar to an interstate. There would be minimal to no impact to the operating characteristics of the Purchase Parkway in the near future if it was designated as I-69 under the current conditions. The operation characteristics of the I-69 corridor would not be expected to be altered until more sections of I-69 are completed across the country especially in Tennessee and Indiana. As sections of I-69 are completed and thus provide continuity at a regional and national level, additional truck traffic volume will likely grow on the Purchase Parkway to the point that estimated truck traffic and congestion along the existing Purchase Parkway may eventually alter the operational characteristics.

¹ Cost per mile based on 51.4 miles of Purchase Parkway.

² Cost for routine maintenance is not depicted in alternatives.

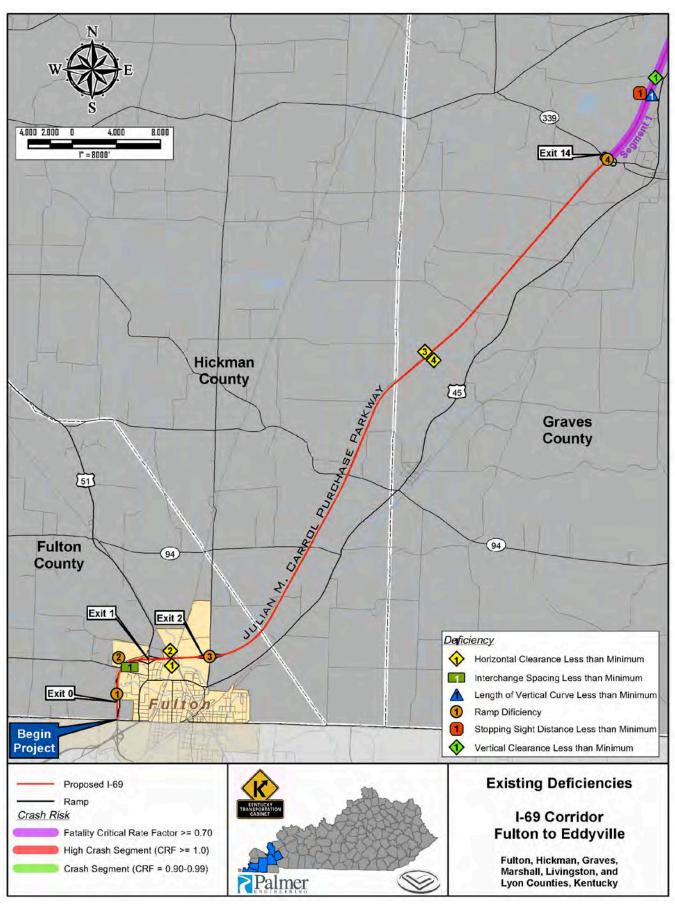
³ This alternative would include upgrading the design features along the Purchase Parkway that potentially represents the most significant safety and operational issues. This alternative requires design exceptions and variances where safety and operational conditions would not create undue risk to the motorist.

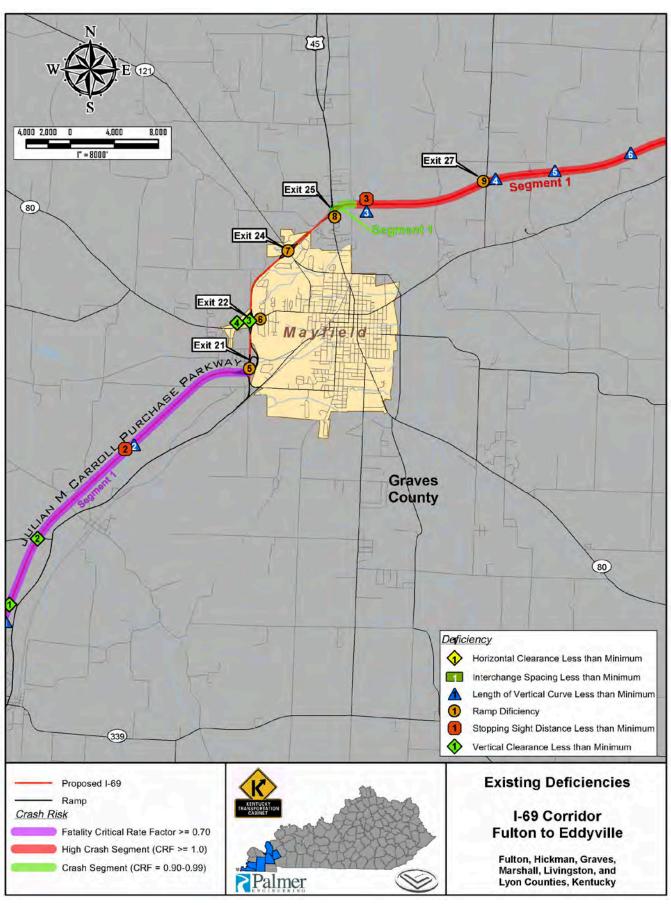
⁴ Cost estimate does not include cost associated with connecting to Segments of Independent Utility (SIU) 5 (I-24 at Western Kentucky Parkway) or SIU 7 (Exits 0,1,2 at Fulton, KY).

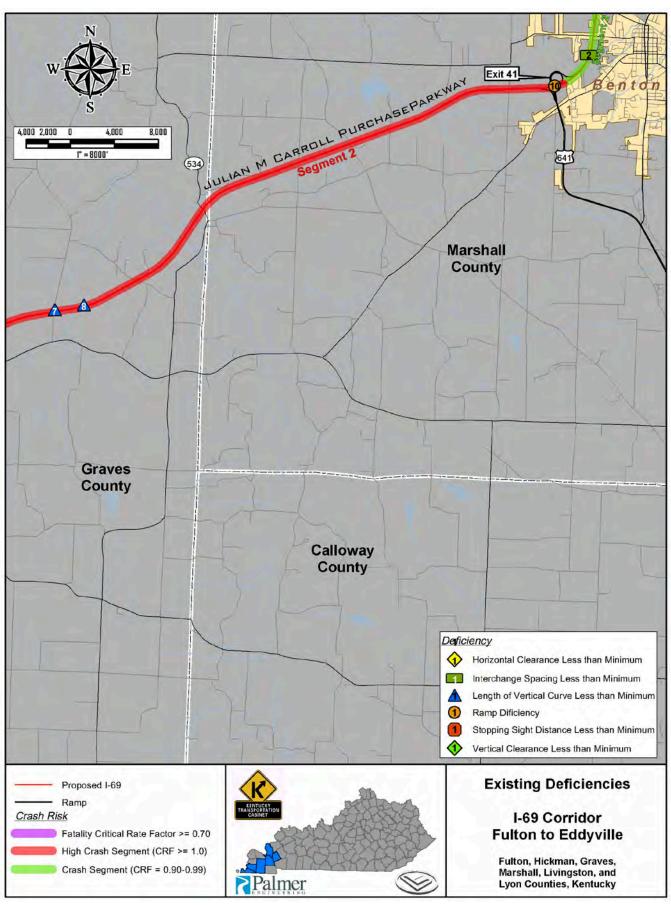
Intuitively, there may be sections of interstate in Kentucky and around the United States that do not meet the current design standards. Some design features on these other interstates may be very similar to the existing design features on the Purchase Parkway. Based on the impact to other sections of Parkways that are designated as future interstate corridors and existing interstates with similar design feature deficiencies, designation of the Purchase Parkway as I-69 under the Parkway's existing conditions appears realistic.

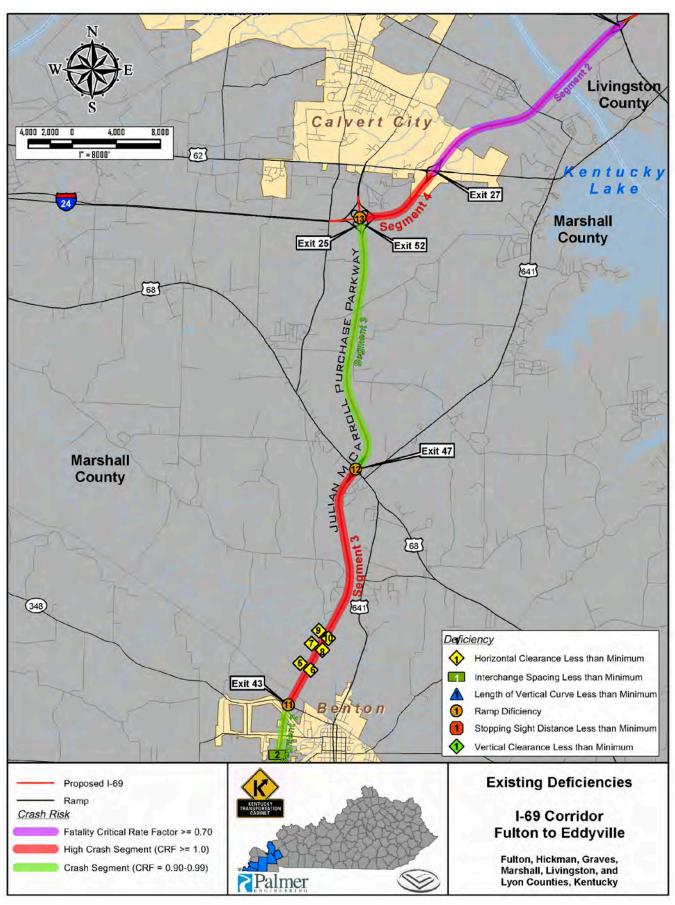
There are two broad based potential improvement alternatives recommended for improving the Purchase Parkway to meet interstate standards. The Necessary Upgrades and Spot Safety Improvement alternative includes upgrading the Purchase Parkway to meet current interstate standards but with design exceptions/variances. The Fully Compliant Reconstruction alternative would upgrade the Purchase Parkway to meet interstate standards with no design exceptions or variances. Right of way acquisitions will be needed for interchange improvements.

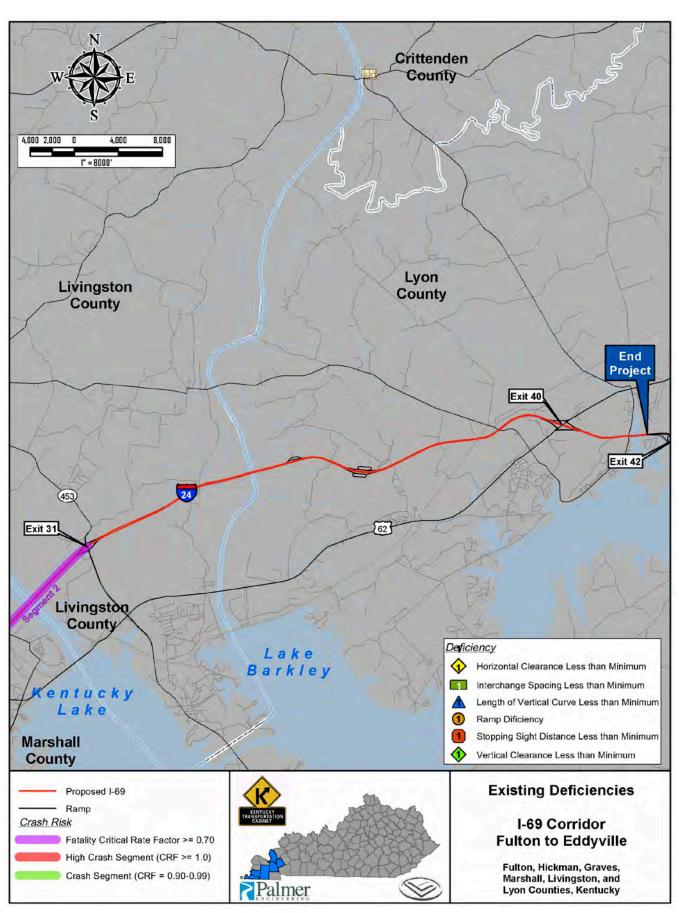
In general, improvements related to bridge deficiencies, Mayfield Bypass median, interchange acceleration and deceleration lanes, and previous toll plaza interchange improvements are recommended. It is also recommended that initially, minimal improvements should be made to the Purchase Parkway and I-24 interchange and US 45 interchange in Mayfield. The minimal improvements should be designed to provide continuity and capacity for the forecasted traffic, while maintaining consideration for crash history and safety for the traveling public. Ultimately, as traffic operations change and traffic volumes increase, additional improvements to these interchanges may be needed to improve safety and meet current interstate criteria.











Deficiency Type	Milepoint	Deficiency Description
Purchase Parkway	/ - Fulton/Hickma	n County
1	Exit 0	Taper Length < Min; Rolled Curb
1	MP 1.0	Interchange Spacing less than 3 mile minimum
2	Exit 1	Taper Length < Min; Rolled Curb
3	Exit 2	Taper Length < Min; Divergence Angle > Max; Rolled Curb
1	1.781	Horizontal Clearance = 30' (Note bridge is over 200' long)
2	1.781	Horizontal Clearance = 30' (Note bridge is over 200' long)
Purchase Parkway	y - Graves County	
3	9.082	Horizontal Clearance = 30' (Note bridge is over 200' long)
4	9.082	Horizontal Clearance = 30' (Note bridge is over 200' long)
1	13.645 - 21.305	Fatality CRF = 0.75 (CRF >=0.70)
4	Exit 14 MP 13.653	Taper Length < Min; Degree of Curve > Max; Ramp Entrance/Exit Deficient; Rolled Curb; Interchange control of access less than 300' minimum
1	14.965	Length of Vertical Curve = 500' (696' calcuated minimum)
1	14.965	Stopping Sight Distance = 554' (730' minimum)
1	15.302	Vertical clearance = 15.88' (16' minimum)
2	16.526	Vertical clearance = 15.94' (16' minimum)
2	18.727	Length of Vertical = 600' (624' calculated minimum)
2	18.727	Stopping Sight Distance = 727' (730' minimum)
5	Exit 21 MP 21.285	Taper Length < Min;; Divergence Angle > Max; Rolled Curb
6	Exit 22 MP 22.267	Taper Length < Min; Interchange control of access less than 100' minimum
3	22.267	Vertical clearance = 15.30' (16' minimum)
4	22.267	Vertical clearance = 15.12' (16' minimum)
7	Exit 24 MP 23.701	Taper Length < Min
8	Exit 25 MP 24.726	Taper Length < Min; Rolled Curb

Deficiency Type	Milepoint	Deficiency Description
1	24.747 - 25.100	Crash Segment CRF = 0.9 (CRF 0.90-0.99)
	25.100 - 27.452	High Crash Segment - CRF= 1.33 (CRF >=1.0)
	27.452 - 34.487	High Crash Segment - CRF = 1.05 (CRF >=1.0)
3	25.32	Length of Vertical Curve = 536' (584' calcuated minimum)
3	25.32	Stopping Sight Distance = 721' (730' minimum)
9	Exit 27 MP 27.461	Taper Length < Min; Ramp Entrance/Exit Deficient; Divergence Angle > Max; Rolled Curb; Interchange control of access less than 300' minimum
4	27.517	Length of Vertical Curve = 536' (584' calculated minimum)
5	28.625	Length of Vertical Curve = 400' (438' calculated minimum)
6	29.970	Length of Vertical Curve = 400' (416' calculated minimum)
7	31.144	Length of Vertical Curve = 400' (467' calcuated minimum)
8	31.646	Length of Vertical Curve = 600' (608' calculated minimum)
Purchase Parkway	y - Marshall Count	у
2	34.487 - 41.035	High Crash Segment - CRF = 1.05 (CRF >=1.0)
10	Exit 41 MP 40.809	Taper Length < Min; Divergence Angle > Max
2	MP 41.682	Interchange spacing less than 3 mile minimum
2	41.035 - 42.555	Crash Segment -CRF = 0.99 (CRF 0.90-0.99)
3	42.555 - 46.942	High Crash Segment CRF =1.0 (CRF >=1.0)
11	Exit 43 MP 42.555	Taper Length < Min; Degree of Curve > Max; Ramp Entrance/Exit Deficient; Rolled Curb
5	43.277	Horizontal Clearance = 30' (Note bridge is over 200' long)
6	43.277	Horizontal Clearance = 30' (Note bridge is over 200' long)
7	43.614	Horizontal Clearance =30' (Note bridge is over 200' long)
8	43.614	Horizontal Clearance =30' (Note bridge is over 200' long)
9	43.872	Horizontal Clearance =30' (Note bridge is over 200' long)
10	43.872	Horizontal Clearance = 30' (Note bridge is over 200' long)
12	Exit 47 MP 46.942	Taper Length < Min; Rolled Curb; Interchange control of access less than 300' minimum
3	46.942 - 51.398	Crash Segment - CRF = 0.91 (CRF 0.90-0.99)
13	Exit 52 MP 51.398	Taper Length < Min; Degree of Curve > Max

Deficiency Type	Milpoint	Deficiency Description										
Interstate 24 - Ma	nterstate 24 - Marshall County											
4	24.941 - 26.558	High Crash Segment - CRF =1.10 (CRF >=1.0)										
2	26.558 - 29.352	Fatality CRF = 0.71 (CRF >=0.70)										
Interstate 24 - Liv	ringston/Lyon Co	unty										
2	29.352 - 30.742	Fatality CRF = 0.71 (CRF >=0.70)										

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I. PROJECT INTRODUCTION

The Kentucky Transportation Cabinet (KYTC) has undertaken a strategic corridor planning study for a portion of a proposed interstate route, Interstate 69 (I-69), which is proposed to travel from Indiana through Kentucky and into Tennessee. As shown in **Figure 1-1** the project corridor travels the Purchase Parkway north from the Tennessee state border to the I-24 interchange, and then travels east along I-24 to west of the Wendell H. Ford (Western Kentucky) Parkway. The corridor travels through Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties.

Project Purpose and Need

The primary purposes of the strategic corridor study are to review the existing conditions along the Purchase Parkway and I-24 to identify locations that may not meet American Association of State Highway Transportation Officials (AASHTO) highway design guidelines, evaluate the degree to which these guidelines are not met, identify options for making improvements to address identified deficiencies, and make recommendations regarding suitability of routing I-69 along the Purchase Parkway and I-24.

This planning-level analysis utilized As-built construction plans provided by KYTC, the KYTC Highway Information System (HIS) database, and field reviews to collect roadway geometry and highway operations. National I-69 studies undertaken for the Federal Highway Administration (FHWA) were also reviewed for information on a national level.

This study addresses the need and justification of upgrades to the Purchase Parkway to achieve interstate highway design guidelines. The study includes an Environmental Overview and an Environmental Justice Review (Appendix A) to evaluate associated environmental factors, social/economic conditions of the project area. Comments and suggestions from a local/elected officials meeting are included in Appendix B. Appendix C includes minutes and material from a public meeting held November 15, 2005. A Geotechnical Overview of the Purchase Parkway was conducted to summarize the existing geotechnical conditions along the project corridor and is included in Appendix D.

A. Background of I-69 Corridor

The federal Intermodal Surface Transportation Efficiency Act (ISTEA) of 1991 identified the I-69 (Corridor 18) as a Priority Corridor. The results from a 1995 Federal Highway Administration *Corridor 18 Feasibility Study* concluded that the future construction of I-69 from Canada to Mexico was economically feasible. The I-69 corridor begins at Port Huron, Michigan, and the Canadian border; passes through Michigan, Indiana, Kentucky, Tennessee, Mississippi, Arkansas, Louisiana, and Texas; and terminates at the Lower Rio Grande Valley and Mexican border.

The *Corridor 18 Special Issues Study* completed in 1997 identified a Representative Corridor which best serves the purposes of Corridor 18 and yields the most benefits relative to facility costs. This study also identified Segments of Independent Utility (SIU) that would allow completion of the I-69 corridor in segments that could function independently on a reasonable basis. In Kentucky, the Representative SIU segments were defined as follows:

- SIU 4
 - I-64/I-164 north of Evansville to the Edward T. Breathitt (Pennyrile) Parkway at Henderson, Kentucky;
- SIU 5
 - The Edward T. Breathitt (Pennyrile) Parkway from Henderson, Kentucky to the interchange with Wendell H. Ford (Western Kentucky) Parkway;
 - The Ford Parkway to the interchange with I-24;
- SIU 6
- I-24 at Ford Parkway to the interchange with the Julian M. Carroll (Purchase) Parkway;
- o The Purchase Parkway to the Tennessee state line.



Figure 1-1Study Area, Fulton to Eddyville, KY

The initial national goals for this project included enhancing the movement of goods, creating greater employment opportunities, and improving system linkage. Because of these goals one of the primary justifications for the I-69 route is increased freight transport along the corridor between Canada and Mexico. The I-69 corridor has been designated by Congress as a "North America trade route".

In Kentucky these national goals are consistent with the regional and local goals of providing improved mobility and serving local connectivity needs. Utilizing the existing Parkway system for I-69 meets the national and local goals.

SIU 4 in Kentucky crosses the Ohio River at Henderson connecting SIU 5 in Kentucky and SIU 3 in Indiana.

A strategic corridor planning study for SIU 5 has been completed and KYTC is currently developing strategies for implementation. With completion of this study, recommendations for needed improvements in SIU 6 will be developed and presented.

B. Highway Segments - SIU 6

The segments of SIU 6 include the Purchase Parkway and I-24. Since I-24 is currently in the interstate system, analysis for this study is of a cursory nature. Thus I-24 was only evaluated in bridge vertical clearances, traffic operations, and crash history. A more thorough evaluation of the Purchase Parkway was conducted and compared to the current interstate standards.

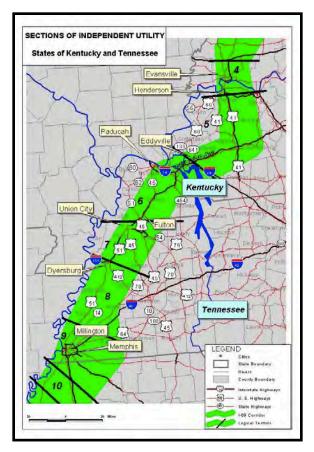


Figure 1-2 Interstate 69 Representative Corridor Sections of Independent Utility

A more descriptive summary of the I-69 corridor along the Purchase Parkway and I-24 follows:

The Purchase Parkway begins at the city of Fulton at the Tennessee state line (MP 0.0) and runs north through the cities of Mayfield and Benton before intersecting with I-24 (MP 51.398) near Calvert City.

The segment of I-24 within the I-69 corridor runs from the interchange with the Purchase Parkway (MP 24.941) to just west of the interchange with the Wendell H. Ford Parkway (MP 41.25).

The following chart summary illustrates the I-69 corridor by county.

ROUTE	COUNTY	BEGIN MP	END MP	TOTAL MILEAGE
	Fulton	0	3.43	3.43
PURCHASE	Hickman	3.43	8.35	4.92
PARKWAY	Graves	8.35	34.49	26.14
	Marshall	34.49	51.40	16.91
			Total	51.40
	Marshall	24.94	29.35	4.41
I-24	Livingston	29.35	33.88	4.53
	Lyon	33.88	41.25	7.37
			Total	16.31
	TOTAL PRO	JECT		67.71

Table 1-1 I-69 Corridor Mileage

C. Analysis Considerations

The Purchase Parkway was evaluated based on the current KYTC and FHWA design standards and guidelines. Applicable references are listed below:

- "A Policy on Geometric Design of Highways and Streets, 4" Edition" (American Association of State Highway Officials, Current Edition)
- "AASHTO Roadside Design Guide" (American Association of State Highway Officials, Current Edition)
- "Highway Capacity Manual" (Transportation Research Board, Current Edition)
- "Manual of Uniform Traffic Control Devices, Millennium Edition" (Institute of Transportation Engineers, Current Edition)
- "A Policy on Design Standards Interstate System" (American Association of State Highway Officials, Current Edition).
- Kentucky Transportation Cabinet Highway Design Manual (KYTC, Current Edition)

The existing conditions of the Purchase Parkway were established by utilizing as-built plans provided by KYTC, the HIS database, and CRASH database. This information was analyzed based on the reference list above to determine the extent to which it meets the current design guidelines. The analysis also includes determining whether the Purchase Parkway currently satisfies the safety and operational concerns that might be expected from converting the parkway into an interstate highway. The rural and urban sections of the Purchase Parkway were compared to interstate criteria. The rural sections of the Purchase Parkway traverses from Kentucky-Tennessee state line (MP 0.0) to MP 21.3 south of Exit 21 (US 45) and from north of Exit 25 (US 45) at MP 25.1 to the I-24 interchange (MP 51.4). The section of the Purchase Parkway that traverses through the city of Mayfield is commonly known as the Mayfield Bypass and was compared to the urban interstate criteria.

The following report is structured based on key factors in determining applicable design guideline compliance of the Purchase Parkway. A summary of these key factors are described below.

- Chapter 2: Early Coordination and Public Involvement
- Chapter 3: Operational Considerations An analysis of operational factors including crash history, traffic volumes, and operational levels of service for existing and future traffic conditions.
- Chapter 4: Mainline Geometry and Typical Section A discussion and evaluation of the existing corridor on the following topics: Mainline geometric issues, design speed,

- median widths, clear zones, horizontal and vertical alignments, superelevation rates, and sight distances.
- Chapter 5: Bridges and Overpasses An evaluation of the existing bridges and overpasses based on lateral and vertical clearance.
- Chapter 6: Interchanges and Ramps A summary of interchange and ramp conditions and a comparison of those conditions with AASHTO guidelines for design speed, typical sections, alignment geometry, speed-change lanes, and weaving situations.
- Chapter 7: Key Findings of Existing Conditions Overview An overview of the identified deficiencies in the project corridor.
- Chapter 8: Potential Improvement Alternatives and Development Costs An overview of a range of alternatives under consideration for development of the I-69 corridor.
- Chapter 9: Recommendations Recommendations based on the Strategic Corridor Planning Study and future analysis necessary to provide direction for design decisions for the corridor.

D. Design Exceptions and Variances

The FHWA has identified thirteen design features that are important to the operational and safety performance of a highway. These controlling design features compiled are commonly known as the 13 controlling criteria. A formal written design exception is required when any of the 13 criteria are not met on the National Highway System (NHS). The Interstate System is part of the NHS. The 13 controlling criteria are listed below. These design features are evaluated in this report and are evaluated for compliance. Design features that deviate from common practice but are not included in the 13 controlling criteria will be termed design variance. There are two categories for design variances. A design variance is a design feature that (1) varies from the current AASHTO criteria but not part of the 13 controlling criteria or (2) a design feature that varies from common practice but not part of the 13 controlling criteria.

- 1. Design speed
- 2. Lane width
- 3. Shoulder width
- 4. Bridge width
- 5. Horizontal alignment
- 6. Superelevation
- 7. Vertical alignment
- 8. Grade
- 9. Stopping sight distance
- 10. Cross slope
- 11. Vertical clearance
- 12. Lateral offset to obstruction
- 13. Structural capacity

II. EARLY COORDINATION AND PUBLIC INVOLVEMENT

As part of this study, a local officials and public involvement meeting was conducted in an effort to gain public input on issues, concerns, and to provide the public with information about this study and the potential I-69 corridor. These meetings took place at the Purchase Area Development District Office in Mayfield, Kentucky, on November 15, 2010.

At these meetings, preliminary information was collected and compiled for public display. A handout and questionnaire were given to the meeting attendees and six displays were presented. The information provided included the purpose and need for the project. Other information collected, compiled, analyzed, and presented in display format included the current traffic operations, existing conditions, and a crash history of the Parkway and I-24. In efforts to educate the public on the existing conditions, a powerpoint presentation was presented on previous studies along with the scope of work for this study. In the presentation and displays, key geometric features were identified as needing improvements in order to meet interstate standards and assign the corridor as I-69. Impacts to the traffic operations due to increased I-69 traffic volumes were also presented.

The questionnaire provided collected information regarding specific safety issues, traveler use, and suggested interchange improvements on the Parkway and I-24.

A question and answer session was held following the meetings where Kentucky Transportation Cabinet and design consultants were available to answer questions regarding the project.

A detailed summary of these meetings and questionnaire responses is provided in **Appendix B** and **C** of this report.

III. OPERATIONAL CONSIDERATIONS

The current and future operations of the Purchase Parkway, functioning as both a parkway and an interstate, should be evaluated for the proposed designation as I-69. The evaluation of the operational considerations includes a crash history and traffic analysis of the Purchase Parkway.

A. Crash History and Analysis

The objective of the crash history analysis was to identify locations of high crash rates and crash patterns on the Purchase Parkway and I-24. Further investigation of these high crash rate locations was conducted to establish causation or whether they occurred randomly.

1. Crash Analysis Methodology and Data Source

Transportation Kentucky Center's Analysis of Traffic Crash Data in Kentucky (2005-2009) was referenced for methodology, formulas, and factors to calculate crash rates for the Purchase Parkway and I-24. Segments of the project for the analyses were established based on changes in Daily Traffic (ADT), Annual roadway features and roadway classification. The crash rate was calculated within each segment based on length, ADT, type of roadway (parkway/interstate), functional classification (rural/urban), and crashes that occurred in the segment during



the crash history period. Crash data for the analyses was collected from the Collision Report Analysis for Safer Highways (CRASH) database from January 1, 2005 to December 21, 2009 within the project limits of the Purchase Parkway and I-24.

2. Types and Locations of Crashes

In order to calculate the crash rate, utilizing the referenced documentation, the parkway was divided into segments based on roadway geometry, roadway classification, and traffic volumes. The required inputs are functional classification (rural/urban), median type (divided/undivided), and changes in ADT volume.

The graph below shows the total number and type of crashes during the analysis time frame for the Purchase Parkway and I-24. For this analysis, crashes were classified as fatal, injury, or property-damage-only type. Of the total crashes on the Purchase Parkway, there were 7 fatal (1%), 136 injury (23%), and 449 property-damage-only (76%) crashes.

Number of Crashes by Type

(January 2005 - December 2009)



Source: Collision Report Analysis for Safer Highways (CRASH) database

3. Analysis as a Parkway Facility

The crash history data from the Purchase Parkway was analyzed as a parkway facility and as an interstate facility. The following discussion relates to the analysis of the Purchase Parkway as a parkway facility. The analysis of the Purchase Parkway functioning as an interstate is discussed in the following section.

For the analysis of segments, a high crash segment was defined as having a critical crash rate factor greater than or equal to 1.0. A fatality crash rate factor was calculated for the segments to identify segments with a history of fatal crashes. Segments with a fatal crash rate greater than 0.7 were identified in the analysis.

The Purchase Parkway was divided into 14 segments for the analyses. Crash rates were calculated for each segment. The statewide average crash rate for all parkways is 60 crashes per one-hundred million vehicle miles (acc/hmvm) for rural areas and 104 acc/hmvm in urban areas. Based on the calculation and data, the crash rates range from 6.8 to 199.26 acc/hmvm.

Reviewing **Table 3-1**, there is a high crash segment on the Purchase Parkway in Graves County (MP25.100 – MP 27.452) and a crash rate segment in Graves and Marshall Counties (MP 27.452 – MP 41.035) with a critical crash rate factor between 0.9 and 0.99.

Using the Kentucky Transportation Center's *Analysis of Traffic Crash Data in Kentucky (2005-2009)* methodology, an additional critical "spot" analysis was conducted on the high crash segment between MP 25.100 and MP 27.452 from 2005 to 2009. The methodology defines a critical "spot" as a 0.3 mile length of roadway with more than the critical number of crashes defined by roadway type and area type. This segment has three critical "spots" that meet the eight crashes for a rural parkway. Two of these spots overlap. These spots are as follows:

- 9 Crashes MP 25.189 MP 25.422
- 10 Crashes MP 26.200 MP 26.423
- 9 Crashes MP 26.300 MP 26.579

COLINITY	BEGIN	END	LENGTH	ADT	Lanes	Divided	Rural	Avg Crash	Critical	Avg	Critical		Crashes					Rates p	Critical Crash Rate Factor	Critical Fatality		
COUNTY	MP	MP	(miles) ADT Lattes Undivided Urban Rate Rate	Crash Rate	Fatality Rate	Fatality Rate	Fatal	Injury	PDO	Total	HMVM	Fatal	Injury	PDO	Total	Rate Factor						
Fulton	0.000	0.360	0.36	8,500	4	Divided	Rural	60	153.39	0.7	18.77	0	0	5	5	0.06	0.00	0.00	89.53	89.53	0.58	0.00
Fulton	0.360	1.424	1.064	7,570	4	Divided	Rural	60	115.45	0.7	9.72	0	0	1	1	0.15	0.00	0.00	6.80	6.80	0.06	0.00
Fulton	1.424	2.478	1.054	7,060	4	Divided	Rural	60	117.83	0.7	10.23	0	1	3	4	0.14	0.00	0.33	22.09	29.45	0.25	0.00
Fulton/ Hickman/ Graves/	2.478	13.645	11.167	7,290	4	Divided	Rural	60	76.71	0.7	2.80	1	19	43	63	1.49	0.67	0.44	28.94	42.40	0.55	0.24
Graves	13.645	21.305	7.66	8,590	4	Divided	Rural	60	78.63	0.7	3.08	3	11	50	64	1.20	2.50	0.22	41.64	53.30	0.68	0.81
Graves	21.305	22.239	0.934	14,300	4	Divided	Urban	104	159.26	0.6	6.69	0	4	16	20	0.24	0.00	0.25	65.64	82.05	0.52	0.00
Graves	22.239	23.701	1.462	13,100	4	Divided	Urban	104	149.87	0.6	5.41	0	6	15	21	0.35	0.00	0.40	42.92	60.08	0.40	0.00
Graves	23.701	24.747	1.046	12,000	4	Divided	Urban	104	161.07	0.6	6.95	0	0	6	6	0.23	0.00	0.00	26.19	26.19	0.16	0.00
Graves	24.747	25.100	0.353	7,790	4	Divided	Urban	104	231.23	0.6	19.47	0	2	8	10	0.05	0.00	0.25	159.41	199.26	0.86	0.00
Graves	25.100	27.452	2.352	7,790	4	Divided	Rural	60	96.00	0.7	5.92	1	8	29	38	0.33	2.99	0.28	86.73	113.64	1.18	0.50
Graves/ Marshall	27.452	41.035	13.583	7,320	4	Divided	Rural	60	75.09	0.7	2.58	1	34	91	126	1.81	0.55	0.37	50.15	69.44	0.92	0.21
Marshall	41.035	42.555	1.52	16,700	4	Divided	Rural	60	90.40	0.7	4.95	1	11	25	37	0.46	2.16	0.44	53.97	79.87	0.88	0.44
Marshall	42.555	46.942	4.387	18,800	4	Divided	Rural	60	76.60	0.7	2.79	0	21	81	102	1.51	0.00	0.26	53.81	67.77	0.88	0.00
Marshall	46.942	51.398	4.456	19,200	4	Divided	Rural	60	76.29	0.7	2.75	0	19	76	95	1.56	0.00	0.25	48.67	60.84	0.80	0.00

Table 3-1 Purchase Parkway Crash Analysis as a Parkway Facility

Source: Collision Report Analysis for Safer Highways (CRASH) database, and the Kentucky Transportation Center's Analysis of Traffic Accident Data in Kentucky (2005-2009)

Crash Rate Segment (CRF = 0.9-0.99)

High Crash Rate Segment (CRF >= 1.0)

Concerned Fatal Crash Segment

Legend

Abbreviations shown are defined as follows: MP – Milepoint; ADT – Average Daily Traffic (vehicles per day); PDO – Property Damage Only; HMVM – Hundred Million Vehicle Miles (vehicle miles per year divided by 100,000,000)

Crash Analysis Methodology

The Kentucky Transportation Center Analysis of Traffic Accident Data in Kentucky (2005-2009) was referenced for crash analysis methodology, formulas, and factors to calculate crash rates.

4. Analysis as an Interstate Facility

In Kentucky, the average crash rate for an interstate facility is lower than a parkway facility. The statewide average crash rate for an interstate facility for urban areas is 97 acc/hmvm and 52 acc/hmvm for rural areas. The lower average crash rate for an interstate facility versus a parkway facility indicates that per vehicle-mile of travel there are fewer crashes.

Table 3-2 illustrates that there are three crash rate segments on the Purchase Parkway with a critical crash rate factor between 0.9 and 0.99. One of these segments in Graves County is between MP 24.747 and 25.100. This segment is located between the US 45 interchange and the end of the typical urban section. The other two crash segments in Marshall County are located at MP 41.035 – MP 42.555 and MP 46.942 – MP 51.398. The first segment is located between US 641 and KY 348 interchanges in Benton, KY. The second segment is located north of the US 68 Interchange and south of the I-24 Interchange on the Purchase Parkway.

There are four segments that are defined as high crash rate segments. They are located in the following mile post ranges for the Purchase Parkway: MP 25.1 - MP 27.452, MP 27.452 - MP 41.035, and MP 42.555 - MP 46.942. On I-24 there is one segment in Marshall County west of the Purchase Parkway (MP 24.941 – MP 26.558) that is a high crash rate segment.

Of the high crash rate segments identified in the analysis as an interstate facility, the MP 24.941 – MP 26.558 segment on I-24 had a critical



'spot'. For a rural interstate section, a critical 'spot' is defined to have had 18 crashes occur within a 0.3 mile segment of roadway. Nineteen crashes occurred between MP 29.41 and MP 25.200. This 'spot' coincides with the Purchase Parkway Interchange.

Figures 3-1 through 3-5 and Table 3-2 illustrate the Purchase Parkway crash analysis as an interstate facility.

DOUTE	COUNTY	BEGIN	END	LENGTH	ADT	1	Divided Undivided	Rural	Avg Crash	Critical Crash	Avg Fatality	Critical		Cra	shes		HMVM		Rates		Critical Fatality		
ROUTE	COUNTY	MP	MP	(miles)	ADT	Lanes		Urban	Rate	Rate	Rate	Fatality Rate	Fatal	Injury	PDO	Total	ним	Fatal	Injury	PDO	Total	Rate Factor	Rate Factor
	Fulton	0.000	0.360	0.36	8,500	4	Divided	Rural	52	139.56	0.8	19.50	0	0	5	5	0.06	0.00	0.00	89.53	89.53	0.64	0.00
	Fulton	0.360	1.424	1.064	7,570	4	Divided	Rural	52	103.85	0.8	10.21	0	0	1	1	0.15	0.00	0.00	6.80	6.80	0.07	0.00
	Fulton	1.424	2.478	1.054	7,060	4	Divided	Rural	52	106.09	0.8	10.73	0	1	3	4	0.14	0.00	0.33	22.09	29.45	0.28	0.00
	Fulton/ Hickman/ Graves	2.478	13.645	11.167	7,290	4	Divided	Rural	52	67.58	0.8	3.03	1	19	43	63	1.49	0.67	0.44	28.94	42.40	0.63	0.22
	Graves	13.645	21.305	7.66	8,590	4	Divided	Rural	52	69.37	0.8	3.32	3	11	50	64	1.20	2.50	0.22	41.64	53.30	0.77	0.75
	Graves	21.305	22.239	0.934	14,300	4	Divided	Urban	97	150.44	0.5	6.24	0	4	16	20	0.24	0.00	0.25	65.64	82.05	0.55	0.00
PURCHASE	Graves	22.239	23.701	1.462	13,100	4	Divided	Urban	97	141.34	0.5	5.01	0	6	15	21	0.35	0.00	0.40	42.92	60.08	0.43	0.00
	Graves	23.701	24.747	1.046	12,000	4	Divided	Urban	97	152.19	0.5	6.49	0	0	6	6	0.23	0.00	0.00	26.19	26.19	0.17	0.00
	Graves	24.747	25.100	0.353	7,790	4	Divided	Urban	97	220.21	0.5	18.59	0	2	8	10	0.05	0.00	0.25	159.41	199.26	0.90	0.00
	Graves	25.100	27.452	2.352	7,790	4	Divided	Rural	52	85.62	0.8	6.28	1	8	29	38	0.33	2.99	0.28	86.73	113.64	1.33	0.48
	Graves/ Marshall	27.452	41.035	13.583	7,320	4	Divided	Rural	52	66.07	0.8	2.79	1	34	91	126	1.81	0.55	0.37	50.15	69.44	1.05	0.20
	Marshall	41.035	42.555	1.52	16,700	4	Divided	Rural	52	80.37	0.8	5.26	1	11	25	37	0.46	2.16	0.44	53.97	79.87	0.99	0.41
	Marshall	42.555	46.942	4.387	18,800	4	Divided	Rural	52	67.47	0.8	3.01	0	21	81	102	1.51	0.00	0.26	53.81	67.77	1.00	0.00
	Marshall	46.942	51.398	4.456	19,200	4	Divided	Rural	52	67.19	0.8	2.96	0	19	76	95	1.56	0.00	0.25	48.67	60.84	0.91	0.00
	Marshall	24.941	26.558	1.617	21,900	4	Divided	Rural	52	75.88	0.8	4.44	0	14	40	54	0.65	0.00	0.35	61.89	83.56	1.10	0.00
1-24	Marshall/ Livingston	26.558	30.742	4.184	28,200	4	Divided	Rural	52	64.89	0.8	2.60	4	24	59	87	2.15	1.86	0.41	27.40	40.40	0.62	0.71
1.24	Livingston/ Lyon	30.742	39.553	8.811	25,700	4	Divided	Rural	52	61.26	0.8	2.05	1	24	119	144	4.13	0.24	0.20	28.80	34.85	0.57	0.12
	Lyon	39.553	41.647	2.094	25,500	4	Divided	Rural	52	71.33	0.8	3.65	1	6	37	44	0.97	1.03	0.16	37.97	45.15	0.63	0.28

Table 3-2 Crash Analysis as an Interstate Facility

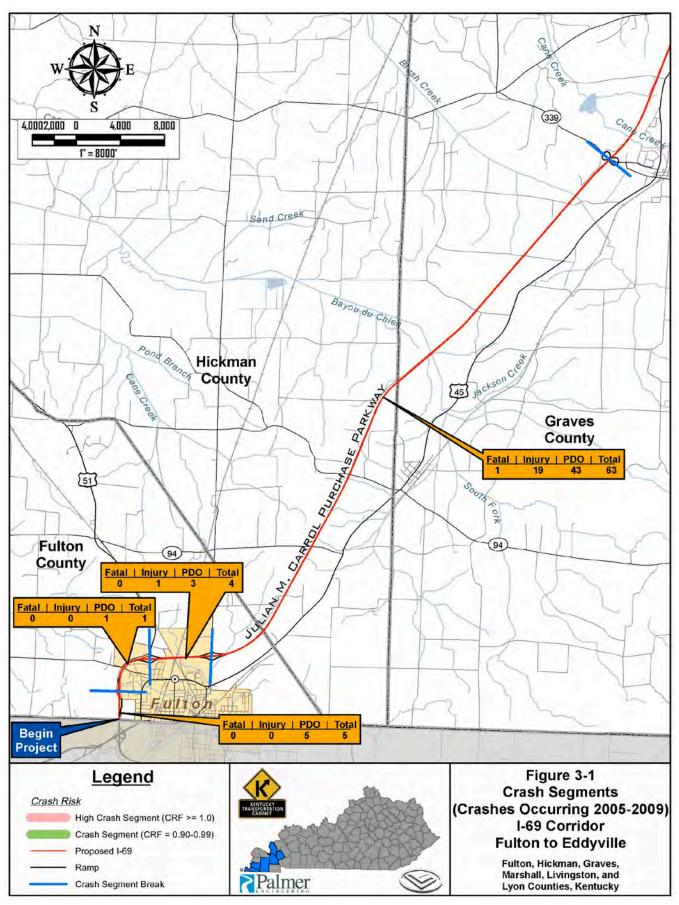
Crash Rate Segment (CRF = 0.9-0.99)
High Crash Rate Segment (CRF => 1.0)
Concerned Fatal Crash Segment

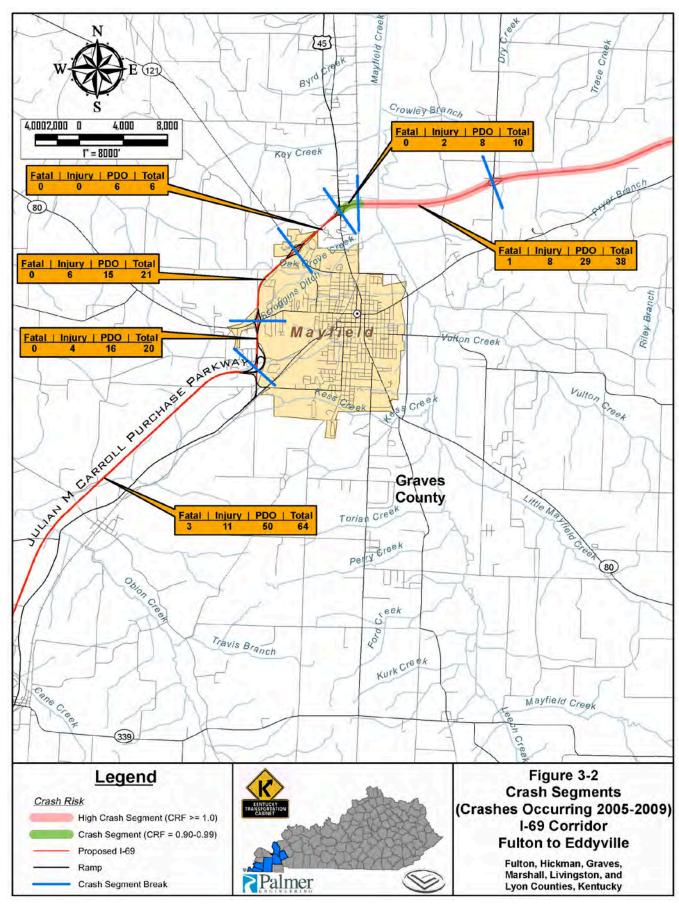
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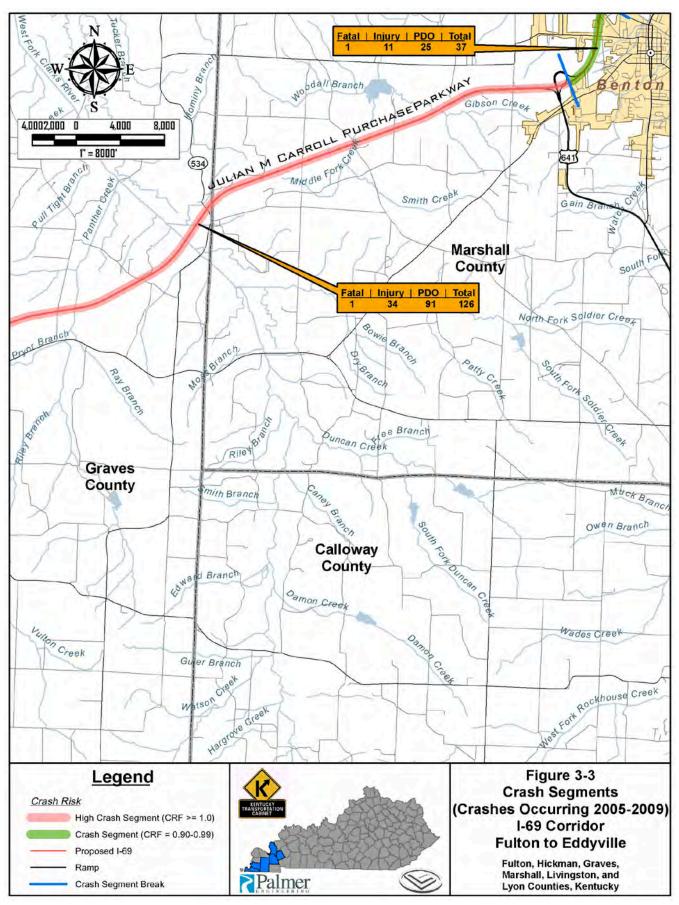
Abbreviations shown are defined as follows: MP – Milepoint; ADT – Average Daily Traffic (vehicles per day); PDO – Property Damage Only; HMVM – Hundred Million Vehicle Miles (vehicle miles per year divided by 100,000,000)

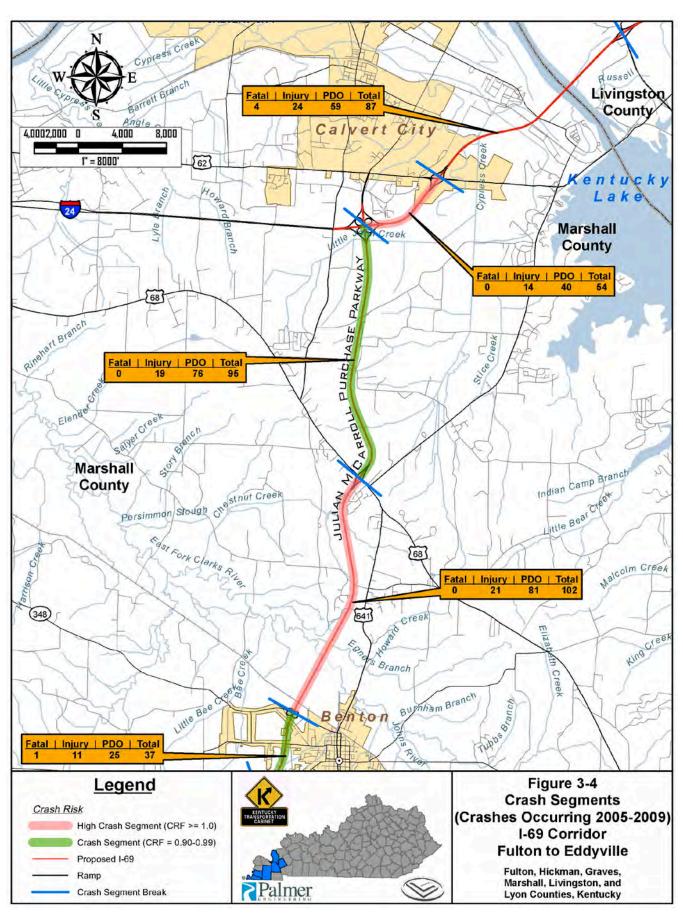
Crash Analysis Methodology

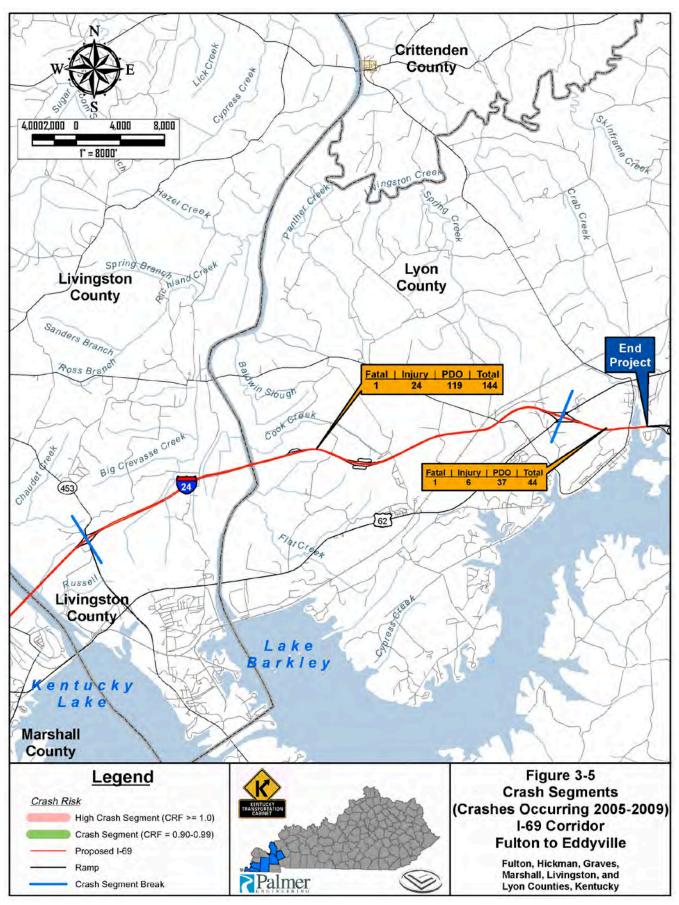
The Kentucky Transportation Center Analysis of Traffic Accident Data in Kentucky (2005-2009) was referenced for crash analysis methodology, formulas, and factors to calculate crash rates.











5. Crash Causation Factors

Determining the crash causation factors for the high crash areas will help identify potential problem areas. To identify the cause of the crash for crash rate segments, crashes for each segment were grouped into major crash types which are summarized below:

High Crash Rate Segments-Critical Crash Rate Factor >=1.0

Along the Purchase Parkway in Graves County, between MP 25.1 and MP 27.452, crash causation factors included the following:

• 50% (19 crashes) of crashes were coded *Ran Off Roadway*. This percentage is much higher than the 29% of *Ran Off Roadway* crashes that occurred along the project corridor. This segment of the Purchase Parkway has one horizontal curve and it meets the minimum horizontal curve guideline. There is one vertical curve within this segment that does not meet the current minimum stopping sight distance. The road conditions for 15 of the 19 crashes were wet or icy.

Along the Purchase Parkway in Graves and Marshall County, between MP 27.452 and MP 41.035, crash causation factors included the following:

- 43% (54 crashes) of crashes were coded Ran Off Roadway. This percentage is much higher than the 29% of Ran Off Roadway crashes that occurred along the project corridor.
 34 of the 54 crashes occurred with wet/icy/slushy road conditions. This segment includes Exit 27 – KY 131.
- Collisions with Animal represented 24% of all crashes (30 of 126 crashes), which is greater than the 19% of all cashes on the project of the same type.

Along the Purchase Parkway in Marshall County, between MP 42.555 and MP 46.942, crash causation factors included the following:

43 of the 102 (42%) crashes on this segment were Collision with Fixed Object. This
percentage is much greater than the 26% of collisions for the project corridor of the same
type.

Along I-24 in Graves and Marshall County, between MP 24.941 and MP 26.558, crash causation factors included the following:

- Collision with Fixed Object accounted for 14 of the total 54 (26%) crashes on this segment.
- 9% of the crashes on the segment were *Rear-End* collisions and 16% of collisions were coded *Sideswipe*. These percentages are comparable to the same type collisions for the project corridor, 15% and 14% respectfully.

Crash Rate Segments – Critical Crash Rate Factor = 0.9-0.99

Along the Purchase Parkway in Graves County, between MP 24.747 and MP 25.1, crash causation factors included the following:

- Collision with Fixed Object accounted for 3 of the total 10 (30%) crashes on this segment, which is higher than the project corridor average.
- 30% of crashes (3 of 10) on this segment were *Other Type* collisions which is higher than the project corridor average.

Along the Purchase Parkway in Marshall County, between MP 41.035 and MP 42.555, crash causation factors included the following:

- Sideswipe collisions accounted for 16% (6 crashes) of all crashes in this segment, which is twice the 7% of all crashes of the same type for the project corridor. Only one of these Sideswipe crashes was coded to involve an interchange ramp.
- 38% of crashes (14 of 37) on this segment were *Ran Off Roadway* collisions which is higher than the project corridor average.

Along the Purchase Parkway in Marshall County, between MP 46.942 and MP 51.398, crash causation factors included the following:

Collision with Animal accounted for 21% (20 of 95) of all crashes on this segment.

• Collision with Fixed Object, Ran Off Roadway, and All Other Types collisions were comparable to the average percentage of such collisions on the project corridor.

Concerned Fatal Crash Rate Segment Summary

Along the Purchase Parkway in Graves County, between MP 13.645 and MP 21.305, crash causation factors include the following:

- At MP 18.953, a fatal collision occurred that involved one vehicle. It had the following coding information: Directional Analysis=OTHER ROADWAY OR MID-BLOCK COLLISION, Weather=SNOWING, Roadway Character=STRAIGHT & LEVEL, Light Condition=DAYLIGHT. This collision occurred within 0.3 miles of a deficient vertical curve.
- At MP 20.573, a fatal collision occurred that involved one vehicle. It had the following coding information: Directional Analysis=OTHER COLLISIONS ON SHOULDER, Weather=CLOUDY/DRY, Roadway Character=CURVE & GRADE, Light Condition=DARK – HWY NOT LIGHTED.
- At MP 21.295, a fatal collision occurred that involved one vehicle. It had the following coding information: Directional Analysis=OVERTURNED IN ROADWAY, Weather=CLEAR/DRY, Roadway Character=CURVE & GRADE, Light Condition=DAWN. This collision occurred in the US 45 interchange in Graves County.

Along I-24 in Marshall and Livingston Counties, between MP 26.558 and MP 30.742, crash causation factors included the following:

- At MP 26.952, a fatal head on collision occurred. It had the following coding information: Directional Analysis=HEAD-ON COLLISION, Weather=CLEAR/DRY, Roadway Character=STRAIGHT & LEVEL, Light Condition=DAYLIGHT. This collision occurred at the US 62 interchange.
- At MP 28.048, a fatal collision occurred that involved one vehicle. It had the following coding information: Directional Analysis=COLLISION WITH A PEDESTRIAN, Weather=CLEAR/DRY, Roadway Character=STRAIGHT & LEVEL, Light Condition=DARK-HWY NOT LIGHTED.
- At MP 30.022, a fatal head-on collision occurred. It had the following coding information: Directional Analysis=VEHICLE GOING IN WRONG DIRECTION, Weather=CLOUDY/DRY, Roadway Character=STRAIGHT & GRADEL, Light Condition=DARK-HWY NOT LIGHTED.
- At MP 30.699, a fatal head-on collision occurred. It had the following coding information: Directional Analysis=VEHICLE GOING IN WRONG DIRECTION, Weather=CLEAR/DRY, Roadway Character=STRAIGHT & HILLCREST, Light Condition=DAYLIGHT. This collision occurred at the KY 453 interchange.

Table 3-3 Crash Types for Crash Segments

<u>High Crash Rate Segments-Critical Crash Rate Factor >=1.0</u> Purchase Parkway - Graves County - MP 25.1 – MP 27.452

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Collision with Animal	4	10.53%	111	18.75%
Collision with Fixed Object	8	21.05%	152	25.68%
Ran Off Roadway	19	50.00%	170	28.72%
All Other Types	7	18.42%	159	26.86%

Segments with % crashes higher than average for Parkway/Interstate

Table 3-3 Crash Types for Crash Segments (continued)

Purchase Parkway - Graves/Marshall Counties - MP 27.452 - MP 41.035

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Collision with Animal	30	23.81%	111	18.75%
Collision with Fixed Object	22	17.46%	152	25.68%
Ran Off Roadway	54	42.86%	170	28.72%
All Other Types	20	15.87%	159	26.86%

Purchase Parkway - Marshall County - MP 42.555 - MP 46.942

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Collision with Animal	16	15.69%	111	18.75%
Collision with Fixed Object	43	42.16%	152	25.68%
Ran Off Roadway	15	14.71%	170	28.72%
All Other Types	28	27.45%	159	26.86%

I-24 - Marshall County - MP 24.941 - MP 26.558

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Rear-End	9	16.67%	48	14.59%
Collision with Fixed Object	14	25.93%	63	19.15%
Sideswipe	9	16.67%	45	13.68%
All Other Types	22	40.74%	173	52.58%

<u>Crash Rate Segments – Critical Crash Rate Factor = 0.9-0.99</u>

Purchase Parkway - Graves County - MP 24.747 - MP 25.100

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Collision with Animal	2	20.00%	111	18.75%
Collision with Fixed Object	3	30.00%	152	25.68%
Ran Off Roadway	2	20.00%	170	28.72%
All Other Types	3	30.00%	159	26.86%

Segments with % crashes higher than average for Parkway/Interstate

Table 3-3 Crash Types for Crash Segments (continued)

Purchase Parkway - Marshall County - MP 41.035 - MP 42.555

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Sideswipe	6	16.22%	44	7.43%
Collision with Fixed Object	5	13.51%	152	25.68%
Ran Off Roadway	14	37.84%	170	28.72%
All Other Types	12	32.43%	226	38.18%

Purchase Parkway - Marshall County - MP 46.942 - MP 51.398

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Collision with Animal	20	21.05%	111	18.75%
Collision with Fixed Object	24	25.26%	152	25.68%
Ran Off Roadway	20	21.05%	170	28.72%
All Other Types	31	32.63%	159	26.86%

Table 3-4 Concerned Fatal Crash Segments

Concerned Fatal Crash Rate Segments

Purchase Parkway – Graves County – MP 13.645 – MP 21.305

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Collision with Animal	17	26.56%	111	18.75%
Collision with Fixed Object	12	18.75%	152	25.68%
Ran Off Roadway	17	26.56%	170	28.72%
All Other Types	18	28.13%	159	26.86%

I-24 – Marshall/Livingston Counties – MP 26.558 – MP 30.742

Crash Type	Crashes in Segment	% in Segment	Crashes on Parkway/Interstate	% on Parkway/Interstate
Rear-End	22	25.29%	48	14.59%
Collision with Fixed Object	18	20.69%	63	19.15%
Sideswipe	10	11.49%	45	13.68%
All Other Types	37	42.53%	173	52.58%

Segments with % crashes higher than average for Parkway/Interstate

6. Other Crash Considerations

In efforts to identify potential problem areas on Purchase Parkway, cross-over or head-on collisions and collisions near interchanges were further evaluated.

Cross-Over and Head-On Crashes

A trend of cross-over or head-on collisions on the parkway could indicate potential problems with median width and type, directional separation, or interchange signage. Between 2005 and 2009, there were six crashes coded *median cross-over* or *head-on collision*. These crashes are shown in **Table 3-5.** Cross-over and head-on crashes account for 0.3% of crashes on the Purchase Parkway and 1.2% of crashes on I-24.

COUNTY	MP	CRASH TYPE	INTERCHANGE WITH IN 1 MILE	
Purchase Parkway				
Marshall	44.6	Median cross-over	No	
Marshall	50.9	Head-on	I-24 (MP 51.4)	
Interstate 2	24			
Marshall	27.0	Head-on	US 62 (MP26.6)	
Marshall	28.7	Median cross-over	No	
Livingston	31.1	Head-on	KY 453 (MP 30.7)	
Lyon	36.0	Head-on	No	

Table 3-5 Cross-Over and Head-on Crashes

Crashes at Interchanges

As part of this analysis, crashes occurring within 0.1 mile of either direction of an interchange were summarized by crash type. The interchange crash types are summarized as follows:

Purchase Parkway

- There were 272 crashes with in 0.1 mile of the interchanges on the Purchase Parkway
- 53% (146 of 272) were coded as ramp related crashes, including rear-end and other multiple-vehicle collisions.
- 15% (42 of 272) were collisions with a fixed object

I-24

- There were 219 crashes within 0.1 mile of the interchanges on I-24.
- 44% (97 of 219) were coded as ramp related crashes, including rear-end and other multiple-vehicle collisions.
- 10% (21 of 219) were collisions with a fixed object

Table 6-2 Interchange Crash Data provides a more detailed summary of crash types at interchanges along the Purchase Parkway and I-24

B. Traffic Volumes and Operational Level of Service

A traffic analysis was conducted on the Purchase Parkway to identify any traffic congestion problems related to increased traffic on the parkway from interstate traffic projections. Current and future traffic projections were conducted based on the parkway with and without I-69.

1. Current Traffic Volumes (2010)

The 2010 traffic volumes for this project are based on data from the KYTC HIS database and traffic classification counts conducted by KYTC in 2010. Truck percentage and directional design hourly volumes were calculated based on the classification counts in 2010.

The current traffic (2010) for the Purchase Parkway ranges from 7,060 vehicles per day (vpd) in Fulton, Kentucky, to 19,200 vpd near the I-24 interchange. On I-24, the traffic volumes range from 21,900 vpd near the Purchase Parkway interchange in Marshall County to 28,200 vpd near Calvert City, Kentucky, in Marshall County. The existing truck percentages on the Purchase Parkway range from 24.9% at Mayfield, Kentucky, in Graves County to 34.5% near Benton, Kentucky, in

Marshall County. On I-24, the truck percentage is 24.9%. Average Daily Traffic and corresponding truck percentages are provided below in **Table 3-6**.

COUNTY	BEGIN MP	END MP	LENGTH (miles)	Rural/ Urban	% Trucks	2010 ADT	Los
Purchase F	arkway						
Fulton	0.000	0.360	0.36	Rural	31.5%	8,500	Α
Fulton	0.360	1.424	1.064	Rural	31.5%	7,570	Α
Fulton	1.424	2.478	1.054	Rural	31.5%	7,060	Α
Fulton	2.478	3.434	0.956	Rural	31.5%	7,290	Α
Hickman	3.434	8.352	4.918	Rural	31.5%	7,290	Α
Graves	8.352	13.645	5.293	Rural	31.5%	7,290	Α
Graves	13.645	21.305	7.66	Rural	31.5%	8,590	Α
Graves	21.305	22.239	0.934	Urban	24.9%	14,300	Α
Graves	22.239	23.701	1.462	Urban	24.9%	13,100	Α
Graves	23.701	24.747	1.046	Urban	24.9%	12,000	Α
Graves	24.747	25.100	0.353	Urban	34.5%	7,790	Α
Graves	25.100	27.452	2.352	Rural	34.5%	7,790	Α
Graves	27.452	34.487	7.035	Rural	34.5%	7,320	Α
Marshall	34.487	41.035	6.548	Rural	34.5%	7,320	Α
Marshall	41.035	42.555	1.52	Rural	32.9%	16,700	Α
Marshall	42.555	46.942	4.387	Rural	32.9%	18,800	Α
Marshall	46.942	51.398	4.456	Rural	32.9%	19,200	Α
Interstate 2	24						
Marshall	24.941	26.558	1.617	Rural	24.9%	21,900	Α
Marshall	26.558	29.352	2.794	Rural	24.9%	28,200	В
Livingston	29.352	30.742	1.39	Rural	24.9%	28,200	В
Livingston	30.742	33.880	3.138	Rural	24.9%	25,700	В
Lyon	33.880	39.553	5.673	Rural	24.9%	25,700	В
Lyon	39.553	41.647	2.094	Rural	24.9%	25,500	В

Table 3-6 Current Traffic Characteristic (2010)

Also included as part of this study is the Directional Design Hourly Volume (DDHV) in the context of minimum outside shoulders discussed in **Chapter IV**. The following table summarizes the DDHV data for the Purchase Parkway and I-24 based on classification counts conducted by KYTC in 2010 (**Appendix E**).

COUNTY	Rural/ Urban	DDHV	% Trucks at Peak Hour	DDHV	% Trucks at Peak Hour	
Purchase F	Parkway					
		Nort	hbound	Sout	thbound	
Fulton	Rural	341	24%	360	24%	
Hickman	Rural	293	24%	309	24%	
Graves	Rural	345	24%	364	24%	
Graves	Urban	574	21%	605	22%	
Marshall	Rural	771	28%	813	29%	
Interstate 24						
		Eastbound		Wes	stbound	
Marshall	Rural	1132	15%	1194	17%	
Livingston	Rural	1132	15%	1194	17%	
Lyon	Rural	1032	16%	1088	19%	

Table 3-7 Existing (2010) Directional Design Hourly Volumes (DDHV)

2. Future Traffic Volumes (2040) without I-69

The future traffic volumes (2040) were calculated using growth rates based on available previous studies. The future traffic volumes are shown in **Table 3-8**. The annual growth rate used for the Purchase Parkway and I-24 without I-69 is 2.0%. This growth rate resulted in a range from 12,800 vpd to 34,800 vpd on the Purchase Parkway and from 39,700 vpd to 51,100 vpd on I-24.

COUNTY	BEGIN MP	END MP	LENGTH (miles)	Rural/ Urban	% Trucks	2040 ADT	LOS		
Purchase Parkway									
Fulton	0.000	0.360	0.36	Rural	31.5%	15,397	Α		
Fulton	0.360	1.424	1.064	Rural	31.5%	13,712	Α		
Fulton	1.424	2.478	1.054	Rural	31.5%	12,788	Α		
Fulton	2.478	3.434	0.956	Rural	31.5%	13,205	Α		
Hickman	3.434	8.352	4.918	Rural	31.5%	13,205	Α		
Graves	8.352	13.645	5.293	Rural	31.5%	13,205	Α		
Graves	13.645	21.305	7.66	Rural	31.5%	15,560	Α		
Graves	21.305	22.239	0.934	Urban	24.9%	25,902	В		
Graves	22.239	23.701	1.462	Urban	24.9%	23,729	В		
Graves	23.701	24.747	1.046	Urban	24.9%	21,736	Α		
Graves	24.747	25.100	0.353	Urban	34.5%	14,111	Α		
Graves	25.100	27.452	2.352	Rural	34.5%	14,111	Α		
Graves	27.452	34.487	7.035	Rural	34.5%	13,259	Α		
Marshall	34.487	41.035	6.548	Rural	34.5%	13,259	Α		
Marshall	41.035	42.555	1.52	Rural	32.9%	30,250	В		
Marshall	42.555	46.942	4.387	Rural	32.9%	34,054	В		
Marshall	46.942	51.398	4.456	Rural	32.9%	34,778	В		
Interstate 2	24								
Marshall	24.941	26.558	1.617	Rural	24.9%	39,669	В		
Marshall	26.558	29.352	2.794	Rural	24.9%	51,080	С		
Livingston	29.352	30.742	1.39	Rural	24.9%	51,080	С		
Livingston	30.742	33.880	3.138	Rural	24.9%	46,552	С		
Lyon	33.880	39.553	5.673	Rural	24.9%	46,552	С		
Lyon	39.553	41.647	2.094	Rural	24.9%	46,190	С		

Table 3-8 Future Traffic Volumes without I-69

3. Future Traffic Volumes (2040) with I-69

The future traffic volumes (2040) with I-69 are shown in following table and figure. The annual growth rate used was 2.5%, which is consistent with previous studies. This growth rate resulted in

traffic volumes ranging from 14,800 vpd to 40,300 vpd on the Purchase Parkway and from 45,900 vpd to 53,900 vpd on I-24.

COUNTY	BEGIN MP	END MP	LENGTH (miles)	Rural/ Urban	% Trucks	2040 ADT	LOS		
Purchase Parkway									
Fulton	0.000	0.360	0.36	Rural	31.5%	17,829	Α		
Fulton	0.360	1.424	1.064	Rural	31.5%	15,879	Α		
Fulton	1.424	2.478	1.054	Rural	31.5%	14,809	Α		
Fulton	2.478	3.434	0.956	Rural	31.5%	15,291	Α		
Hickman	3.434	8.352	4.918	Rural	31.5%	15,291	Α		
Graves	8.352	13.645	5.293	Rural	31.5%	15,291	Α		
Graves	13.645	21.305	7.66	Rural	31.5%	18,018	Α		
Graves	21.305	22.239	0.934	Urban	24.9%	29,995	В		
Graves	22.239	23.701	1.462	Urban	24.9%	27,478	В		
Graves	23.701	24.747	1.046	Urban	24.9%	25,171	В		
Graves	24.747	25.100	0.353	Urban	34.5%	16,340	Α		
Graves	25.100	27.452	2.352	Rural	34.5%	16,340	Α		
Graves	27.452	34.487	7.035	Rural	34.5%	15,354	Α		
Marshall	34.487	41.035	6.548	Rural	34.5%	15,354	Α		
Marshall	41.035	42.555	1.52	Rural	32.9%	35,029	В		
Marshall	42.555	46.942	4.387	Rural	32.9%	39,434	С		
Marshall	46.942	51.398	4.456	Rural	32.9%	40,273	С		
Interstate 2	24								
Marshall	24.941	26.558	1.617	Rural	24.9%	45,937	С		
Marshall	26.558	29.352	2.794	Rural	24.9%	59,151	С		
Livingston	29.352	30.742	1.39	Rural	24.9%	59,151	С		
Livingston	30.742	33.880	3.138	Rural	24.9%	53,907	С		
Lyon	33.880	39.553	5.673	Rural	24.9%	53,907	С		
Lyon	39.553	41.647	2.094	Rural	24.9%	53,488	С		

Table 3-9 Future Traffic Volumes with I-69

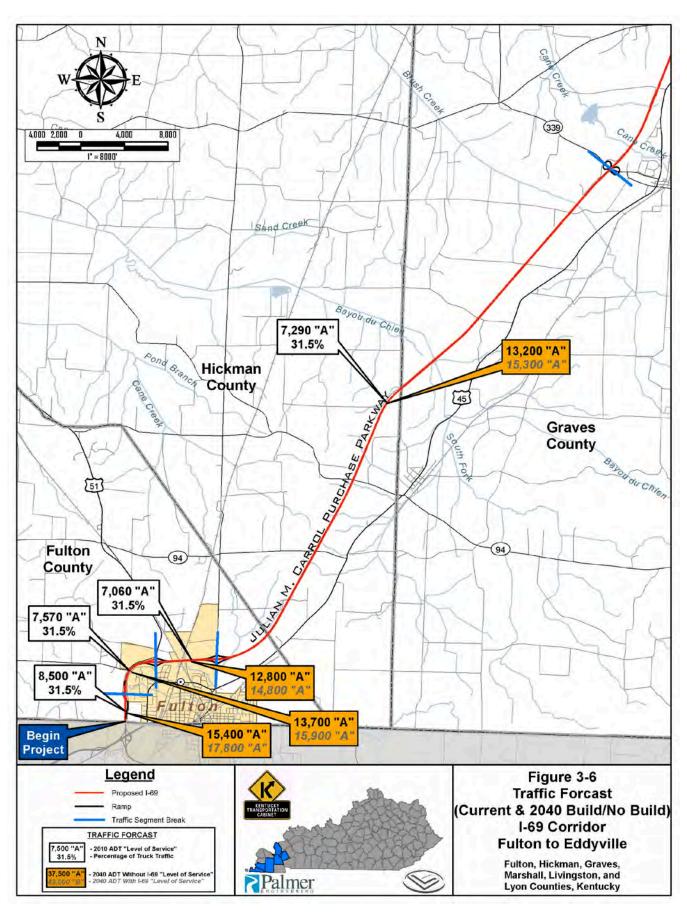
4. Level of Service

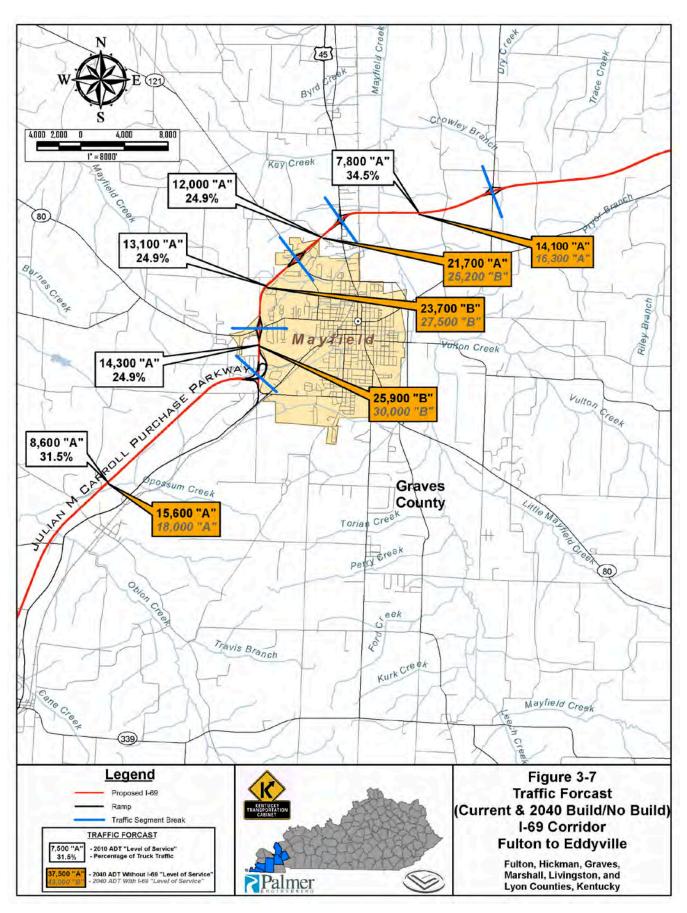
Level of service (LOS), as defined by the *Highway Capacity Manual 2000*, is a quality measure describing operational conditions within a traffic stream, based on services measures such as speed and travel time, freedom to maneuver, traffic interruptions, comfort and convenience. There are six LOS and are designated by the letters A through F. LOS A represents the best operating conditions and service and LOS F represents the worst.

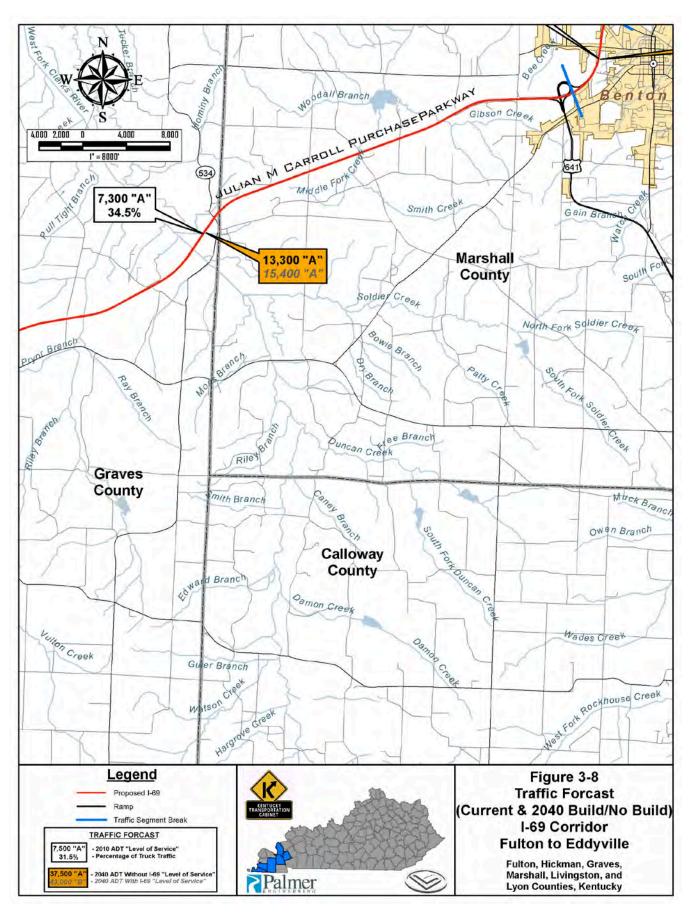
It is preferred to design a rural interstate to a LOS B, but a LOS C is acceptable. For an urban interstate, it is preferred to design to a LOS C, but a LOS D is acceptable. The *Highway Capacity Manual 2000 Edition* and Highway Capacity Software were used to calculate the LOS for the project area. The LOS was calculated based on the mainline geometry and traffic operations for the Purchase Parkway and I-24. This LOS does not represent the LOS for interchanges in the project corridor.

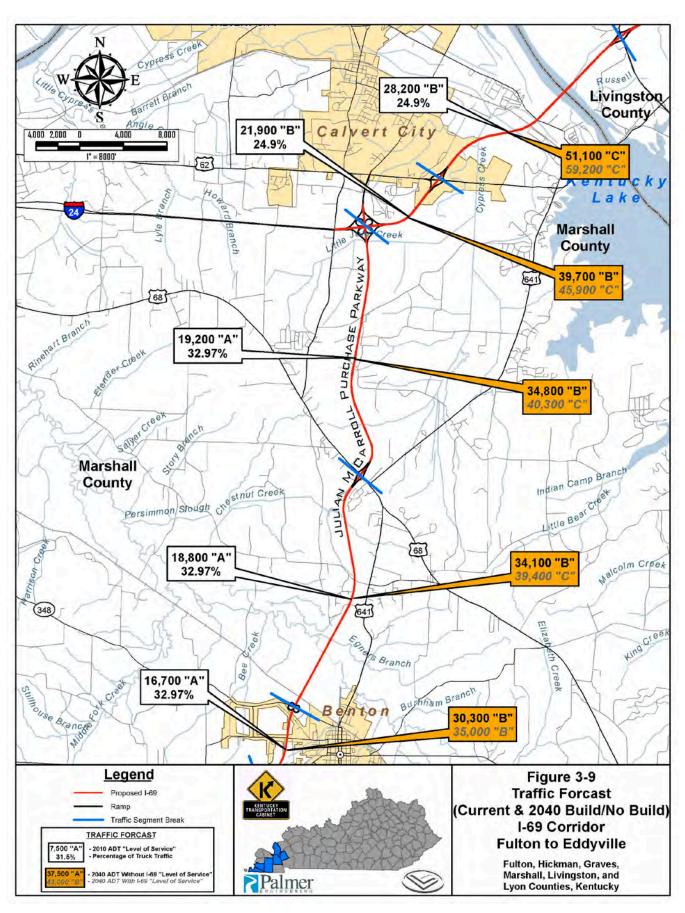
Referring to **Tables 3-8** and **3-9**, the Purchase Parkway and I-24 will operate at a LOS C or better with or without the estimated additional I-69 projected traffic. There is an increase from LOS B to LOS C with the increased projected I-69 traffic in Marshall County (MP 42.555 – 51.398) on the Purchase Parkway and on I-24 in Marshall County (MP 24.941 – MP 26.558).

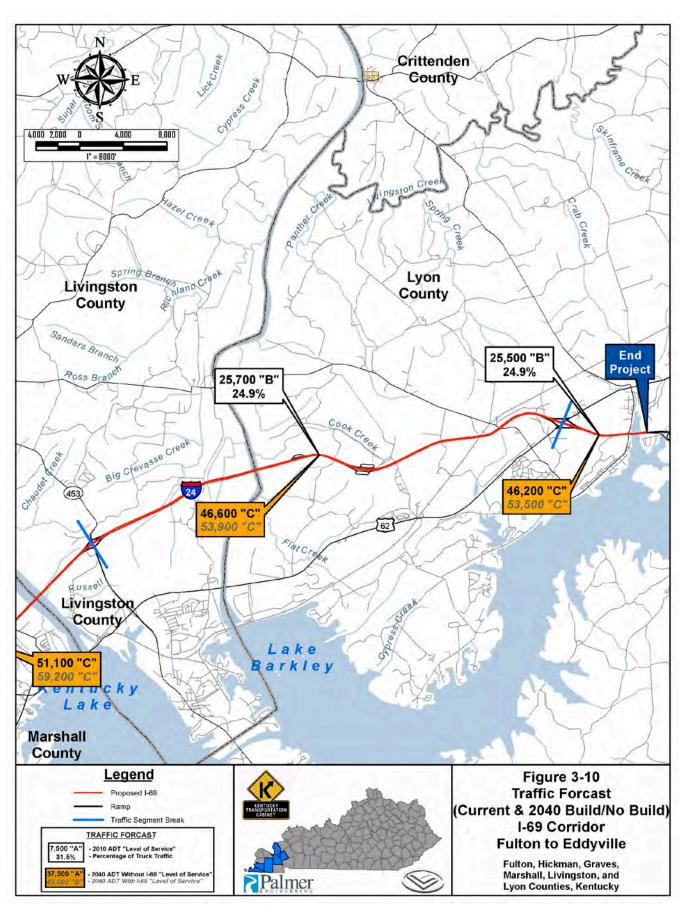
Figure 3-6 through **Figure 3-10** illustrates the current and future traffic projections with and without I-69 on the Purchase Parkway and I-24.











5. I-24 and Purchase Parkway Interchange Ramp Volumes

ADT traffic volumes were evaluated at the I-24 and Purchase Parkway interchange. The most current ramp traffic volumes were collected in 2007. The future traffic volumes (2040) without I-69 were calculated using a 2% annual growth rate. The future traffic volumes (2040) with I-69 were calculated using a 2.5% annual growth rate. The design hourly volumes (DHV) shown in **Figure 3-11** was calculated using 15% of the ADT. The northbound Purchase Parkway to eastbound I-24 movement has the largest ramp volume in the interchange with a projected 2040 DHV of 911 vehicles without I-69 and 1,054 vehicles with I-69. The second largest ramp volume is the westbound I-24 to southbound Purchase Parkway movement with a projected 2040 DHV of 820 vehicles without I-69 and 950 vehicles with I-69.



Figure 3-11 I-24 and Purchase Parkway Interchange Ramp Volumes

IV. MAINLINE GEOMETRY/TYPICAL SECTION

A Policy of Geometric Design of Highways and Streets, 5th Edition, 2004, published by the AASHTO provides design guidelines for streets and highways. This manual is commonly referred to as The Green Book. For guidelines related to roadside clearance, obstructions, and barriers, the Roadside Design Guide, Current Edition, written by AASHTO was referenced. More specific guidance on interstate standards is provided in AASHTO's A Policy on Design Standards Interstate System, Current Edition.

The intent of the ASSHTO Green Book is to provide guidance for the design of highways and streets. The book references a recommended range of values for critical dimensions based on established practices and recent research. The Green Book provides minimum values for critical dimensions of roadway design.

In order for the Purchase Parkway to be designated as I-69, the existing geometric conditions need to be compared to current interstate guidelines set forth by AASHTO. To identify these deficiencies and necessary design modifications, design information on the Purchase Parkway was collected from the Kentucky Transportation Cabinet's HIS, *As-Built Plans* for the Parkway, and site visits and compared to the current AASHTO guidelines for interstate highway facilities. A summary of this information is provided in **Appendix F.**

This chapter reviews the existing design speed, median width and type, clear zone, horizontal and vertical alignments, superelevation rates, and sight distance and compares them to the AASHTO guidelines. Although this chapter is based on comparing the existing geometric conditions of the Purchase Parkway to the minimum AASHTO guidelines set forth in the Green Book and other references mentioned, the Green Book permits and encourages sufficient design flexibility based on the project needs.

A. Design Speed

The design speed of a facility dictates many of the geometric design parameters. The design speed selected is meant to satisfy a level of public expectation for safety and LOS.

The AASHTO minimum mainline design speed for a rural interstate is 70 mph and 50 mph for an urban interstate. According to the As-Built plans, this minimum requirement is met for the Purchase Parkway. The following sections and chapters that review existing mainline geometric conditions of the Purchase Parkway is based on these design speeds.

B. Typical Roadway Sections

The Purchase Parkway has two typical roadway cross-sections. The Mayfield Bypass section of the Purchase Parkway was designed to lesser standards than the Purchase Parkway and is considered to be held to urban design standards due to its location within the city of Mayfield. These typical sections generally represent the existing conditions along the Parkway; however, any improvements made over the life of the Parkway may have resulted in changes to information that may not be represented in this study. The typical sections of the Purchase Parkway are shown in **Figure 4-1.**

1. Lane Widths

The minimum lane width of a freeway facility is 12 feet. The existing lane widths of the Purchase Parkway mainline is 12 feet, therefore meeting the minimum AASHTO guidelines for interstate design.

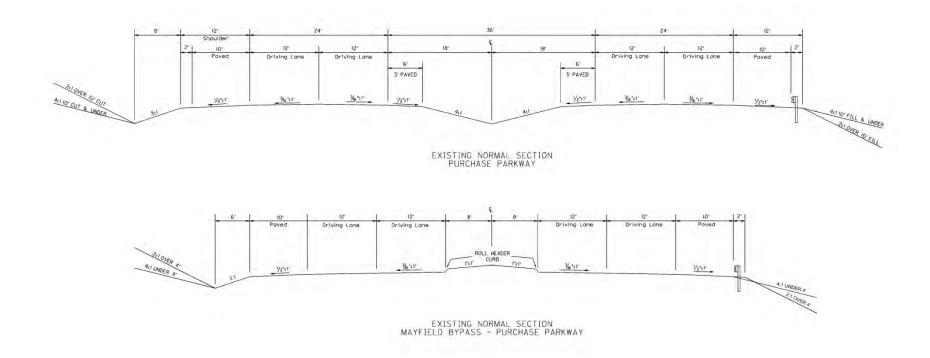


Figure 4-1 Existing Purchase Parkway Typical Section

Note: Typical Sections are based upon As-Built Plans provided by the Kentucky Transportation Cabinet

2. Shoulder Widths

The minimum AASHTO guidelines for interstate shoulders are 10 foot paved outside shoulder and 4 foot paved inside shoulder for each direction of travel. According to the As-built plans, the inside shoulder width on the Purchase Parkway is 6 foot wide, of which 3 feet is paved. The Mayfield Bypass does not have an inside shoulder. The AASHTO guideline for an inside shoulder is 4 feet paved. Therefore, the entire length of the Purchase Parkway does not comply with the AASHTO design guidelines for the inside shoulder width on freeways. Based on the existing (2010) directional design hourly volumes (**Table 3-7**), the Purchase Parkway outside paved shoulder width meets the minimum criteria.

A more detail summary of the locations of inside and outside widths is presented in

PURCHASE PARKWAY	COUNTY	BEGIN MP	END MP	LENGTH (miles)	Shoulder Width (ft)	Paved Shoulder Width (ft)	AASHTO MINIMUM
	Fulton	0	3.43	3.43	12	10	10 ft paved (Truck DDHV <= 250 vph)
	Hickman	3.43	8.35	4.92	12	10	
OUTSIDE SHOULDERS	Graves	8.35	21.89	13.54	12	10	
OUTSIDE SHOULDERS	Graves	21.89	24.90	3.01	10	10	
	Graves	25.1	34.49	9.39	12	10	
	Marshall	34.49	51.40	16.91	12	10	
	Fulton	0	3.43	3.43	6	3	
	Hickman	3.43	8.35	4.92	6	3	
INSIDE SHOULDERS	Graves	8.35	21.9	13.55	6	3	4 ft payed
INSIDE SHOOLDERS	Graves	21.89	24.90	3.01	0	0	4 ft paved
	Graves	24.90	34.49	9.59	6	3	
	Marshall	34.49	51.40	16.91	6	3	

Table 4-1 Summary of Inside and Outside Shoulder Widths

3. Median Width and Type

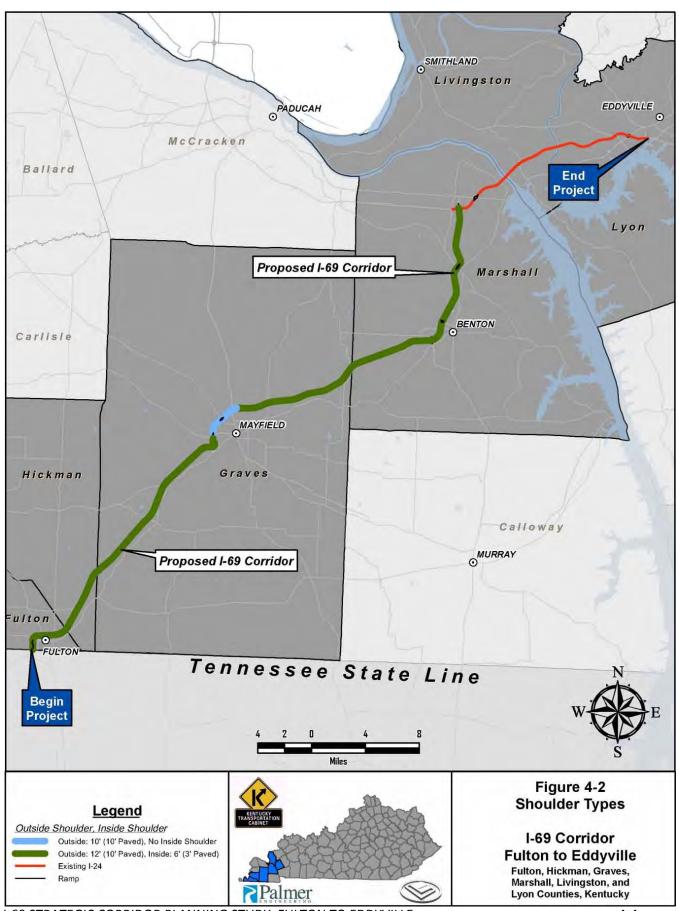
The purpose of a median separation is to provide driver comfort and safety. The width of a median is measured from the inside edge of the travel lane in one direction to the inside edge of the travel lane in the other direction. The median width also includes the shoulder width. The Purchase Parkway has a 36 foot depressed median, except for the Mayfield Bypass (MP 21.9-MP 24.9). This section has a 16 foot raised mountable median.

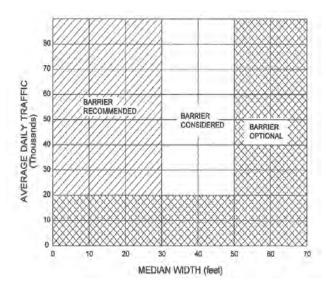
Guidelines contained within the AASHTO Green Book recommends a minimum 50 foot median for a rural interstate. Within the AASHTO *Policy on Design Standards, Interstate System* a minimum 36 foot median for a rural interstate is required. According to



Mayfield Bypass median and inside paved shoulder width does not meet current interstate standard

the Green Book, the minimum guidelines for an urban interstate are based on the number of lanes and number of large trucks. A ten foot median is recommended for a four lane urban interstate. This would allow for 4 foot inside shoulders and a 2 foot concrete median. For urban interstates with more than two lanes in each direction, the minimum median width is 22 feet for truck volumes less than 250 vph and 26 feet for truck volumes greater than 250 vph.





Suggested guidelines for median barriers on high-speed roadways

The AASHTO's Roadside Design Guide is referenced to determine the warrants for barrier installation in the median. The guide provides a warrants chart based on average daily traffic, median width, and crash history. The chart to the left (Figure 6.1 in the Roadside Design Guide) is the warrant chart for the suggested guidelines for the installation of a median barrier on a high speed facility taken from Chapter 6 of the Roadside Design Chapter 6 has a detailed discussion of the installation of median barrier and curbs. Besides drainage purposes. serving curbs are not recommended on high speed facilities.

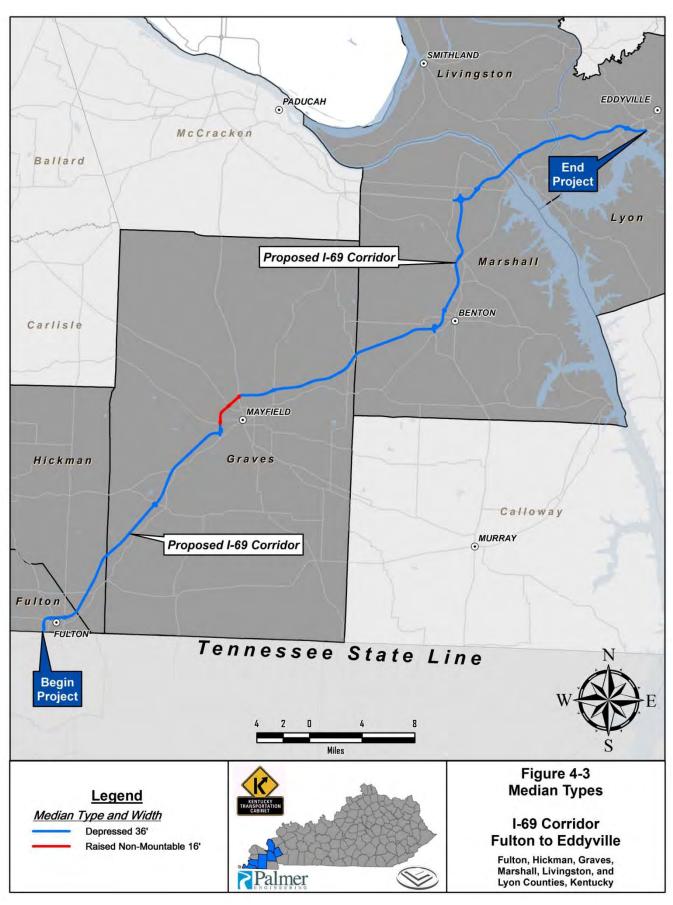
The median type and widths are provided below in **Table 4-2**. The location of these median attributes is shown in **Figure 4-3**.

The crash history review in Chapter III indicates that there is not a history of cross-over collisions on the Purchase Parkway. The current traffic (2010) volumes for the Purchase Parkway ranges from 7,060 vpd to 19,200 vpd. The future traffic (2040) volumes ranges from 12,800 vpd to 34,800 vpd without I-69 designation.

PURCHASE PARKWAY	COUNTY	BEGIN MP	END MP	LENGTH (miles)	MDIAN TYPE	MEDIAN WIDTH (ft)	AASHTO MINIMUM MEDIAN WIDTH	
Rural	Fulton	0.00	3.43	3.43	Depressed	36	RURAL	
Rural	Hickman	3.43	8.35	4.92	Depressed	36		
Rural	Graves	8.35	20.50	12.15	Depressed	36	Depressed 36 ft	
Urban	Graves	20.50	21.89	1.39	Depressed	36		
Urban	Graves	21.89	24.90	3.01	Raised Non Mountable	16	LIDDAN	
Urban	Graves	24.90	25.10	0.20	Depressed	36	URBAN 10ft	
Rural	Graves	25.10	34.49	9.39	Depressed	36	1011	
Rural	Marshall	34.49	51.40	16.91	Depressed	36		

Table 4-2 Summary of Median Types and Widths along Purchase Parkway

Based on the references, minimum guidelines and ADT, the Purchase Parkway median is in compliance.



4. Clear Zones

The clear zone of a roadway is the area outside the edge of the travel lane, including the shoulder, that is free of obstructions and used for vehicle recovery. Guidelines for clear zone widths for roadways based on design speed, traffic volumes, fill/cut slopes, ditch slopes, and distance from fixed obstruactions such as bridge piers, sign supports, culvert headwalls, trees, rock outcrops, and drainage channels are provided in the *Roadside Design Guide*.

A foreslope of 1V:4H or flatter is considered recoverable and a foreslopes between 1V:3H and 1V:4H is considered traversable, but non-recoverable. As described in the guide, the recommendation for a clear zone range is 30 to 46 feet for recoverable fill slopes (1V:4H or flatter)

on a roadway with a design speed of 70 mph and Average Daily Traffic (ADT) greater than 6,000 vehicles. Foreslopes steeper than 1V:4H, obstructions should not be present in the vicinity of the toes of these slopes.

For a roadway in a cut section, the clear zone for backslope of 1V:3H or flatter varies from 22 feet to 30 feet.

In the review of the as-built plans, the fill and cut slopes vary from 1V:2H to 1V:4H, see typical provided in **Figure 4-1**. These side slopes depend on the height of fill or depth of cut required. Based on information available in as-built plans, it is not possible to readily summarize the clear zone distances for the Purchase Parkway.



Clear zones may require guardrail placement or extensions to meet current standards.

5. Guardrail Placement and Condition

Guardrail is a longitudinal barrier to shield motorists from natural or man-made obstacles located on either side a traveled way. The guardrail protects a vehicle potentially leaving the roadway by absorbing the vehicle's energy, protecting it from roadside hazards. Chapter 5 of the *Roadside Design Guide* addresses the application and situation of guardrail placement. The information available on the as-built plans does not provide sufficient information to determine if the guardrail placement on the Purchase Parkway meets the current standard. A field review of the existing guardrail end treatment was conducted on the Purchase Parkway mainline and interchange ramps. This review showed that the end treatments on parkway meet current KYTC standards.

C. Horizontal Alignment

This section includes the review of existing superelevation and horizontal curvature for the Purchase Parkway and compares them to the current minimum standards.

1. Superelevation Rate

Superelveation (road banking) is the physical tilting of the roadway to help counteract the centripetal forces developed as a vehicle goes around a curve. Superelevation and friction keep a vehicle from sliding off the roadway while traveling through a curve. Superelevation is calculated based on design speed and horizontal curvature of the roadway. According to the AASHTO Policy on Geometric Design of Highways and Streets, current edition (commonly referred to as the Green Book), the maximum superelevation rate is controlled by climate conditions, terrain conditions, type of area, and frequency of slow-moving vehicles that may be affected by high superelevation rates. A specific maximum superelevation is not recommended for an Interstate facility by AASHTO. It is left to the user agencies to make specific policy decisions concerning allowable rates of superelevation. The KYTC policy references the Green Book for freeway geometric design. The Green Book provides superelevation rate tables for 4%, 6%, 8%, 10%, and 12% maximum superelevations. From review of as-builts plans and field inspection, it appears that the Purchase

Parkway was constructed on the basis of a 10% maximum superelevation. The superelevations for the Purchase Parkway are compiled in **Appendix F**.

2. Degree of Horizontal Curvature

guidelines for horizontal curvature design were designated by degree of curvature during the design of the Purchase Parkway. The existing parkway was designed to a maximum 3° 00' curve, which equates to minimum radius of approximately 1910 feet. current AASHTO guidance for minimum curvature references curve radius rather than degree of curvature. The current minimum horizontal curvature radius for a design speed of 70 mph for a rural interstate is 1810 feet, which equates to approximately 3°10' of curvature. The minimum radius for an urban interstate with a 50 mph design speed is 758 feet or Horizontal curvature along the Purchase Parkway meets Interstate approximately 7° 33' of curvature.



design guidelines

The smallest radius of curve is 1146 feet located on the Mayfield Bypass at MP 21.585 and MP 21.793, which equals a design speed of 59 mph with a 8% superelevation. Although this radius is the smallest, it meets the current minimum radius of 758 feet for an urban interstate with a design speed of 50 mph. The horizontal curves that are closest to exceeding the minimum radius for a rural interstate are located in Fulton County at MP 1.022 and Marshall County at MP 47.42. This curve has a radius of 1910 feet which still meets the minimum criteria. A compilation of horizontal curves is located in Appendix F.

D. **Vertical Alignment**

The vertical alignment of a roadway depends on the existing terrain. The changes in vertical terrain result in the introduction of vertical curves into the roadway design. A vertical curve is classified as sag or crest. A sag vertical curve is used when traversing through a valley, and a crest vertical curve is introduced when traveling over a hill. The design of these curves is critical to stopping sight distance. Stopping sight distance is measured by how far a driver can see while traveling in typical vehicle and still maintain the ability to stop within that distance. Design speed, length of vehicle light beam distance, and approach and departing grades determine the length of crest and sag vertical curves.

1. Vertical Grade

The Purchase Parkway is considered to have a rolling terrain. AASHTO guidelines designate a maximum 4% vertical grade for a rural section with a rolling terrain and 5% for an urban section. According to the as-built plans, the Purchase Parkway meets this maximum criteria for both urban and rural sections. The largest vertical grade is at the vertical curves located at MP 21.075 (3.84%), MP 21.245 (3.84%, -3.87%), and MP 21.463 (-3.87%) on the Mayfield Bypass. The review of the as-built plans for the Purchase Parkway showed all sections met the maximum vertical grade.

2. Vertical Length of Curve

The minimum length of curve was calculated based on the vertical grades of the approaching alignment, recommended rate of vertical curvature for a design speed. There are eight vertical curves that do not meet the recommended vertical length of curve based on this calculation. These eight vertical curve deficiencies are located in rural areas in Graves County at:

- MP 14.965 Actual 500 feet, calculated minimum 696 feet;
- MP 18.727 Actual 600 feet, calculated minimum 624 feet;
- MP 25.320 Actual 536 feet, calculated minimum 584 feet;
- MP 27.517 Actual 500 feet, calculated minimum 543 feet;
- MP 28.625 Actual 400 feet, calculated minimum 438 feet;
- MP 29.970 Actual 400 feet, calculated minimum 416 feet;
- MP 31.144 Actual 400 feet, calculated minimum 467 feet;
- MP 31.646 Actual 600 feet, calculated minimum 608 feet.

3. Stopping Sight Distance

Stopping sight distance was reviewed for all vertical curves on the Purchase Parkway. Stopping sight distance is calculated based on design speed and sight distance. The minimum stopping sight distance for a 70 mph design is 730 feet and 425 feet for a 50 mph design speed. There are three vertical curves that have less than the minimum stopping sight distance. They are located in rural areas in Graves County at:

- MP 14.965 Calculated Stopping Sight Distance 554 feet;
- MP 18.727 Calculated Stopping Sight Distance 727 feet;
- MP 25.320 Calculated Stopping Sight Distance 721 feet.

V. **BRIDGES AND OVERPASSES**

The Purchase Parkway has eighty-five bridge structures; a detailed table of data for these structures is included in Appendix F. A mainline bridge is a structure that carries the parkway through traffic. An overpass bridge is a structure that carries traffic over the mainline roadway. The following table illustrates the breakdown of mainline and overpass bridges and culverts on the Purchase Parkway. Only overpass bridge structures on I-24 were evaluated for this study since it is already an interstate. There are five overpass bridge structures on I-24 within the project study area.

TYPE	NUMBER OF STRUCTURES
MAINLINE BRIDGES	46
OVERPASSES	35
CULVERTS	6
TOTAL	87

Table 5-1 Summary of Structure Types for Purchase Parkway

The concerns for mainline bridge structures on the Parkway and overpass bridge structures passing above the Parkway and I-24 are the lateral widths and vertical height clearance. In addition to geometric design, functional and structural condition of these bridges is a concern. Given the increased traffic, especially truck traffic, the functional and structural capacity of these structures is a safety concern. The following discussions include lateral and vertical clearance issues, condition, and safety appurtenances to identify structures in the project area that are deficient under current design guidelines.

Lateral Clearances of Bridges

Lateral clearance is defined as the width of a mainline bridge, measured from curb to curb. The lateral clearances of the Parkway's mainline bridges were evaluated to determine if they were too narrow to meet current design guidelines.

According to the latest AASHTO guidelines, A Policy on Design Standards Interstate System (American Association of State Highway Officials, 2005), the width of a mainline bridge, less than 200 feet in length, shall equal the full paved width of the approach roadway. The full paved width of the approach roadway includes the two 12 foot travel lanes, 4 foot inside paved shoulder and 10 foot outside paved shoulder for a total of 38 feet. AASHTO guidelines allow bridges over 200 feet in length be evaluated individually and that the minimum distance between the travel lane and barrier shall be at least 3.5 feet for these bridges. Therefore, a bridge over 200 feet can have a The mainline bridges that do not meet minimum lateral clearance minimum clearance of 31 feet (2-12 foot lanes and 3.5 foot inside and outside shoulders). Further guidance is given



are greater than 200 ft long.

on evaluating long bridges, over 200 feet in length. From page 506 the Green Book:

"On bridges longer than 60 m [200 ft], some economy in substructure costs may be gained by building a single structure rather than twin parallel structures. In such cases, the approach shoulder widths are provided and a median barrier is extended across the bridge."

Further discussion of lateral clearance on long bridges is found on page 760 of the same reference:

"On long bridges, particularly on long-span structures where cost per square meter [yard] is greater than the cost on short-span structures, widths that are less than ideal may be acceptable; however, economy alone should not be the governing factor in determining structure widths. The analysis of traffic characteristics, safety features, emergency contingencies, and benefit/cost ratios should be fully considered before the desirable structure width is compromised."

The following paragraph taken from *A Policy on Design Standard Interstate System*, 2005 addresses existing bridges to remain in place when a route is to be incorporated in the interstate system:

"Mainline bridges on the interstate system and bridges on routes to be incorporated into the system may remain in place if, as a minimum, they meet the following: a) the bridge cross section consists of 3.6 m (12 ft) lanes, 3.0 m (10 ft) shoulder on the right and 1.1 m (3.5 ft) shoulder on the left; b) for long bridges, the offset to the face of parapet or bridge rail on both the left and right is 1.1 m (3.5 ft) measured from the edge of the nearest traveled lane; c) bridge railing shall meet or be upgraded to current standards."

The following table **(Table 5-2)** summarizes the length and horizontal width of the Purchase Parkway mainline bridges. The horizontal clearance is measured from curb to curb.

All bridges less than 200 feet in length meet the minimum horizontal clearance criteria. All of the bridges that do not meet the minimum horizontal clearance, of which there are 10, are over 200 feet in length.

In addition to the lateral clearance, mainline bridge side railings/barriers are a concern for bridges on the interstate system. Since the construction of the bridges on the Purchase Parkway, side railings/barriers design guidelines have been modified to improve crash worthiness. Mainline bridges on the Purchase Parkway are constructed with a vertical barrier railing with aluminum handrail and a 10 inch high curb, or brush block, which does not meet current specifications. Retrofitting options are available to update the bridge railing to meet current crash worthy criteria.

Figures 5-1 through **5-5** show the locations of the bridges that do not meet the minimum lateral clearance. The bridge lateral clearance was not collected for I-24 structures.

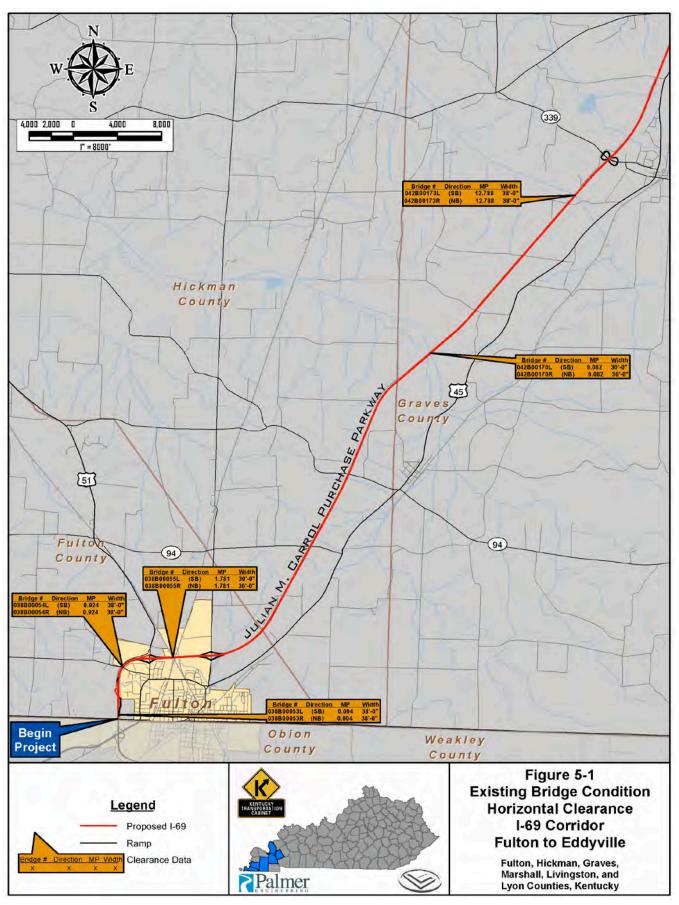


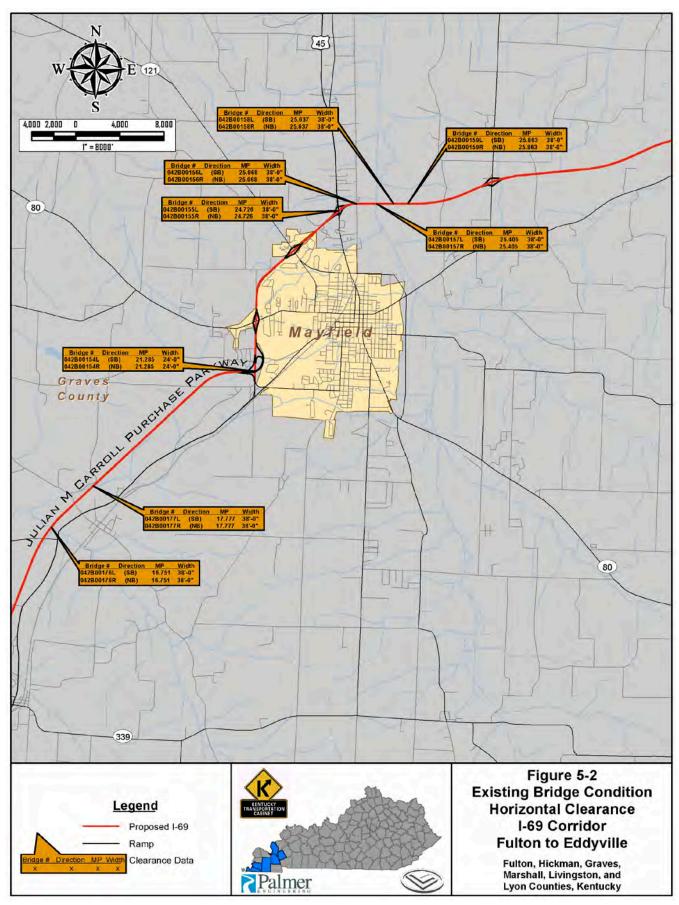
All of the Purchase Parkway mainline bridges have side railings/barriers that do not meet current standards.

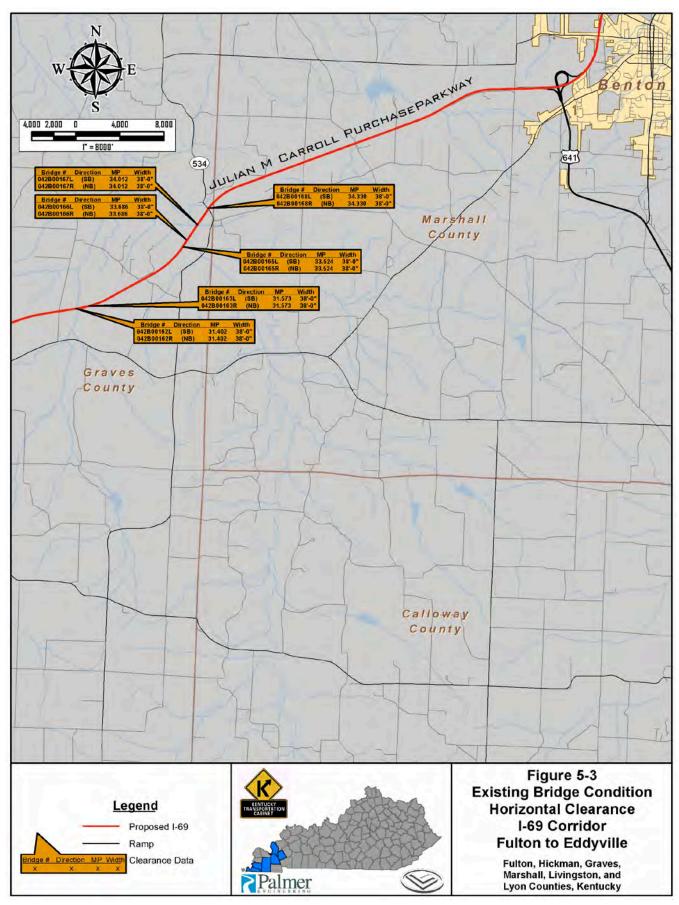
COUNTY	BRIDGE NO.	MP	FEATURES INTERSECTED	LENGTH (ft)	WIDTH (GUTTER TO GUTTER)(ft)
Fulton	B00053L	0.004	KY 116 (W. State Line St)	153	38
Fulton	B00053R	0.004	KY 116 (W. State Line St)	153	38
Fulton	B00054L	0.924	KY 166 (Middle Rd)	142	38
Fulton	B00054R	0.924	KY 166 (Middle Rd)	142	38
Fulton	B00055L	1.781	Illinois Central RR	539	30
Fulton	B00055R	1.781	Illinois Central RR	485	30
Graves	B00170L	9.082	Bayou Du Chien Creek	310	30
Graves	B00170R	9.082	Bayou Du Chien Creek	310	30
Graves	B00173L	12.788	Bush Creek	127	38
Graves	B00173R	12.788	Bush Creek	127	38
Graves	B00176L	16.751	Obion Creek	208	38
Graves	B00176R	16.751	Obion Creek	208	38
Graves	B00177L	17.777	Opossum Creek	211	38
Graves	B00177R	17.777	Opossum Creek	211	38
Graves	B00154L	21.285	US 45 (Mayfield Bypass)	208	24
Graves	B00154R	21.285	US 45 (Mayfield Bypass)	201	24
Graves	B00155L	24.726	US 45 (Paducah Rd)	238	38
Graves	B00155R	24.726	US 45 (Paducah Rd)	238	38
Graves	B00156L	25.068	Illinois Central RR	172	38
Graves	B00156R	25.068	Illinois Central RR	172	38
Graves	B00157L	25.405	Mayfield Creek	208	38
Graves	B00157R	25.405	Mayfield Creek	208	38
Graves	B00158L	25.637	Mayfield Creek Overflow No. 1	97	38
Graves	B00158R	25.637	Mayfield Creek Overflow No. 1	97	38
Graves	B00159L	25.863	Mayfield Creek Overflow No. 2	97	38
Graves	B00159R	25.863	Mayfield Creek Overflow No. 2	97	38
Graves	B00162L	31.402	Panther Creek	189	38
Graves	B00162R	31.402	Panther Creek	189	38
Graves	B00163L	31.573	Panther Creek Overflow	97	38
Graves	B00163R	31.573	Panther Creek Overflow	97	38
Graves	B00165L	33.524	West Fork Clarks River Overflow 1	97	38
Graves	B00165R	33.524	West Fork Clarks River Overflow 1	97	38
Graves	B00166L	33.686	West Fork Clarks River	208	38
Graves	B00166R	33.686	West Fork Clarks River	208	38
Graves	B00167L	34.012	West Fork Clarks River Overflow 2	108	38
Graves	B00167R	34.012	West Fork Clarks River Overflow 2	108	38
Graves	B00168L	34.330	KY 564 (Wayne Freeman Rd)	132	38
Graves	B00168R	34.330	KY 564 (Wayne Freeman Rd)	132	38
	B00074L	42.748	NC & St. Louis RR	158	38
	B00074R	42.748	NC & St. Louis RR	158	38
	B00075L	43.277	Clarks River Relief No. 1	291	30
Marshall	B00075R	43.277	Clarks River Relief No. 1	291	30
Marshall	B00076L	43.614	East Fork Clarks River	519	30
Marshall		43.614	East Fork Clarks River	519	30
Marshall		43.872	Clarks River Relief No. 2	387	30
Marshall		43.872	Clarks River Relief No. 2	387	30

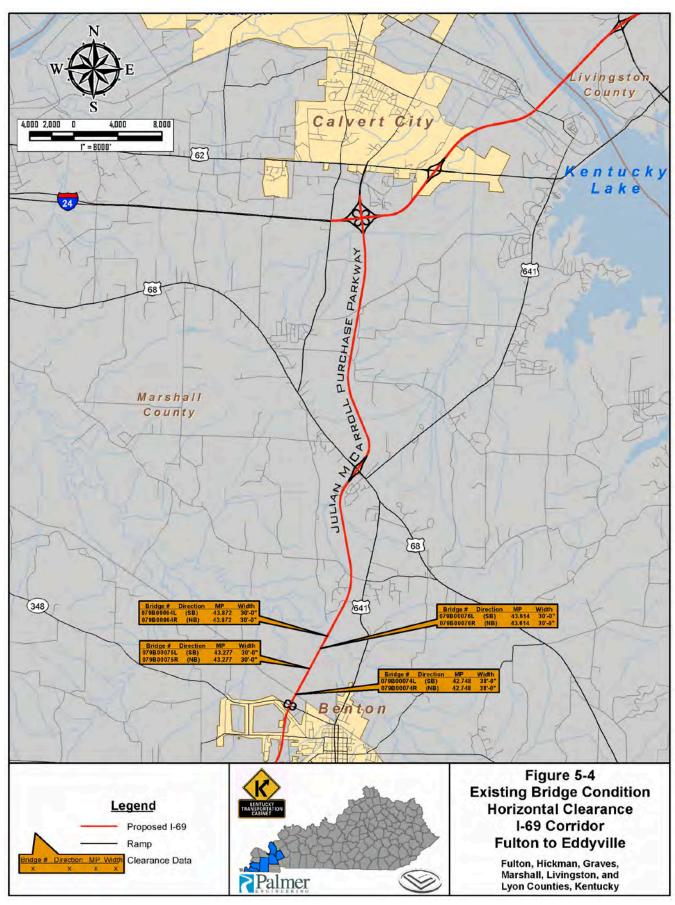
Bridge over 200' long with horizontal clearance less than 38'
One lane bridge - Mayfield Bypass Trumpet

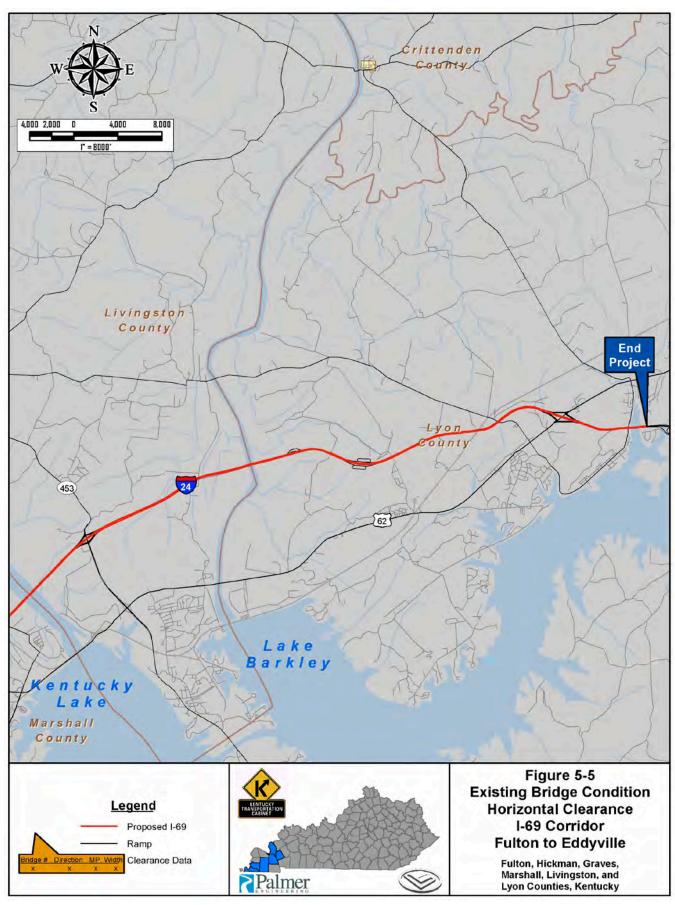
Table 5- 2 Summary of Substandard Lateral Clearances











B. Vertical Clearance of Overpasses and Sign Trusses

The vertical clearance of an overpass bridge is defined as the minimum height between the pavement and the bottom of the overpass structure and should be at least 16 feet across the entire width of the roadway, including the auxiliary lanes and the width of paved shoulder. The vertical clearance for a sign truss that crosses over the highway is minimum 17 feet for the entire width of the roadway.

The vertical clearance for the overpass bridges on the Purchase Parkway and I-24 were measured in the field to obtain the most accurate results for this study. The vertical clearance of sign trusses that cross over the Purchase Parkway were measured in the field as well. The clearance values depicted in **Table 5-3** are the minimum clearance measured by location on the roadway. Also noted are bridges that are less than 16.5 feet. The vertical clearance of these bridges will need to be monitored closely with future pavement rehabilitation.

As shown in the table, the Purchase Parkway has 4 bridges that have clearances less than 16 feet, all located in Graves County. The lowest clearances are 15.12 feet (southbound) and 15.30 feet (northbound) at the KY 80 interchange bridges. The other two bridges that do not meet minimum clearance are the KY 58 and Tater Road overpass bridges. The vertical bridge clearance information is provided in **Figure 5-6** through **Figure 5-10**.

C. Crash Worthy Pier Protection

There are currently earthen mound bridge pier protections that do not meet current standards. These protections are located at the following overpass bridge locations: MP 1.424 (US 51), MP 10.186 (KY 1763), MP 11.428 (Grissom Road), MP 12.607 (KY 944), MP 15.302 (Tater Road), MP 16.526 (KY 58), MP 17.334 (KY 1748), and MP 20.229 (Cardinal Road).



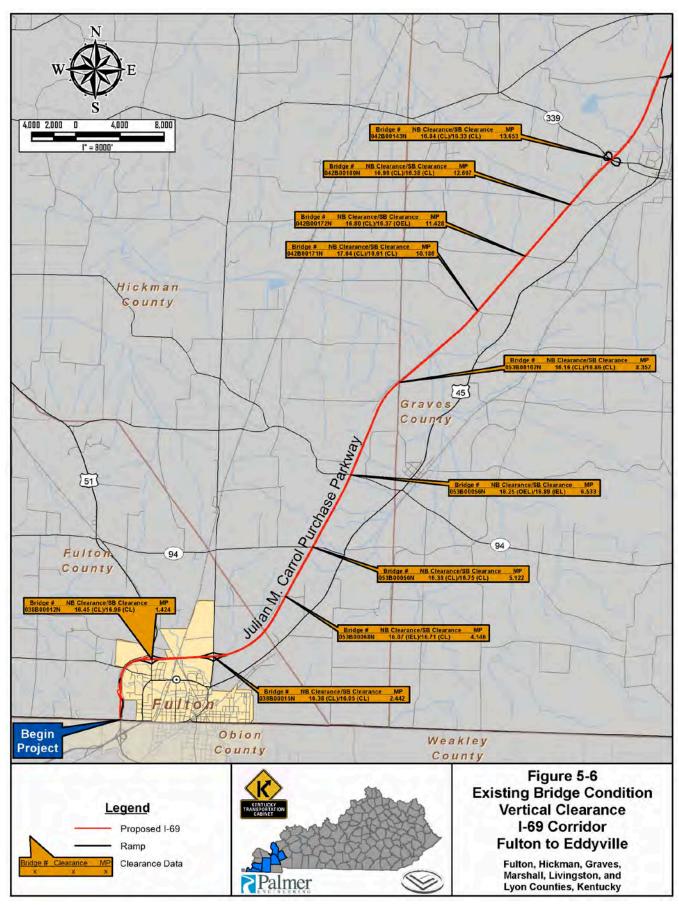
Eight overpass bridges have crash pier protection that does not meet current standard

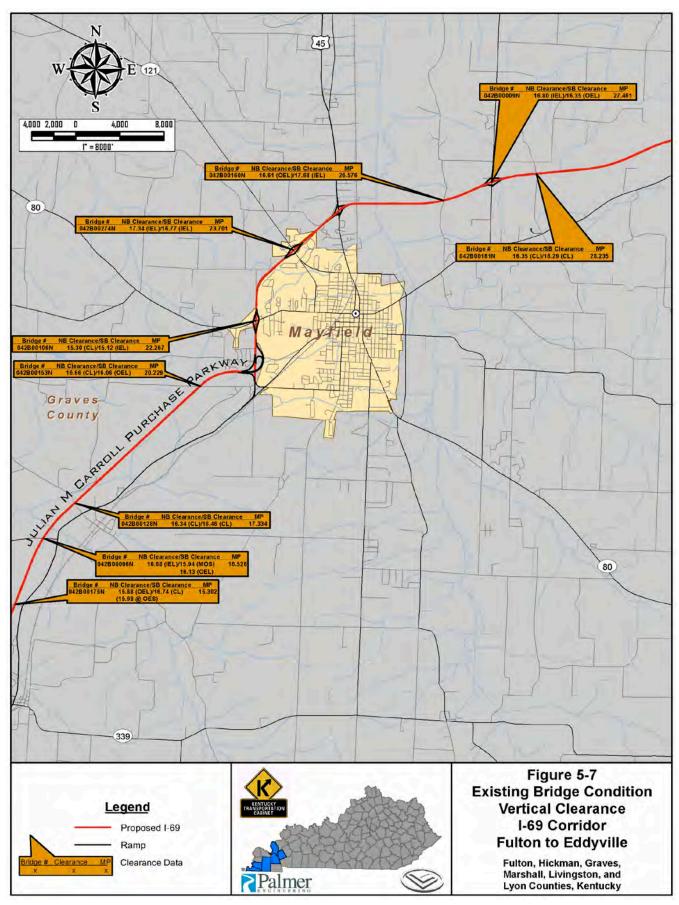
		BRIDGE		MINIMUM VERTICAL			
MP	COUNTY	NO.	FEATURES INTERSECTED	CLEARA			
			DUDOLIAGE DADIGA(A)	NB	SB		
4 404	F. Itaa	D00040	PURCHASE PARKWAY	40.45.(01.)	40.00 (01.)		
1.424	Fulton	B00012	US 51	16.45 (CL)	16.96 (CL)		
2.442	Fulton	B00015	KY 307 (Fulgham Rd)	16.38 (CL)	16.05 (CL)		
4.146	Hickman	B00068	KY 2569 (Holland Rd)	16.07 (IEL)	16.71 (CL)		
5.122	Hickman	B00050	KY 94	16.38 (CL)	16.75 (CL)		
6.533	Hickman	B00056	KY 1529	16.25 (OEL)	16.89 (IEL)		
8.352	Hickman	B00102	KY 1283	16.16 (CL)	16.86 (CL)		
10.186	Graves	B00171	KY 1763	17.04 (CL)	16.61 (CL)		
11.428	Graves	B00172	Grissom Rd	16.80 (CL)	16.37 (OEL)		
12.607	Graves	B00180	KY 944	16.96 (CL)	16.38 (CL)		
13.653	Graves	B00143	KY 339/Relocated KY 58	16.84 (CL)	16.33 (CL)		
15.302	Graves	B00175	Tater Rd	15.88 (OEL) 15.98 (OES)	16.74 (CL)		
16.526	Graves	B00096	KY 58	16.68 (IEL)	15.94 (MOS) 16.13 (OEL)		
17.334	Graves	B00128	KY 1748	16.34 (CL)	16.46 (CL)		
20.229	Graves	B00153	Cardinal Rd (Pryorsburg- Macedonia Rd)	16.66 (CL)	16.06 (OEL)		
22.267	Graves	B00106	KY 80 (Fancy Farm Rd)	15.30 (CL)	15.12 (IEL)		
23.701	Graves	B00274	KY 121	17.34 (IEL)	16.77 (IEL)		
26.576	Graves	B00160	Hopewell Rd	16.61 (OEL)	17.68 (IEL)		
27.461	Graves	B00009	KY 131	16.80 (IEL)	16.35 (OEL)		
28.235	Graves	B00161	Twin Hill Rd (Spence Chapel Rd)	16.35 (CL)	16.29 (CL)		
31.129	Graves	B00028	KY 301	16.22 (CL)	16.53 (CL)		
32.734	Graves	B00164	Panther Creek (School) Rd	16.24 (CL)	16.38 (CL)		
36.197	Marshall	B00068	KY 2603 / Vanzora Church Rd (Hale Springs Rd)	16.26 (CL)	16.54 (CL)		
37.868	Marshall	B00071	Bondurant Ln / KY 2604 (Marvin Jones Rd)	16.33 (CL)	16.40 (CL)		
40.054	Marshall	B00073	Jackson School Rd / KY 2606 (KY 299)	16.77 (CL)	16.42 (CL)		
40.809	Marshall	B00126L	US 641 SB / Benton Bypass	16.98 (OEL)	17.45 (IEL)		
40.809	Marshall	B00144R	US 641 NB / Benton Bypass	16.45 (OEL)	17.05 (IEL)		
42.017	Marshall	B00103	KY 408 / Oak Level Rd	16.89 (CL)	16.20 (CL)		
42.555	Marshall	B00102	KY 348 / Symsonia Rd	16.88 (CL)	16.43 (CL)		
45.024	Marshall	B00012	KY 795 / Scale Rd (Scale- Briensburg Rd)	16.38 (OEL)	16.88 (IEL)		
46.942	Marshall	B00001R	US 68 EB	16.29 (CL)	16.07 (CL)		
46.942	Marshall	B00001L	US 68 WB	16.84 (CL)	16.61 (CL)		
48.979	Marshall	B00050	Palma Rd (Palma-Birmingham Rd Relocation)	16.14 (OEL)	16.58 (CL)		
49.84	Marshall	B00066	KY 2595 / Lakeview Church Rd	16.67 (CL)	16.32 (CL)		
51.398 / 24.941	Marshall	B00114R	I-24 EB	18.26 (IEL)	17.25 (CL)		
51.398 / 24.941	Marshall	B00114L	I-24 WB over Pkwy	17.27 (CL)	16.27 (CL)		
MP	COUNTY	BRIDGE NO.	FEATURES INTERSECTED	MINIMUM VERTICAL CLEARANCE (ft)			
	EB WB						
			I-24				
30.696	Livingston	B00064	KY 453	19.48 (OES)	16.51 (OEL)		
35.293	Lyon	B00032	KY 6008 (Hopewell Church Rd)	16.27 (OEL)	17.46 (OEL)		
36.413	Lyon	B00033	KY 810 (Martins Chapel Rd)	16.46 (CL)	16.00 (CL)		
37.305	Lyon	B00034	KY 6010 (Poplar Creek Rd)	16.30 (OES)	16.59 (OES)		
40.744	Lyon	B00038	KY 295	16.66 (CL)	16.24 (CL)		
Bridge with Vertical Clearance less than the ASSHTO recommended minimum of 16 feet							

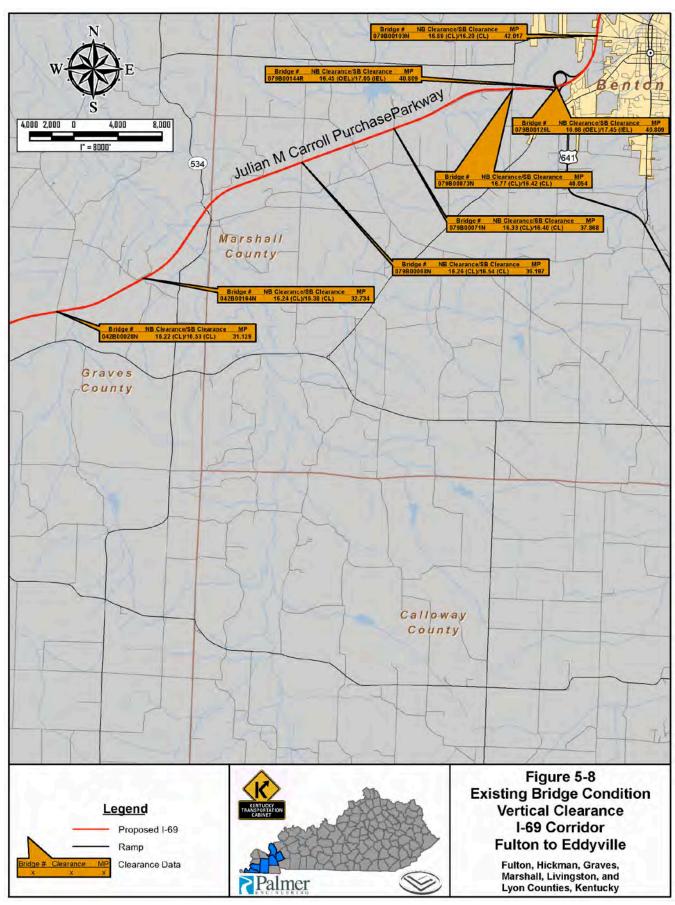
Bridge with Vertical Clearance less than the ASSHTO recommended minimum of 16 fee Bridge with Vertical Clearance less than 16.5 feet

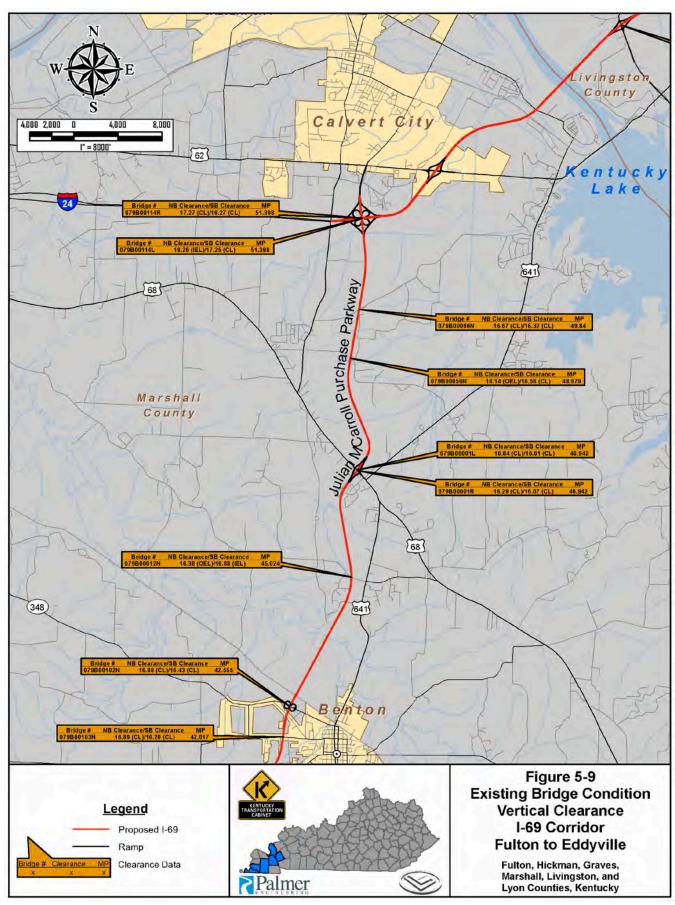
IEL- Inside Edge of Lane; CL-Center Line; OEL-Outside Edge of Lane; OES-Outside Edge of Shoulder; MOS-Middle of Outside Shoulder

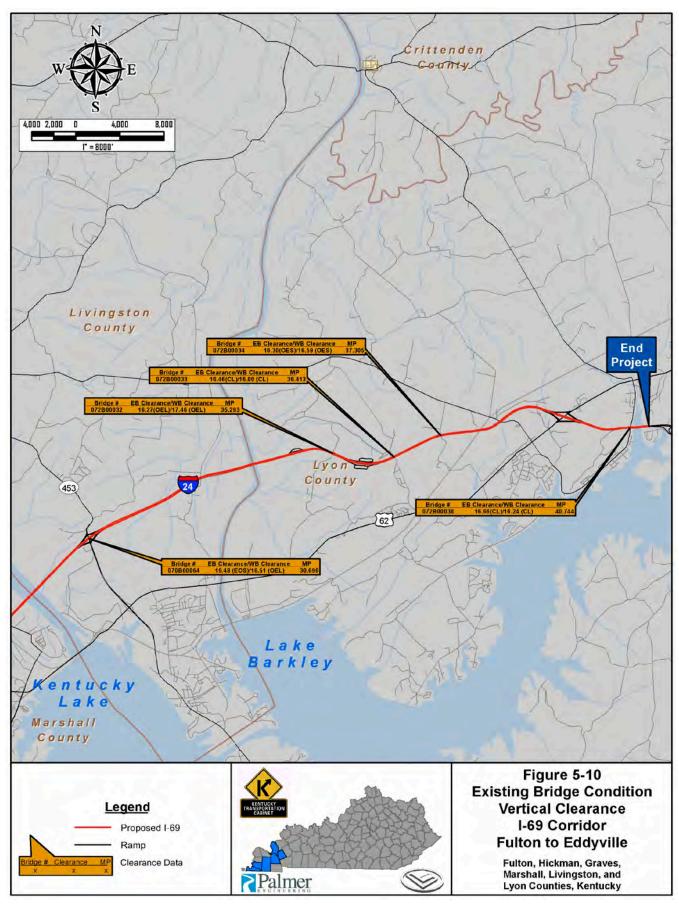
Table 5- 3 Summary of Substandard Vertical Clearances











D. Bridge Conditions

For this study the structural and functional capacity of each bridge was evaluated. The structural capacity of a bridge is determined by its sufficiency rating. A bridge that can no longer carry the vehicle weight it was originally designed to carry is classified as structurally deficient. A bridge that does not meet current geometric design guidelines, such as lane width, approach alignment, overhead clearance, etc is defined as functionally obsolete. Sufficiency and inventory ratings for bridges on the Purchase Parkway are provided in **Appendix F**. The following is a summary of the bridge sufficiency ratings on the Purchase Parkway.

- Currently, all Purchase Parkway mainline and overpass bridges have a sufficiency rating greater than 60.0.
- There are two overpass bridges at MP 46.942 in Marshall County that have a sufficiency rating of 66.2.
- The culvert at MP 38.687 in Marshall County has a sufficiency rating of 49.3, an inventory rating of HS 11.1 and an operating rating of HS 33.3
- The culverts at MP 37.135 and MP 44.587 in Marshall County have sufficiency ratings of 68.9 and 66.9, respectfully.

According to the KYTC Bridge Division, there is one bridge identified as functionally obsolete in the study area. It is a mainline bridge located in Graves County at MP 21.285. This bridge is part of the Exit 21 interchange on the Mayfield Bypass section of the Purchase Parkway. This identification is not in comparison to Interstate standards. Additional bridges can be expected to be identified as functionally obsolete when compared to interstate standards.

E. Overhead Signs

The minimum vertical clearance for an overhead sign truss is 17 feet according to current guidelines. The vertical clearances of the overhead sign trusses on the Purchase Parkway were measured in the field and none were found to be less than 17 feet. The overhead sign attached to the overpass bridge at MP 22.2 in the southbound direction is less than the 17 foot minimum. For this study, the overhead sign truss, cantilever sign trusses, and signs connected to overpass bridges were measured. The following table shows the locations and vertical clearance of overhead signs on the Purchase Parkway.

COUNTY	DIRECTION	MP	TYPE	VERTICAL CLEARANCE (ft)
Graves	NB	13.6	Bridge	18.1
Graves	NB	20	Overhead Truss	18.2
Graves	NB	20.9	Overhead Truss	18.1
Graves	NB	22	Overhead Truss	17.7
Marshall	NB	40.8	Bridge	17
Marshall	NB	42.5	Bridge	> Bottom of Bridge
Marshall	NB	51.1	Overhead Truss	18.5
Marshall	NB	51.4	Bridge	> Bottom of Bridge
Marshall	SB	51.5	Bridge	> Bottom of Bridge
Marshall	SB	42.6	Bridge	> Bottom of Bridge
Graves	SB	22.2	Bridge	15.93
Graves	SB	21.9	Overhead Truss	17.6
Graves	SB	21.6	Overhead Truss	17.9
Graves	Graves SB		Bridge	> Bottom of Bridge
Fulton	Fulton SB		Cantilever	> 20

Table 5- 4 Overhead Sign Vertical Clearance

VI. INTERCHANGES AND RAMPS

This chapter summarizes the interchanges and ramp conditions on the Purchase Parkway. There are 13 interchanges on the Purchase Parkway along the project study corridor. Similar to the mainline geometry guidelines, AASHTO has criteria for minimum standards for interchanges and ramps. These guidelines are design speed, typical sections, and horizontal and vertical alignment. This chapter addresses each of those factors along with speed-change lanes, weaving characteristics, interchange crash data, interchange spacing, control of access, and interchange configuration. **Figures 6-1 through 6-3** and **Table 6-1** summarize the comparison of the interchange and ramp conditions taken from the as-built plans with the AASHTO guidelines for the key areas for interchange design.

A. Design Speed

The AASHTO minimum design speed for directional entrance ramps and exit ramps is 40 mph. The design speed for semi-directional ramps in rural areas is 35 mph and 25 mph in urban areas. For urban and rural areas, the minimum design speed for loop ramps is 25 mph. The corresponding horizontal minimum radii for 40 mph, 35 mph, and 25 mph design speeds is 444 feet, 314 feet, 134 feet, respectively.

The design speed for most of the ramps was not available or illegible on the as-built plans.

Weigh station exit ramp at Tennessee state

B. Typical Sections

Similar to AASHTO minimum guidelines of lane widths and shoulder widths of the mainline typical section, there are also guidelines for minimum typical section for lane and

shoulder width. The following is a comparison of the existing typical section for lane and shoulder widths to the current AASHTO guidelines and a discussion of existing rolled curbs on interchange ramps.

1. Lane Widths

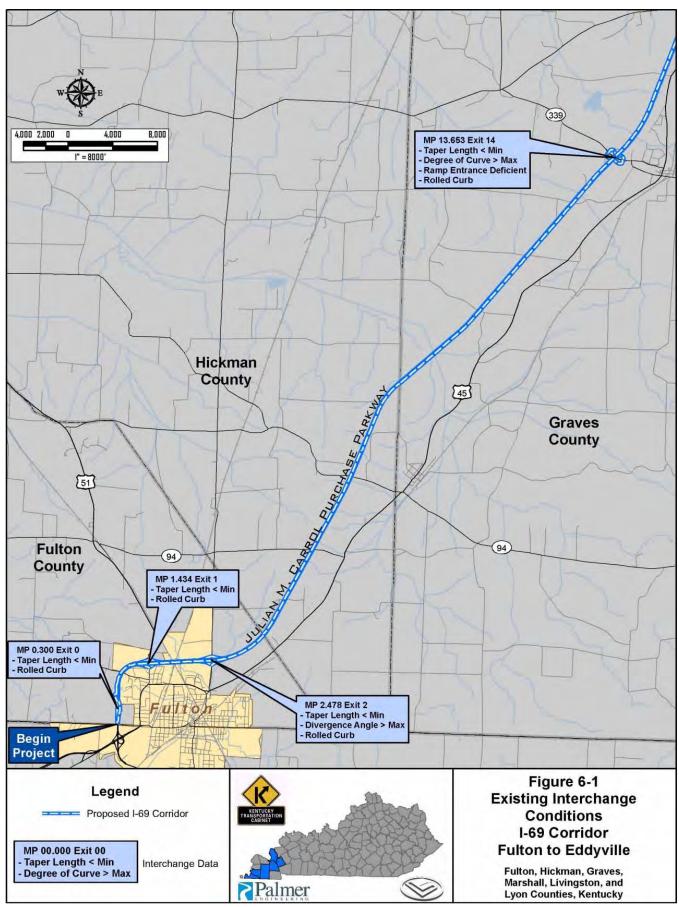
The current AASHTO minimum lane width along an interchange ramp is 15 feet. All of the interchange ramps on the Purchase Parkway meet the minimum requirement ranging in width from 15 to 18 feet. Refer to the **Table 6-1** for specific interchange ramp lane widths.

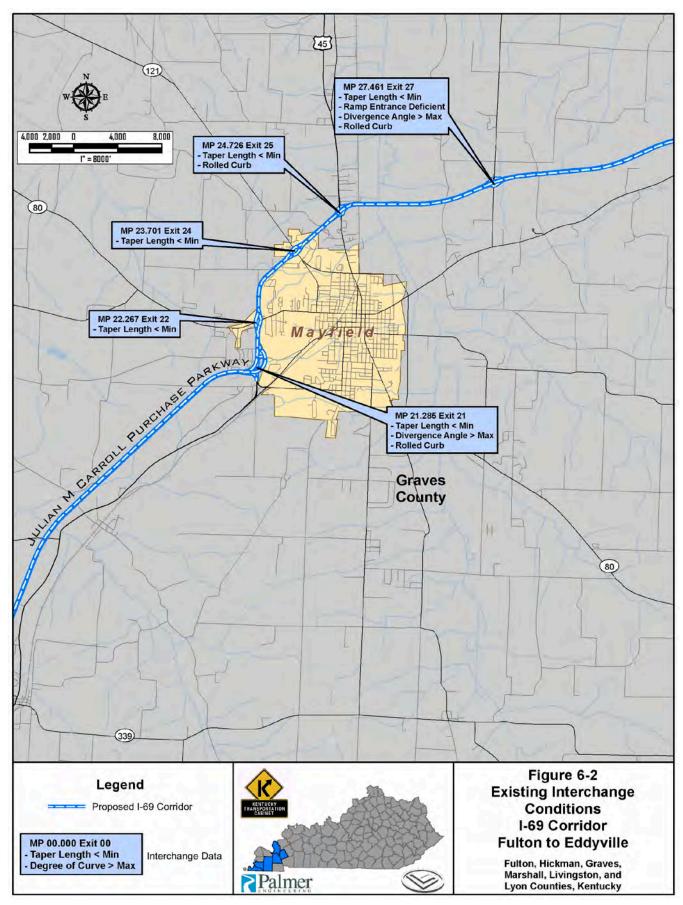
2. Shoulder Widths

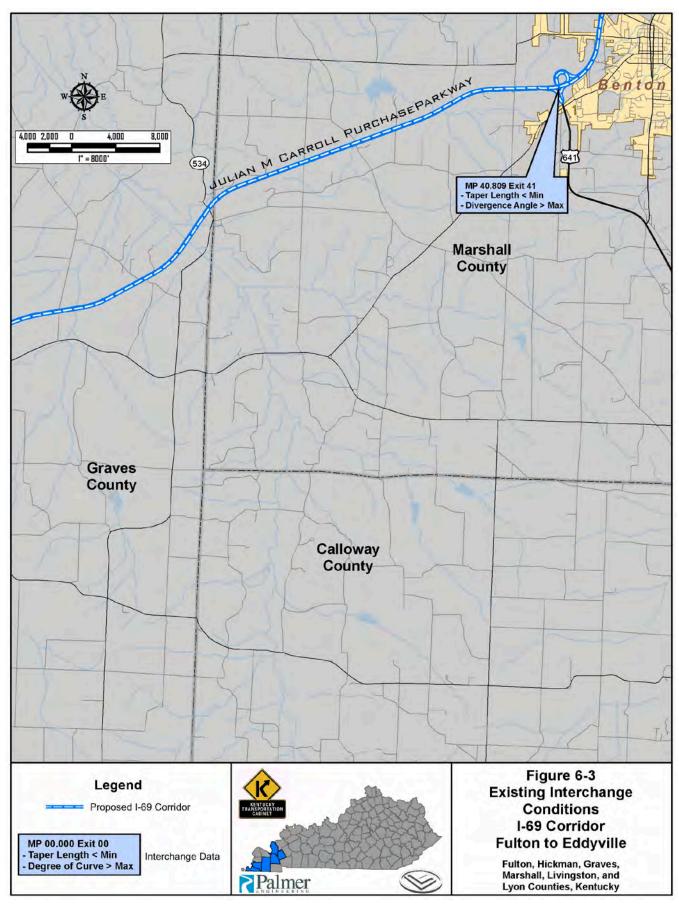
For normal one-way operation, the inside shoulder width should be 2 to 4 feet wide and paved and the outside shoulder width should be 8 to 10 feet wide and paved. The sum of the inside and outside shoulder width should not exceed 10 to 12 feet wide and paved. Much of the current interchange ramps have a rolled curb at the edge of the driving lane, therefore they do not meet AASHTO guidelines for shoulder widths. According to the as-built plans, the interchange ramps that do not have a rolled curb have an inside shoulder width of 6 feet and the outside shoulder width ranges from 6 to 10 feet.

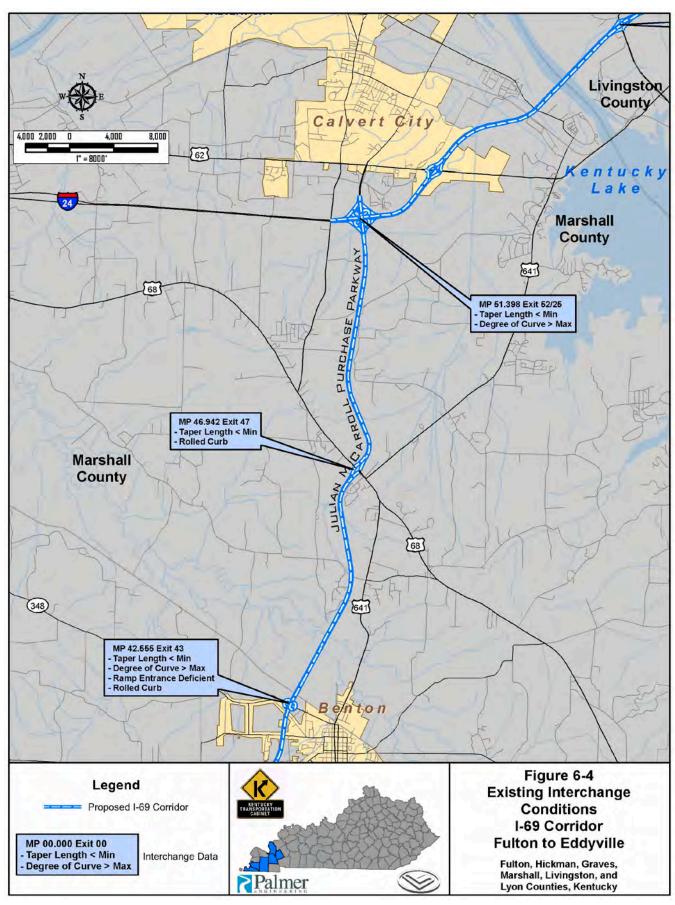
3. Rolled Curb

The current standard restricts the use of curb on mainline and ramps that are not intended for drainage purposes. The Purchase Parkway exits that have a rolled curb on the interchange ramps are Exits 0, 1, 2, 14, 21, 25, 27, 43, and 47. Refer to the following figures for the location of ramps with rolled curb that do not meet current standards.









					RAMP C	HARACTERISTICS			MEET INTER	STATE STAN	IDARDS?	
COUNTY	INTERSECTING ROUTE	EXIT NUMBER	MP	PLAN YEAR	ENTRANCE	EXIT	TYPE	WIDTH	ENTRANCE / EXIT RADIUS ¹	RAMP RADIUS	TAPER ²	REMARKS
						A (RAMP "A")	Taper	18	Yes	Yes	No	
FULTON	Frontage Road A;	0	0.300	1966	B (RAMP "B")		Taper	18	Yes	Yes	No	KY 166 and
TOLION	Frontage Road B	O	0.300	1900		C (Ramp "C")	Taper	18	Yes	No	No	WeighStation
					D (Ramp "D")		Parallel	18	Yes	No	Yes	
						A (Ramp "SW")	Taper	18	Yes		No	
FULTON	US 51	4	4 404	1966	B (Ramp "SE")		Parallel	18	Yes		No	US 51
FULTON	05 51	1	1.424	1966		C (Ramp "NE")	Taper	18	Yes		No	Diamond Interchange
					D (Ramp "NW")		Taper	18	Yes	Yes	No	
						A (Ramp "B")	Taper	18	Yes		No	
		_			B (RAMP "D")	, , ,	Parallel	18	Yes	Yes	Yes	KY 307
FULTON	KY 307	2	2.478	1966	,	C (Ramp "C")	Taper	18	Yes	Yes	No	Diamond Interchange
					D (RAMP "A")	, , ,	Parallel	18	Yes		Yes	Ī
						A (Ramp "D")	Loop	18	No	No	No	
				_	B (Ramp "C")	(2 1 /	Loop	18	No	No	No	KY 339
Graves	KY 339	14	13.653	?	(22)	C (Ramp "A")	Loop	18	No	No	No	(Previous Toll Plaza)
					D (RAMP "B")	, , ,	Loop	18	No	No	No	1
						A (RAMP "D")	Taper	18		Yes	Yes	
Graves	MAYFIELD	21	21.285	1966	C (RAMP "C")	71 (10 tivii 2)	Taper	18	Yes	100	No	US 45
	BYPASS				D (RAMP "B")		Loop	18	Yes		No	Trumpet
					/	A (RAMP 1)	N/A	16			N/A	
					B (RAMP 4)	A (IVAIVII 1)	Taper	16	Yes		No	KY 80
Graves	KY 80	22	22.267	1961	D (IVAIVII 4)	C (RAMP 3)	Taper	16	Yes		No	Diamond Interchange
					D (RAMP 2)	O (TO-AWII 3)	Taper	16	Yes		No	1
			 		_ (. c 2)	A (DAMD "A")	·	18			No	
					B (RAMP "C")	A (RAMP "A")	Taper Parallel	18	Yes Yes	Yes	Yes	KY 121
Graves	KY 121	24	23.701	2000	D (KAIVIP C")	C (RAMP "D")	Taper	18 18	Yes	res	Y es No	Diamond Interchange
					D (RAMP "B")	C (NAIVIE D)	Taper	18	Yes	Yes	Yes	go
			l 		D (IVAINI D)	A (DAMP IICII)		-	100			
]	D (DAME IICII)	A (RAMP "C")	Taper	18	Y	Yes	No	110.45
Graves	US 45	25	24.726	1966	B (RAMP "D")	C (DAMP IIDII)	Parallel	18 18	Yes	Yes	Yes No	US 45 Diamond Interchange
					D (RAMP "A")	C (RAMP "B")	Taper	18 18	Tangent Yes	Yes	No Yes	Diamond interchange
			<u> </u>		D (RAIVIP A")		Taper	-		res		
						A (Ramp "SW")	Taper	15	No	-	No	
Graves	KY 131	27	27.461	1966/1978(?)	B (Ramp "SE")	0 (0	Parallel	18	Yes		Yes	KY 131 Diamond Interchange
]	D (D	C (Ramp "NE")	Taper	18	No		No	Diamond interchange
			<u> </u>	ļ	D (Ramp "NW")		Parallel	15	No	1	Yes	ļ

Table 6-1 Interchange Geometrics for I-69

					RAMP CI	HARACTERISTICS	;		MEET INTERS	STATE STAN	IDARDS?	
COUNTY	INTERSECTING ROUTE	EXIT NUMBER	MP	PLAN YEAR	ENTRANCE	EXIT	ТҮРЕ	WIDTH	ENTRANCE / EXIT RADIUS ¹	RAMP RADIUS	TAPER ²	REMARKS
						A (Ramp "B")	Taper	15	Yes		No	
Marshall	US 641	41	40.809	1998	B (Ramp "A")		Parallel	18	Yes		Yes	US 641
IviaiSilali	03 041	41	40.609	1990		C (Ramp "D")	Taper	18	Yes	Yes	No	Trumpet
					D (Loop "D")		Loop	15	Yes	Yes	Yes	
						A (Ramp "D")	Loop	18	No	No	No	
Manaball	KY 348	43	40.555	1966	B (RAMP "B")		Loop	18	No	No	No	KY 348
Marshall	K Y 348	43	42.555	1966		C (Ramp "A")	Loop	18	No	No	No	(Previous Toll Plaza)
					D (Ramp "C")		Loop	18	No	No	No	
						A (Ramp "B")	Taper	18		No	No	
	110.00	47	40.040	4000	B (RAMP "D")		Parallel	18	no dwg ava	ilable	Yes	US 68
Marshall	US 68	47	46.942	1966		C (Ramp "C")	Taper	18	no dwg ava	ilable	No	Diamond Interchange
					D (RAMP "A")		Parallel	18	Yes		Yes	
						A1 (Ramp "C")	Taper	18	No	Yes	No	
						A2 (Ramp "H")	Loop	18			N/A	
					B1 (Ramp "G")		Parallel	18	Yes	Yes	Yes	
Marshall	I-24	52	51.398	1966	B2 (Ramp "F")		Loop	18		Yes	N/A	I-24 Interchange Full Clover Leaf
iviaistiali	I-2 4	52	31.398	1900		C1 (Ramp "E")	Taper	18	Yes	Yes	No	Interhenage
						C2 (Ramp "B")	Loop	18			N/A	onago
					D1 (Ramp "A")		Parallel	18	Yes	Yes	Yes	
					D2 (Ramp "D")		Loop	18			N/A	

Based on design speed and superelevation provided. Design speed calculated from current AASHTO 10% superelevation table. Reference Appendix F for design speed values.

Table 6-1 Interchange Geometrics for I-69 (continued)

C. Alignment Geometry

1. Horizontal Alignment

The minimum horizontal radius for a directional ramp in rural and urban areas is 444 feet. The minimum horizontal radius for rural and urban loop ramps is 134 feet (25 mph design speed). For rural areas, the minimum horizontal radius for a ramp is 314 feet (35 mph design speed) and 134 feet (25 mph design speed) in urban areas. For the interchanges on the Purchase Parkway, minimum ramp and loop radius are met for all interchanges except for Exit 14 (MP 13.653) in Graves County. Exit 14 is previous toll booth interchange. Refer to **Appendix F** for interchange data.

2. Superelevation Rate

From review of the as-built plans, the superelevation for the ramps, if provided, appear to meet the criterion for a maximum superelevation rate of 10%. Refer to **Appendix F** superelevation data collected.

3. Vertical Alignment

a. Vertical Grade

AASHTO guidelines designate a maximum vertical grade from 5% to 7% for all ramp types in both rural and urban areas. Of the legible information provided on the as-built plans, this minimum recommendation is met for all ramps on the Purchase Parkway. The US 641 interchange (Exit 41) in Marshall County is the only interchange that has a vertical grade greater than 5%. The loop ramp at this interchange has a vertical grade of 6% and -6%. The information provided on the as-built plans is located in **Appendix F**.

b. Vertical Length of Curve

For this study, the minimum length of curve was calculated on the vertical grades of the approaching ramp alignment and the recommended rate of vertical curvature for the minimum design speed. All of the analyses were completed for the entire length of the ramp. The following list provides the locations, actual vertical length of curve, and calculated minimum vertical length of curve of those that do not meet the criteria. The following vertical curves are located at the end of the interchange ramps and would be approaching stopping condition. This data is provided in **Appendix F.**

- Fulton County Exit 1 Ramp NE- Actual 150 feet, calculated minimum 151 feet;
- Fulton County Exit 1 Ramp SE Actual 250 feet, calculated minimum 257 feet;
- Graves County Exit 22 Ramp 1 Actual 150 feet, calculated minimum 183 feet.

c. Stopping Sight Distance

Stopping sight distance was evaluated on the vertical curvature for the ramps. The minimum stopping sight distance with the corresponding ramp type minimum design speed was compared to the calculated stopping sight distance. There are two vertical curves that do not provide the minimum stopping sight distance calculated based on the minimum ramp design speed. The following list provides the locations, actual stopping sight distance, and calculated stopping sight distance. These vertical curves are located at the end of the interchange ramps and travel would be approaching stopping condition. This data is provided in **Appendix F**.

- Fulton County Exit 1 Ramp SE Actual 245 feet, calculated minimum 250 feet;
- Graves County Exit 22 Ramp 1 Actual 246 feet, calculated minimum 250 feet.

4. Divergence Angle

The recommended divergence angle of the alignment break for a taper exit per AASHTO is two to five degrees. The divergence angle was not provided or was illegible on the as-built plans. Aerial mapping was used to estimate the existing divergence angle for the Purchase Parkway

interchange exit ramps. The following exit ramps had divergence angle estimates exceeding 5 degrees:

- Exit 2 (MP 2.478) KY 307, Northbound and Southbound
- Exit 14 (MP 13.653), KY 339, Northbound and Southbound
- Exit 21 (MP21.285) US 45 Southbound;
- Exit 27 (MP27.461) KY 131, Southbound;
- Exit 41 (MP 40.809) US 641, Southbound.

D. Speed-Change Lanes and Weaving Characteristics

Speed-change lanes and traffic weaving situations may be the two most important factors affecting safety at interchanges. The following is summary of the geometry of the as-built ramp configurations when compared to the current AASHTO guidelines for speed-change lanes and designs for traffic weaving.

1. Speed-Change Lanes

Speed-change lane design for an entrance and exit ramp is either a parallel type or taper type. The recommended taper rate for a taper type entrance is 50:1 between the outer edge of the acceleration lane and the edge of the through traffic lane. The entrance length of a ramp for a parallel type entrance is dependent on the mainline design speed. Current practice for entrance ramp speed is 70% of the mainline design speed. The rural interstate design speed of 70 mph results in an entrance ramp speed of 49 mph and 35 mph for an urban interstate design speed of 50 mph. However, it is recommended to use a taper type entrance if the acceleration length is greater than 1300 feet. When a parallel entrance type is used, the taper length rate should be 25:1 from the downstream taper to the mainline.

An exit ramp can have a parallel or taper type configuration. The taper type ramp alignment is generally designed with an alignment break at the outer edge of the pavement. The minimum divergence angle of the taper type alignment break is two to five degrees. The exit parallel type begins with a taper rate of 20:1 and the deceleration length of the parallel lane is dependent on the design speed of the mainline roadway and ramp design speeds. Exiting traffic can be assumed to be traveling 70% of the mainline design speed.

The design speed for entrance and exit ramp curves was calculated based on the superelevation provided in the as-built plans. Most of the interchange ramps have a superelevation greater than 8% but less than 10%, therefore the current AASHTO 10% maximum superelevation tables were referenced for calculating the design speed. In addition to the KY 339 (Exit 14) and KY 348 (Exit 43) interchanges, the KY 131 (Exit 27) interchange does not meet the recommended ramp design speed. Data is provided in **Appendix F**.

Since the original construction, some of the interchange ramps on the Purchase Parkway have been improved. However, a majority of the existing ramps on the parkway do not meet the minimum guideline of 50:1 entrance tapers, and existing ramps have exit tapers less than the minimum 20:1 taper. According to guidelines from AASHTO, the minimum deceleration length for exit ramps to a stop condition is 615 feet. Exit 0 along with all diamond interchange exit ramps on the Purchase Parkway meet the minimum deceleration length.

For this study, a minimum of 413 feet of taper length for an exit taper ramp was used for comparison. This length comes from using the maximum divergence



Exit 14 – One of two previous toll booth plazas on the Purchase Parkway.

angle of 5 degrees and 36 feet of ramp separation from the mainline.

AASHTO recommends a 50:1 - 70:1 uniform taper for the entrance taper ramps. The entrance taper length used was 900 feet for an entrance taper ramp.

2. Weaving Characteristics

There are 2 interchanges that do not meet the 1,000 foot minimum weaving length distance recommended for a service-to-service interchange. These interchanges are located at Exit 14 – Wingo / Clinton (MP 13.653) in Graves County and Exit 43 Benton / Symsonia (MP 42.555) in Marshall County. These two interchanges were initially designed for toll collection stations. The full cloverleaf configurations included short weaving sections of approximately 300 feet. The toll plazas have since been suspended and the interchange at Exit 43 currently is being red-designed.

Currently, the Purchase Parkway interchange with I-24 is a full cloverleaf interchange that has a weaving distance of approximately 480 feet on the Purchase Parkway and approximately 430 feet on I-24. A weaving analysis was not conducted on this interchange because it does not meet the recommended systems interchange configuration.

In addition to the interchanges mentioned, there is another weaving segment on the Purchase Parkway, which is located between the Mayfield Bypass (US 45) trumpet interchange at Exit 21 and KY 80 interchange at Exit 22. An auxiliary lane is provided for the northbound Purchase Parkway to Exit 22. Traffic counts were provided by KYTC and measurements were taken in the field for the weaving analysis. Highway Capacity Software (HCS) was used for the analysis. A 2.5% annual growth rate was applied to the 2010 DHV traffic counts, which is the annual growth rate used to calculate the 2040 traffic with I-69 designation. These traffic volumes and existing weave distance resulted in a LOS of B (15.02 passenger cars per mile per lane). See **Appendix F** for HCS output.

E. Interchange Crash Data

To further analyze the interchange operations, crashes at interchanges were collected and analyzed. **Table 6-2** shows the types of crashes occurring within a 0.1 mile section on either side of the intersecting route at each interchange.

Exit	MP	County	Intersecting Route	Ramp	amp Rear Fixe		Animal	Side Swipe	Ran Off Road	Other	Total	Fatalities
Purcl	hase Par	kway		,			,					
0	0.30	Fulton	Frontage Roads	0	1	1	2	0	1	0	5	0
1	1.42	Fulton	US 51	4	0	0	0	0	0	0	4	0
2	2.48	Fulton	KY 307	1	0	0	3	0	0	0	4	0
14	13.65	Graves	KY 339	2	1	0	0	1	2	0	6	0
21	21.29	Graves	US 45	4	2	14	1	2	1	2	26	1
22	22.27	Graves	KY 80	3	2	4	0	0	2	0	11	0
24	23.70	Graves	KY 121	3	2	1	0	0	2	1	9	0
25	24.73	Graves	US 45	4	0	5	2	3	4	0	18	0
27	27.46	Graves	KY 131	3	0	1	1	0	4	0	9	0
41	40.81	Marshall	US 641	13	0	0	2	1	2	0	18	0
43	42.56	Marshall	KY 348	31	3	7	0	5	1	1	48	0
47	46.94	Marshall	US 68	23	0	6	4	3	3	1	40	0
52	51.40	Marshall	I-24	55	1	3	2	4	5	4	74	0
Inters	state 24											
25	24.94	Marshall	Purchase Pkwy	55	8	9	3	9	4	9	97	1
27	26.57	Marshall	US 62	14	6	7	1	5	2	4	39	1
31	30.73	Livingston	KY 453	1	3	1	3	3	3	9	23	2
40	39.52	Lyon	US 62	4	3	3	5	1	1	3	20	0
42	41.65	Lyon	Western KY Pkwy	23	3	1	2	5	4	3	41	1

¹ Number of crashes in period studied (2005-2009), within 0.1 mile on either side of intersection route.

Table 6-2 Interchange Crash Data

Along the Purchase Parkway, there are three interchanges that fall within a high crash segment, previously mentioned in **Sections 4** and **5** of **Chapter III.**

- Exit 27, KY 131, in Graves County had nine crashes, of which four were ramp related;
- Exit 47, US 641, in Marshall County had 18 crashes. A majority of these crashes (13) were ramp related;
- Exit 43, KY 348 (previous toll plaza), in Marshall County had 48 crashes. 65% (31 of 48) were ramp related collisions.

Along I-24, there is one interchange that falls within a high crash segment.

 Exit 25, Purchase Parkway, in Marshall County had 97 crashes, of which 55 were ramp related.

The 171 crashes that occurred at the I-24 and Purchase Parkway Interchange accounted for 19% of all crashes during the study period.

F. Interchange Spacing

The current minimum spacing between interchanges on an interstate for rural areas is three miles and one mile for urban areas. This spacing is measured between the centerline of intersecting routes. The three interchanges at Fulton are spaced closer than the rural interstate minimum. These three interchange exits are spaced within three miles of each other. The interchange at Exit 0 is an unconventional interchange that is too close to Exit 1 (MP 1.424). The interchanges at Exit 1(MP 1.424) and Exit 2 (MP 2.442) are spaced closer than the minimum for rural areas. The two interchanges at Benton are within the rural three-mile spacing limit. There is less than two miles between Exit 41 (MP 40.809) and Exit 43 (MP 42.555).

G. Interchange Control of Access

Interchange control of access is the distance measured from the ramp termini to the adjacent commercial or residential access. The measurement of control of access according to KYTC standards is from the end of the ramp termini radius or taper to the centerline of the adjacent commercial or residential access. The recommended interchange control of access for an urban area is 100 feet and 300 feet for rural areas. The following table illustrates the interchange control of access distance.

EXIT	RURAL/ URBAN	INTERSECTION ROUTE	QUADRANT	DISTANCE (FT)			
14	RURAL	KY 339	Northwest	115			
14	KUKAL	KT 339	Southwest	105			
			Northeast	65			
22	URBAN	KY 80	Northwest	85			
			Southwest	60			
			Southeast	270			
27	RURAL	KY 131	Northwest	275			
			Southwest	60			
			Southeast	260			
47	RURAL	US 68	Northeast 0				
47	NORAL	03 00	Northwest	108			
			Southwest	40			

Table 6-3 Interchange Control of Access

H. Interchange Configuration

Currently the Purchase Parkway has several interchanges that are not inconsistent with common practice for interstate interchange configuration.

1. Systems Interchanges

Currently, the Purchase Parkway and I-24 interchange is a full clover leaf (graphic below), which meets the recommendation for the two fully controlled access facilities interchange. However, with the implementation of converting the Purchase Parkway to I-69, the clover leaf is inconsistent with AASHTO recommendations for a systems interchange. Currently, the ramps and loop ramps within the interchange are also one lane.



Exit 52 - I-24 / Purchase Parkway Interchange

2. Service Interchanges

Two service interchanges are inconsistent with AASHTO recommendations are the previous toll plazas located Exit 14 and Exit 43 (graphics below). As previously mentioned, both of these interchanges have less than the minimum weaving distance. At the date of this report, Exit 43 is in the design phase to be improved and meet interstate standards.



Exit 14 - Purchase Parkway / KY 339 Previous Toll Plaza



Exit 43 - Purchase Parkway / KY 348 Previous Toll Plaza

Another service interchange that is inconsistent with AASHTO recommended interchange configuration is Exit 0 in Fulton County at the Tennessee state line (graphic below). This interchange includes a weigh station for the northbound Purchase Parkway. The weigh station

provides access to Eastwood Drive. Southbound Purchase Parkway exit and entrance ramps connect to KY 166. The KYTC has made overture to the TDOT regarding this interchange, the existing weigh station, and overall connectivity for I-69 between Kentucky and Tennessee. Thus far there has been no coordination with TDOT.



Exit 0 - Purchase Parkway / Weigh Station / Eastwood Dr / KY 166

The interchange at Exit 21 in Mayfield does not provide continuity for the Purchase Parkway (graphic below). The modified trumpet interchange is configured for the Mayfield Bypass (US 45) as the major route and the Purchase Parkway as the minor route. The northbound Purchase Parkway merges from two lanes to one lane, which then travels over US 45 and ultimately merges into the Mayfield Bypass. The southbound Purchase Parkway exits to the right via a one-lane ramp, while southbound US 45 continues straight.



Exit 21 – Purchase Parkway / Mayfield Bypass Interchange

VII. KEY FINDINGS OF EXISTING CONDITIONS OVERVIEW

Currently, the Purchase Parkway operates similarly to an interstate highway. As discussed in previous chapters, in some cases the Purchase Parkway lacks geometric compliance with current AASHTO guidelines. These AASHTO minimum guidelines are provided in **Table 7-1**. The Purchase Parkway provides the basic geometric characteristics of an interstate highway, such as full control of access, two travels lanes in each direction, 12 foot lanes, 10 foot outside paved shoulders, 36 foot rural medians, 70 mph rural design speed, and 50 mph urban design speed. However, the Purchase Parkway lacks compliance with the dimensions of other design features. **Figure 7-1** through **7-5** summarizes the deficiency locations for the project corridor. Each deficiency labeled on the figures is described in **Table 7-2** and **Table 7-3** in more detail. In addition to those labeled, the inside shoulder for the Purchase Parkway is deficient. The Purchase Parkway has a three foot paved inside shoulder, with the exception of the Mayfield Bypass, which has no inside paved shoulder.

Area Type		Rural			Urban		U	rban/Rural	
Design Element	Mainline	Ramps	Loops	Mainline	Ramps	Loops	Directional	Entrance	Exit
Design Speed (MPH) (507,825,826) [2]	70	35	25	50	25	25	40		
Level of Service (504, 838) [3]		С			D			\setminus	/
Driving Lane Width (504,838) [3]	12'	15'	15'	12'	15'	15'			
Inside Shoulder Width (505,510,513,838) [3]									
4-lane freeway & ramps				4'				\setminus	
6-lane, Truck DDHGV<=250	4'	2'-4'	2'-4'	10'	2'-4'	2'-4'	1'-6'		/
6-lane, Truck DDHGV>250				12'					
Outside Shoulder Width (505,838) [3]									
Truck DDHV <= 250	10'	8'-10'	8'-10'	10'	8'-10'	8'-10'	8'-10'	\setminus	\setminus
Truck DDHV > 250	12'	0-10	0-10	12'	0-10	0-10	6-10		/
Median Width (509) [4] 1	36'		\setminus	10'				\setminus	\setminus
Over Freeway Vertical Bridge Clearance (506,763)					16'-00"				
Over Freeway Vertical Sign Truss Clearance (507)					17-'00"				
Bridge Width (Horizonatal) ADT>2000			Traveled	Lanes + Sh	noulders (ap	proach rao	dway width)		
Bridge Width (Horizonatal), Length > 200' ²				Traveled	Lanes + 3.	5' each side)		
Design ADT (vehicles per day)	> 6,000	750-	1,500	> 6,000	750-	1,500		\setminus	
Clear Zone (Fill Slope 1V:4H or flatter) 3	30'-46'	10'	-14'	20'-28'	10'	-14'		\setminus	\setminus
Clear Zone (Cut Slope 1V:3H or flatter) 3	22'-30'	10'	-12'	14'-22'	10'	-12'		\setminus	
Superelevation (505) 4				+/-8%					/
Horizontal Curvature Minimum Radius (8% max SE) (170)	1810'	314'	134'	758'	134'	134'	444'	\setminus	
Minimum Runoff (8% max SE) (181)	240'	155'	137'	192'	137'	137'	166'		\setminus
Minimum Runout (8% max SE) (181)	60'	39'	34'	48'	34'	34'	41'	\setminus	\setminus
Maximum Grade (506,829)	4%	5%-7%	5%-7%	5%	5%-7%	5%-7%	4%-6%		\setminus
Stopping Sight Distance (112)	730'	250'	155'	425'	155'	155'	305'		
Taper Ratio (845)								50:1	
Divergence Angle (849)									2°-5°

Note: Page number references from AASHTO's A Policy on Geometric Design of Highways and Streets, 2004 are provided in parenthesis. Page number reference from AASHTO's A Policy on Design Standards Interstate System, 2005 are provided in brackets.

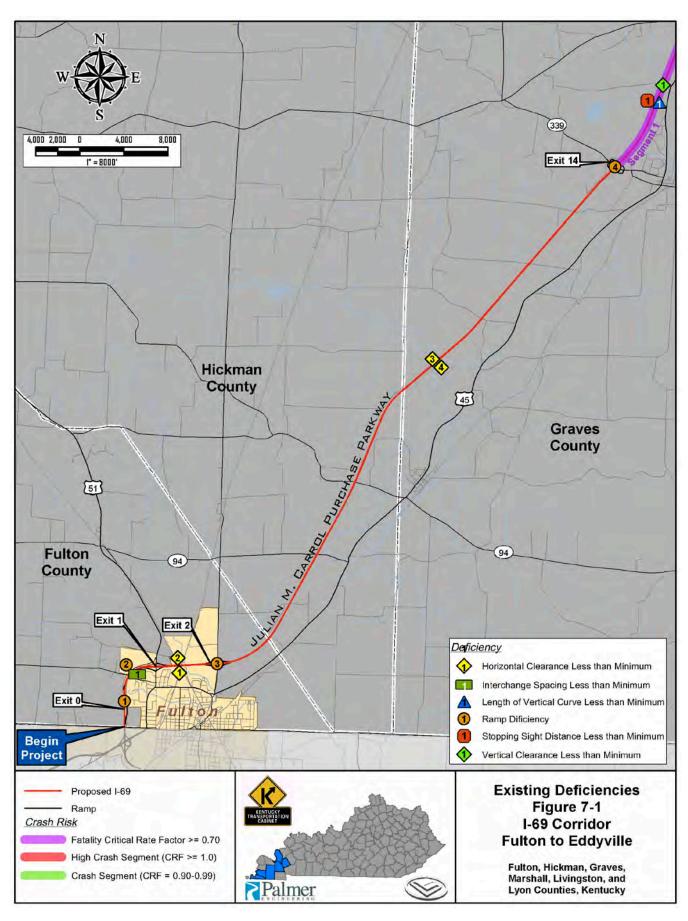
Table 7-1 AASHTO Minimum Guidelines

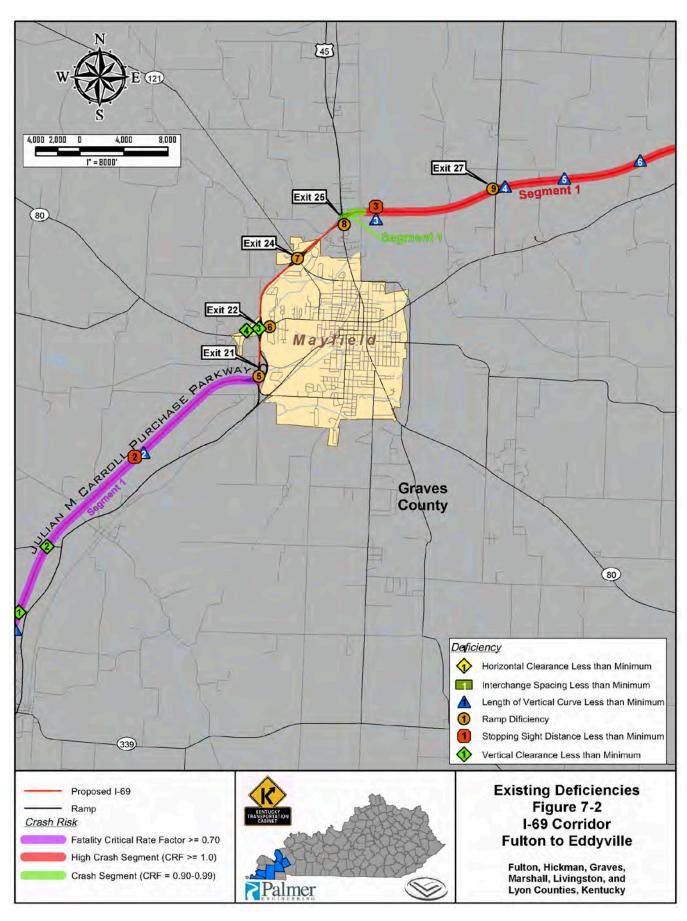
¹ AASHTO's A Policy on Design Standards Interstate System, 2005 states 36' minimum depressed median in rual areas. AASHTO's A Policy on Geometric Design of

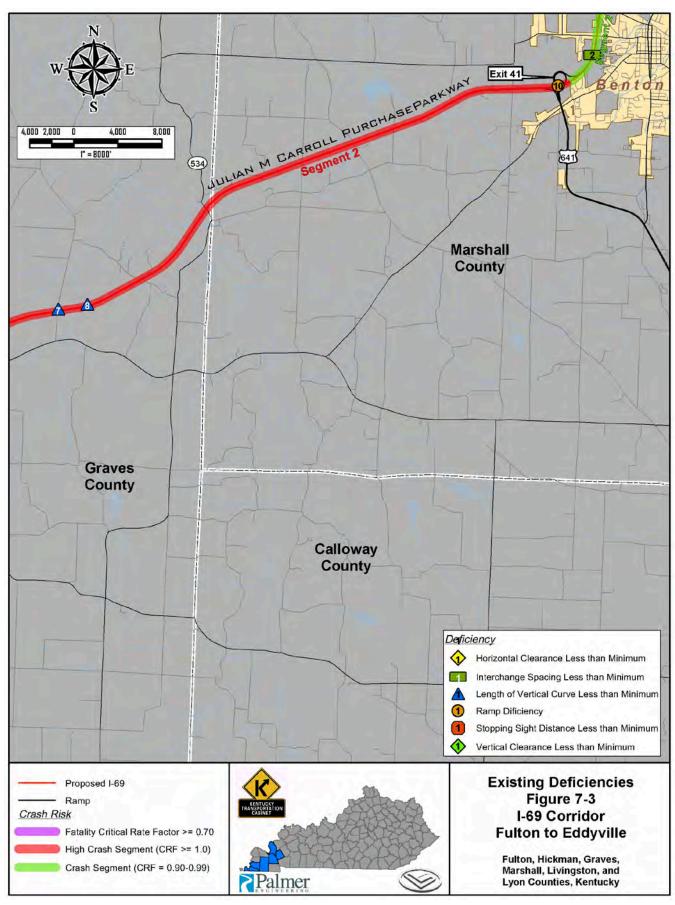
²This item is referenced in the AASHTO A Policy on Design Standards Interstate System, 2005

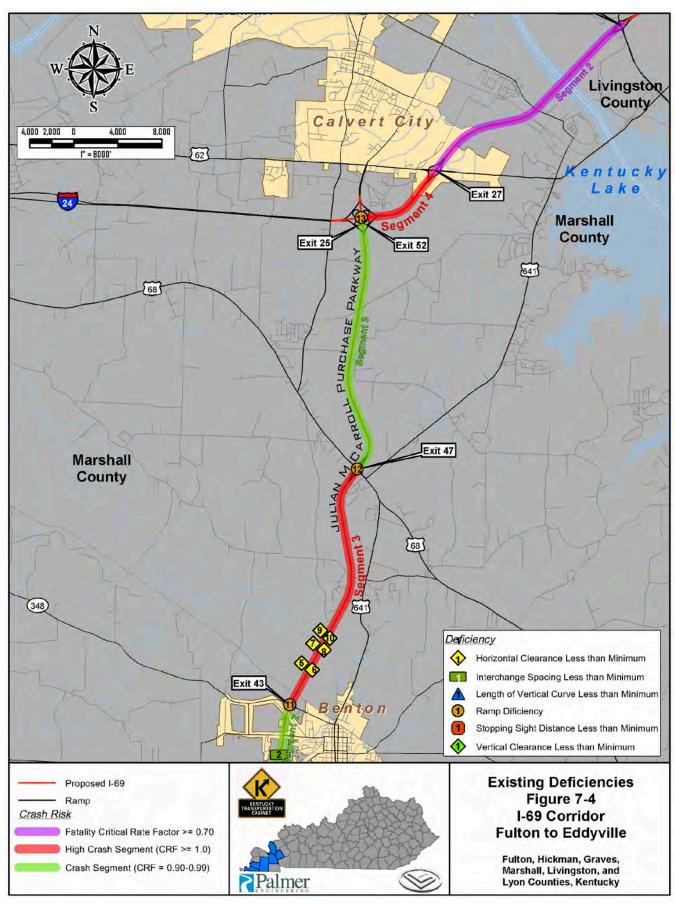
³ Information on clear zones is provided in AASHTO's *Roadside Design Guide Current Edition* .

⁴ Common KYTC Practice is 8% maximum superelevation. KYTC has used 10% maximum superelevation on past projects including the Purchase Parkway.









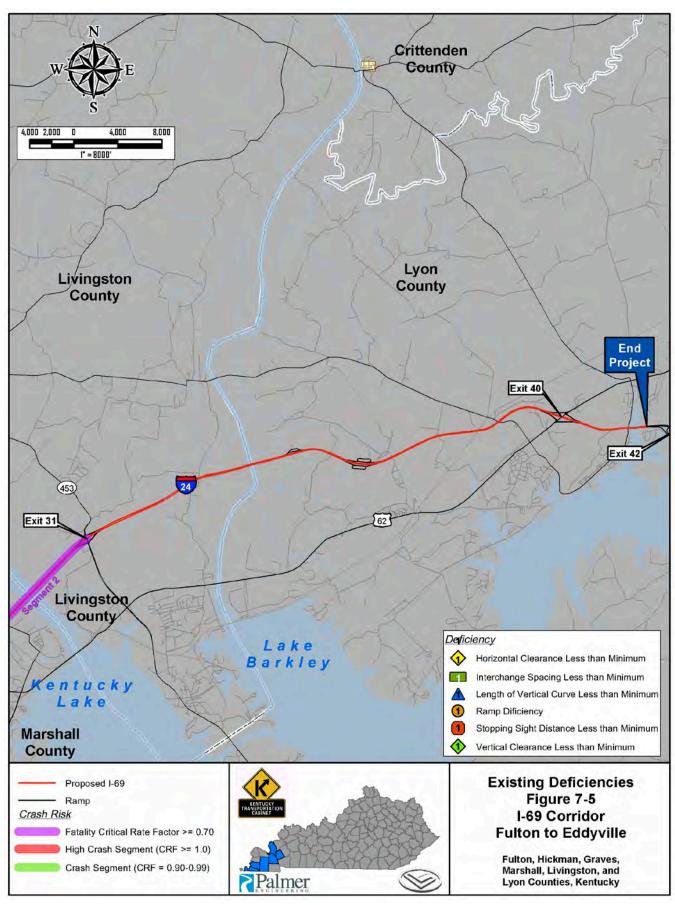


Table 7-2 Deficiencies Summary for the Purchase Parkway

Table 7-2 Defice	ciencies Sumr	mary for the Purchase Parkway
Deficiency Type	Milepoint	Deficiency Description
Purchase Parkway	y - Fulton/Hickma	n County
1	Exit 0	Taper Length < Min; Rolled Curb
1	MP 1.0	Interchange Spacing less than 3 mile minimum
2	Exit 1	Taper Length < Min; Rolled Curb
3	Exit 2	Taper Length < Min; Divergence Angle > Max; Rolled Curb
1	1.781	Horizontal Clearance = 30' (Note bridge is over 200' long)
2	1.781	Horizontal Clearance = 30' (Note bridge is over 200' long)
Purchase Parkway	y - Graves County	
3	9.082	Horizontal Clearance = 30' (Note bridge is over 200' long)
4	9.082	Horizontal Clearance = 30' (Note bridge is over 200' long)
1	13.645 - 21.305	Fatality CRF = 0.75 (CRF >=0.70)
4	Exit 14 MP 13.653	Taper Length < Min; Degree of Curve > Max; Ramp Entrance/Exit Deficient; Rolled Curb; Interchange control of access less than 300' minimum
	14.965	Length of Vertical Curve = 500' (696' calcuated minimum)
1	14.965	Stopping Sight Distance = 554' (730' minimum)
1	15.302	Vertical clearance = 15.88' (16' minimum)
2	16.526	Vertical clearance = 15.94' (16' minimum)
2	18.727	Length of Vertical = 600' (624' calculated minimum)
2	18.727	Stopping Sight Distance = 727' (730' minimum)
5	Exit 21 MP 21.285	Taper Length < Min; Divergence Angle > Max; Rolled Curb
6	Exit 22 MP 22.267	Taper Length < Min; Interchange control of access less than 100' minimum
3	22.267	Vertical clearance = 15.30' (16' minimum)
4	22.267	Vertical clearance = 15.12' (16' minimum)
7	Exit 24 MP 23.701	Taper Length < Min
8	Exit 25 MP 24.726	Taper Length < Min; Rolled Curb

Table 7-2 (Continued) Deficiencies Summary for the Purchase Parkway

Deficiency Type	Milepoint	ciencies Summary for the Purchase Parkway Deficiency Description
Demondriey Type		
1	24.747 - 25.100	Crash Segment CRF = 0.9 (CRF 0.90-0.99)
1	25.100 - 27.452	High Crash Segment - CRF= 1.33 (CRF >=1.0)
	27.452 - 34.487	High Crash Segment - CRF = 1.05 (CRF >=1.0)
3	25.32	Length of Vertical Curve = 536' (584' calcuated minimum)
3	25.32	Stopping Sight Distance = 721' (730' minimum)
9	Exit 27 MP 27.461	Taper Length < Min; Ramp Entrance/Exit Deficient; Divergence Angle > Max; Rolled Curb; Interchange control of access less than 300' minimum
4	27.517	Length of Vertical Curve = 536' (584' calculated minimum)
5	28.625	Length of Vertical Curve = 400' (438' calculated minimum)
6	29.970	Length of Vertical Curve = 400' (416' calculated minimum)
7	31.144	Length of Vertical Curve = 400' (467' calcuated minimum)
8	31.646	Length of Vertical Curve = 600' (608' calculated minimum)
Purchase Parkway	/ - Marshall Count	у
2	34.487 - 41.035	High Crash Segment - CRF = 1.05 (CRF >=1.0)
10	Exit 41 MP 40.809	Taper Length < Min; Divergence Angle > Max
2	MP 41.682	Interchange spacing less than 3 mile minimum
2	41.035 - 42.555	Crash Segment -CRF = 0.99 (CRF 0.90-0.99)
3	42.555 - 46.942	High Crash Segment CRF =1.0 (CRF >=1.0)
11	Exit 43 MP 42.555	Taper Length < Min; Degree of Curve > Max; Ramp Entrance/Exit Deficient; Rolled Curb
5	43.277	Horizontal Clearance = 30' (Note bridge is over 200' long)
6	43.277	Horizontal Clearance = 30' (Note bridge is over 200' long)
₹	43.614	Horizontal Clearance =30' (Note bridge is over 200' long)
8	43.614	Horizontal Clearance =30' (Note bridge is over 200' long)
9	43.872	Horizontal Clearance =30' (Note bridge is over 200' long)
10	43.872	Horizontal Clearance = 30' (Note bridge is over 200' long)
12	Exit 47 MP 46.942	Taper Length < Min; Rolled Curb; Interchange control of access less than 300' minimum
3	46.942 - 51.398	Crash Segment - CRF = 0.91 (CRF 0.90-0.99)
13	Exit 52 MP 51.398	Taper Length < Min; Degree of Curve > Max

Table 7-3 Deficiencies Summary of I-24

Deficiency Type	Milpoint	Deficiency Description
Interstate 24 - Ma	arshall County	
4	24.941 - 26.558	High Crash Segment - CRF =1.10 (CRF >=1.0)
2	26.558 - 29.352	Fatality CRF = 0.71 (CRF >=0.70)
Interstate 24 - Liv	ringston/Lyon Co	unty
2	29.352 - 30.742	Fatality CRF = 0.71 (CRF >=0.70)

A. Operational Considerations and Safety

The following is a summary of the key findings related to the operational considerations and the safety of the Purchase Parkway and I-24:

- <u>Crash Analysis:</u> For the crash analysis, a high crash segment was defined as having a critical crash rate factor greater than or equal to one. Crash segments with a critical crash rate factor between 0.9 and 0.99 are identified in the report.
- <u>Crash Analysis Purchase Parkway:</u> When compared to other Kentucky parkways, there is one high crash segment in Graves County (MP 25.1 MP 27.452) where the crash rate exceeds the statewide average for all parkways. There is one segment in Graves and Marshall Counties (MP 27.452 MP 41.035) with a critical crash rater factor between 0.9 and 0.99.
- <u>Crash Analysis I-24:</u> When compared to other interstates within Kentucky, there is one high crash segment located near the Purchase Parkway interchange in Marshall County (MP 24.941-MP 26.558) where the crash rate exceeds the statewide average for all interstates.
- <u>Crash Analysis Purchase Parkway as an Interstate:</u> When compared to Kentucky interstates, rather than state parkways, two additional high crash segments were identified along the Purchase Parkway located in Graves and Marshall Counties (MP 27.452 MP 41.035 and MP 42.555 MP 46.942).
- <u>Crash Segment Purchase Parkway as an Interstate:</u> There are three segments with a critical crash rate factor between 0.9 and 0.99. These segments are: MP 24.747 MP 25.1, MP 41.035 MP 42.555, and MP 46.942 MP 51.398.
- Additional Findings Related to Crash Analysis: There were only six crashes coded as median cross-over or head-on collisions for the Purchase Parkway and I-24 during the study period (2005-2009). Two crashes occurred on the Purchase Parkway and the remaining four happened on I-24. There were seven fatal crashes on the Purchase Parkway and six fatal crashes on I-24 during the study period (2005-2009).
- <u>Current Traffic (2010):</u> The current Purchase Parkway traffic volumes range from 7,060 vehicles per day (vpd) in Fulton County to 19,200 vpd near I-24 interchange in Marshall County. The current I-24 traffic volumes range from 21,900 vpd near the Purchase Parkway interchange to 28,200 vpd near Calvert City in Marshall County.
- <u>Truck Percentages (2010):</u> The existing truck percentages on the Purchase Parkway range from 24.9% at Mayfield, Kentucky in Graves County to 34.5% near Benton, Kentucky in Marshall County. On I-24, the truck percentage is 24.9%.
- <u>Future Traffic (2040) without I-69:</u> The projected annual growth rate along the Purchase Parkway and I-24 is 2%. This rate results in traffic volumes ranging from 12,800 vpd to 34,800 vpd on the Purchase Parkway and from 39,700 vpd to 51,100 vpd on I-24.
- <u>Future Traffic (2040) with I-69:</u> The projected annual growth rate along the Purchase Parkway and I-24 is 2.5%. This rate results in traffic volumes ranging from 14,800 vpd to 40,300 vpd on the Purchase Parkway and from 45,900 vpd to 53,900 vpd on I-24.
- <u>Truck Percentages (2040):</u> Future truck volumes were not forecasted for this project. However, truck traffic is expected to increase if the national goals of I-69 are met.
- <u>Level of Service (2010):</u> The Purchase Parkway and I-69 currently operate at a LOS C or better, which is acceptable to the AASHTO guidelines.

• <u>Level of Service (2040)</u>: The Purchase Parkway and I-69 are expected to operate at a LOS C or better with or without the I-69 designation.

B. Mainline Geometry/Typical Section

The following is a summary of the key findings related to the Purchase Parkway geometry and typical section:

- <u>Design Speed:</u> The Purchase Parkway meets or exceeds the minimum design speed guidelines for interstate highways in rural and urban areas.
- <u>Lane Width:</u> The lane width on the Purchase Parkway meets the minimum AASHTO guidelines for interstate design.
- <u>Outside Shoulder Width:</u> The Purchase Parkway meets the AASHTO minimum outside shoulder width based on the current truck DDHV.
- Inside Shoulder Width: The Purchase Parkway does not comply with the minimum AASHTO design guidelines for inside paved shoulder widths. The AASHTO minimum inside paved shoulder width is four feet. The Purchase Parkway has a three foot inside paved shoulder with the exception of the Mayfield Bypass where no inside paved shoulder exists.
- Median Width: The Purchase Parkway meets the rural 36 foot AASHTO minimum median width in rural areas and the 10 foot AASHTO minimum median width in urban areas.
- <u>Clear Zones:</u> Based on the provided information and limited field reviews, it is not possible to evaluate the applicability of the current design standards for clear zone on the Purchase Parkway. The fill and cut slopes provided in the typical sections vary from 1V:2H to 1V:4H, the median ditch slope is 1V:4H, and the outside ditch slope is between 1V:3H and 1V:4H.
- <u>Guardrail Placement and Condition:</u> The guardrail end treatments on the Purchase Parkway meet the current standards. An evaluation of guardrail placement is not possible based on the insufficient information provided on the as-built plans.
- <u>Superelevation:</u> From the review of as-built plans, horizontal curves along the Purchase Parkway appear to comply with the AASHTO criteria of 10% maximum superelevation.
- <u>Horizontal Alignment:</u> The horizontal curvature for the Purchase Parkway is acceptable and in compliance with the current AASHTO design guidelines.
- <u>Vertical Alignment:</u> The majority of the vertical curves along the Purchase Parkway meet the current AASHTO guidelines. Eight vertical curves do not meet the guideline for the minimum length of vertical curves.
- <u>Stopping Sight Distance:</u> The minimum stopping sight distance guideline is not met for three vertical curves: MP 14.965, MP 18.727, and MP 25.320.

C. Bridges and Overpasses

The following is a summary of the key findings related to the bridges and overpasses on the Purchase Parkway and I-24:

- <u>Lateral Clearance Purchase Parkway:</u> Of the 46 mainline bridges, 10 do not meet the minimum lateral clearance requirement.
- <u>Vertical Clearance Purchase Parkway and I-24:</u> Of the 35 overpass bridges on the Purchase Parkway, 4 do not meet the minimum 16 foot vertical clearance requirement. The five overpass bridges on I-24 meet the minimum vertical clearance regulation.
- Functional Adequacy: One bridge (MP 21.285) is identified as functionally obsolete.
- <u>Sufficiency Rating:</u> All Purchase Parkway mainline and overpass bridges have a sufficiency rating greater than 60.0.

D. Interchanges and Ramps

The following is a summary of the key findings related to the interchanges and ramps on the Purchase Parkway:

• <u>Design Speed:</u> Design speed for ramps were not provided on the as-built plans.

- <u>Lane Width:</u> Lane widths for the interchange ramps range from 15 feet to 18 feet, which is compliant with AASHTO guidelines.
- <u>Shoulder Width:</u> A majority of the interchange ramps on the Purchase Parkway do not meet the AASHTO guidelines for shoulder width. 10 of the 13 interchanges have ramp shoulder widths that do not meet criteria.
- <u>Horizontal Alignment:</u> One loop ramp at Exit 14 (MP 13.653) does not meet the minimum radius for 25 mph design speed. This ramp has a 130 foot radius within the ramp and the minimum loop ramp radius is 134 feet for 25 mph design speed.
- <u>Vertical Alignment-Vertical Grade:</u> The minimum vertical grade is met on all interchange ramps that were provided on the as-built plans.
- <u>Vertical Alignment-Vertical Length of Curve:</u> Three vertical curve ramps did not meet the
 requirement for minimum length of curve that were calculated based on the minimum ramp
 design speed. These ramps are located at the US 51 interchange (Exit 1) and KY 80
 interchange (Exit 22).
- <u>Vertical Alignment-Stopping Sight Distance:</u> Two vertical curve ramps did not meet the
 minimum stopping sight distance requirement that were calculated based on the minimum
 ramp design speed. These ramps are located at the US 51 interchange (Exit 1) and KY 80
 interchange (Exit 22).
- <u>Superelevation:</u> Based on review of as-built plans, existing ramps appear to satisfy the AASHTO criteria for 10% maximum superelevation.
- <u>Speed-Change Lanes:</u> Many of the existing ramps on the Purchase Parkway do not meet the minimum criteria for acceleration and deceleration lengths.
- <u>Weaving Characteristics</u>: The one location with an existing weaving situation between interchanges will operate at a LOS B with future I-69 traffic projections. The interchanges at Exits 14, 43, and 52 are cloverleaf interchanges with weaving within the interchange.
- <u>Interchange Spacing:</u> On the Purchase Parkway, there are two locations where the minimum interchange spacing requirements are not met. The three interchanges (Exits 0, 1, 2) in Fulton are within three miles of each other. The two interchanges (Exit 41 and Exit 43) in Benton are within three miles of each other.
- <u>Interchange Control of Access:</u> The Purchase Parkway has four interchanges that do not meet the minimum interchange control of access requirements.
- <u>Interchange Configuration:</u> Currently, the Purchase Parkway has four service interchanges that do not meet the recommended interstate interchange configuration. They are located at Exit 0, Exit 14, Exit 21, and Exit 43. The interchange configurations at I-24 and the Purchase Parkway is not recommended for a systems interchange.

E. Design Feature Deficiency and Crash History Analysis

To further evaluate the impact of the roadway feature deficiencies on safety, a crash analysis was conducted to verify the deficiency has an impact on safety.

1. Mainline Geometry/Typical Section

a. Median Type – Mayfield Bypass

On the section of the Purchase Parkway that has a 16 foot non mountable median, there were not any 'cross-over', 'head-on' or fatal collisions. The highest critical rate factor when analyzed as an interstate for segments with this median is 0.55.

b. Vertical Alignment – Minimum Vertical Curves, Minimum Stopping Sight Distance
A rolling crash analysis was conducted for vertical alignment deficiencies. The crashes were analyzed in 0.3 mile segments with reference given to each vertical alignment deficiency.

Table 7-4 below illustrates the findings of analysis.

2. Bridges/Overpasses

a. Bridge Width - Mainline Bridges

A crash analysis was conducted for narrow mainline bridge deficiencies. The crashes were analyzed in 0.3 mile segments with reference given to each vertical alignment deficiency. **Table 7-5** below illustrates the findings of analysis.

F. Superelevation Crash Analysis

As part of this study, a crash analysis was conducted on horizontal curves with a superelevation greater than 8%. **Table 7-6** below illustrates the findings of these analyses. The horizontal curve at MP 47.417 has a critical crash rate factor greater than 1.0. This curve has a radius of 1910 feet and superelevation of 8.3%. From MP 47.117 to 47.717, there were 26 crashes from 2005-2009. Of these crashes, 54% occurred when the roadway condition was either icy, wet, or snow/slush. Five of these crashes (20%) were coded *COLLISION WITH ANIMAL and five* crashes were coded 1 VEHICLE PARKED POSITION (NOT PARKING LOT/DRIVEWAY). Based on the analysis, it is not apparent that the crash history is directly related to superelevation. Therefore, it is not recommended for improvement.

G. Mayfield Bypass

The City of Mayfield, KY has a population of 10,024 and has 4,739 housing units, according to the U.S. Census Bureau. The total area of Mayfield is 6.2 square miles and its population density per square mile of land area is 1,455. Mayfield is the county seat of Graves County. The population of Graves County is 37,121. There are 16,777 housing units within Graves County.

According to KYTC, the functional classification of the Mayfield Bypass is Urban Freeways & Expressways. The Mayfield Bypass traverses approximately for three miles along the west and north borders of the city limits.

The Mayfield Bypass was designed with the intention to serve the City of Mayfield as an urban expressway. The interchanges are spaced at 1 mile or farther. The traffic volumes are approximately 170% higher along the Mayfield Bypass than the rural sections of the Purchase Parkway to the north and south of Mayfield. The 16 foot non mountable median was constructed as an urban expressway. Based on the crash analysis, the Mayfield Bypass operates safer than most of the Purchase Parkway. For this study, the Mayfield Bypass is classified as an urban expressway and was analyzed based on the urban geometric criterion.

DEFIC	IENCY					Avg	Critical	Avg	Critical		Cra	shes				Rates pe	er HMVM		Critical	Critical
Min Length of Vertical Curve (Actual, Minimum)	Min SSD (Actual, Minimum)	MP	Begin MP	End MP	ADT	Crash Rate	Crash Rate	Fatality Rate	Fatality Rate	Fatal	Injury	PDO	Total	HMVM	Fatal	Injury	PDO	Total	Crash Rate Factor	Fatality Rate Factor
			14.665	14.965	8,590	52	148.29	0.8	22.06	0	0	2	2	0.05	0.00	0.00	42.53	42.53	0.29	0.00
500' , 696'	554' , 730'	14.965	14.765	15.065	8,590	52	148.29	0.8	22.06	0	0	2	2	0.05	0.00	0.00	42.53	42.53	0.29	0.00
300 , 030	334 , 730	14.303	14.865	15.165	8,590	52	148.29	0.8	22.06	0	0	2	2	0.05	0.00	0.00	42.53	42.53	0.29	0.00
			14.965	15.265	8,590	52	148.29	0.8	22.06	0	0	1	1	0.05	0.00	0.00	21.26	21.26	0.14	0.00
			18.427	18.727	8,590	52	148.29	0.8	22.06	0	0	1	1	0.05	0.00	0.00	21.26	21.26	0.14	0.00
600' , 624'	727' , 730'	18.727	18.527	18.827	8,590	52	148.29	0.8	22.06	0	0	3	3	0.05	0.00	0.00	63.79	63.79	0.43	0.00
000 , 024	121 , 130	10.727	18.627	18.927	8,590	52	148.29	0.8	22.06	0	0	4	4	0.05	0.00	0.00	85.05	85.05	0.57	0.00
			18.727	19.027	8,590	52	148.29	0.8	22.06	1	0	5	6	0.05	21.26	0.00	106.31	127.58	0.86	0.96
			25.020	25.320	7,790	52	153.67	0.8	23.68	1	1	7	9	0.04	23.45	23.45	164.13	211.02	1.37	0.99
536' , 584'	721' , 730'	25.320	25.120	25.420	7,790	52	153.67	0.8	23.68	1	1	6	8	0.04	23.45	23.45	140.68	187.57	1.22	0.99
330 , 304	721,730	25.320	25.220	25.520	7,790	52	153.67	0.8	23.68	0	2	6	8	0.04	0.00	46.89	140.68	187.57	1.22	0.00
Į.			25.320	25.620	7,790	52	153.67	0.8	23.68	0	1	3	4	0.04	0.00	23.45	70.34	93.79	0.61	0.00
			27.217	27.517	7,320	52	157.27	0.8	24.79	0	1	3	4	0.04	0.00	24.95	74.86	99.81	0.63	0.00
E001 E401		07.547	27.317	27.617	7,320	52	157.27	0.8	24.79	0	1	2	3	0.04	0.00	24.95	49.90	74.86	0.48	0.00
500' , 543'	-	27.517	27.417	27.717	7,320	52	157.27	0.8	24.79	0	1	0	1	0.04	0.00	24.95	0.00	24.95	0.16	0.00
Į.			27.517	27.817	7,320	52	157.27	0.8	24.79	0	0	0	0	0.04	0.00	0.00	0.00	0.00	0.00	0.00
			28.325	28.625	7,320	52	157.27	0.8	24.79	0	2	1	3	0.04	0.00	49.90	24.95	74.86	0.48	0.00
4001 4001			28.425	28.725	7,320	52	157.27	0.8	24.79	0	2	1	3	0.04	0.00	49.90	24.95	74.86	0.48	0.00
400' , 438'	-	28.625	28.525	28.825	7,320	52	157.27	0.8	24.79	0	1	1	2	0.04	0.00	24.95	24.95	49.90	0.32	0.00
Į.			28.625	28.925	7,320	52	157.27	0.8	24.79	0	0	1	1	0.04	0.00	0.00	24.95	24.95	0.16	0.00
			29.670	29.970	7,320	52	157.27	0.8	24.79	0	3	1	4	0.04	0.00	74.86	24.95	99.81	0.63	0.00
4001 4401		00.070	29.770	30.070	7,320	52	157.27	0.8	24.79	0	3	1	4	0.04	0.00	74.86	24.95	99.81	0.63	0.00
400' , 416'	-	29.970	29.870	30.170	7,320	52	157.27	0.8	24.79	0	0	0	0	0.04	0.00	0.00	0.00	0.00	0.00	0.00
Į.			29.970	30.270	7,320	52	157.27	0.8	24.79	0	0	0	0	0.04	0.00	0.00	0.00	0.00	0.00	0.00
			30.844	31.144	7,320	52	157.27	0.8	24.79	0	2	2	4	0.04	0.00	49.90	49.90	99.81	0.63	0.00
4001 4071		04.444	30.944	31.244	7,320	52	157.27	0.8	24.79	0	2	4	6	0.04	0.00	49.90	99.81	149.71	0.95	0.00
400' , 467'	-	31.144	31.044	31.344	7,320	52	157.27	0.8	24.79	0	0	3	3	0.04	0.00	0.00	74.86	74.86	0.48	0.00
!			31.144	31.444	7,320	52	157.27	0.8	24.79	0	0	2	2	0.04	0.00	0.00	49.90	49.90	0.32	0.00
			30.844	31.144	7,320	52	157.27	0.8	24.79	0	1	1	2	0.04	0.00	24.95	24.95	49.90	0.32	0.00
0001 0001		04.040	30.944	31.244	7,320	52	157.27	0.8	24.79	0	2	1	3	0.04	0.00	49.90	24.95	74.86	0.48	0.00
600' , 608'	-	31.646	31.044	31.344	7,320	52	157.27	0.8	24.79	0	3	2	5	0.04	0.00	74.86	49.90	124.76	0.79	0.00
Į.			31.144	31,444	7,320	52	157.27	0.8	24.79	0	2	6	8	0.04	0.00	49.90	149.71	199.62	1.27	0.00

Table 7-4 Vertical Curve Deficiency Crash Analysis

	Dogin			Avg	Critical	Avg	Critical		Cra	shes				Rates pe	er HMVM		Critical	Critical
MP	Begin MP	End MP	ADT	Crash Rate	Crash Rate	Fatality Rate	Fatality Rate	Fatal	Injury	PDO	Total	HMVM	Fatal	Injury	PDO	Total	Crash Rate Factor	Fatalit y Rate Factor
	1.481	1.781	7,060	52	159.42	0.8	25.45	0	0	0	0	0.04	0.00	0.00	0.00	0.00	0.00	0.00
1.781	1.581	1.881	7,060	52	159.42	0.8	25.45	0	0	0	0	0.04	0.00	0.00	0.00	0.00	0.00	0.00
1.761	1.681	1.981	7,060	52	159.42	0.8	25.45	0	1	2	3	0.04	0.00	25.87	51.74	77.61	0.49	0.00
	1.781	2.081	7,060	52	159.42	0.8	25.45	0	1	2	3	0.04	0.00	25.87	51.74	77.61	0.49	0.00
	8.782	9.082	7,290	52	157.51	0.8	24.86	0	1	1	2	0.04	0.00	25.05	25.05	50.11	0.32	0.00
9.082	8.882	9.182	7,290	52	157.51	0.8	24.86	0	1	1	2	0.04	0.00	25.05	25.05	50.11	0.32	0.00
3.002	8.982	9.282	7,290	52	157.51	0.8	24.86	0	0	1	1	0.04	0.00	0.00	25.05	25.05	0.16	0.00
	9.082	9.382	7,290	52	157.51	0.8	24.86	0	2	0	2	0.04	0.00	50.11	0.00	50.11	0.32	0.00
	20.985	21.285	8,590	97 ¹	224.62	0.5	19.53	0	1	5	6	0.05	0.00	21.26	106.31	127.58	0.57	0.00
04 005	21.085	21.385	8,590	97 ¹	224.62	0.5	19.53	1	2	5	8	0.05	21.26	42.53	106.31	170.10	0.76	1.09
21.285	21.185	21.485	8,590	97 ¹	224.62	0.5	19.53	1	1	5	7	0.05	21.26	21.26	106.31	148.84	0.66	1.09
	21.285	21.585	8,590	97 ¹	224.62	0.5	19.53	1	1	6	8	0.05	21.26	21.26	127.58	170.10	0.76	1.09
	42.977	43.277	18,800	52	114.76	0.8	12.84	0	2	12	14	0.10	0.00	19.43	116.58	136.01	1.19	0.00
40.077	43.077	43.377	18,800	52	114.76	0.8	12.84	0	3	7	10	0.10	0.00	29.15	68.01	97.15	0.85	0.00
43.277	43.177	43.477	18,800	52	114.76	0.8	12.84	0	2	4	6	0.10	0.00	19.43	38.86	58.29	0.51	0.00
	43.277	43.577	18,800	52	114.76	0.8	12.84	0	2	4	6	0.10	0.00	19.43	38.86	58.29	0.51	0.00
	43.314	43.614	18,800	52	114.76	0.8	12.84	0	2	5	7	0.10	0.00	19.43	48.58	68.01	0.59	0.00
43.614	43.414	43.714	18,800	52	114.76	0.8	12.84	0	0	5	5	0.10	0.00	0.00	48.58	48.58	0.42	0.00
45.014	43.514	43.814	18,800	52	114.76	0.8	12.84	0	0	6	6	0.10	0.00	0.00	58.29	58.29	0.51	0.00
	43.614	43.914	18,800	52	114.76	0.8	12.84	0	1	10	11	0.10	0.00	9.72	97.15	106.87	0.93	0.00
	43.572	43.872	18,800	52	114.76	0.8	12.84	0	0	6	6	0.10	0.00	0.00	58.29	58.29	0.51	0.00
43.872	43.672	43.972	18,800	52	114.76	0.8	12.84	0	1	9	10	0.10	0.00	9.72	87.44	97.15	0.85	0.00
13.072	43.772	44.072	18,800	52	114.76	0.8	12.84	0	1	9	10	0.10	0.00	9.72	87.44	97.15	0.85	0.00
1 .	43.872	44.172	18,800	52	114.76	0.8	12.84	0	1	9	10	0.10	0.00	9.72	87.44	97.15	0.85	0.00

¹ Average statewide crash rate for interstates in an urban area **Table 7-5 Narrow Bridge Crash Analysis**

Super- elevation MP	MD	BEGIN	END	ADT	Avg Crash	Critical Crash	Avg Fatality	Critical Fatality		Cras	shes		HMVM		Rates p	er HMVI	М	Critical Crash	Critical Fatality
elevation	IVIF	MP	MP	ADI	Rate	Rate	Rate	Rate	Fatal	Injury	PDO	Total	T IIVI V IVI	Fatal	Injury	PDO	Total	Rate Factor	Rate Factor
		0.722	1.022	7,570	52	155.31	0.8	24.18	0	0	1	1	0.04	0.00	0.00	24.13	24.13	0.16	0.00
e = 0.088	1.022	0.822	1.122	7,570	52	155.31	0.8	24.18	0	0	1	1	0.04	0.00	0.00	24.13	24.13	0.16	0.00
e = 0.000	1.022	0.922	1.222	7,570	52	155.31	0.8	24.18	0	0	1	1	0.04	0.00	0.00	24.13	24.13	0.16	0.00
		1.022	1.322	7,570	52	155.31	0.8	24.18	0	0	0	0	0.04	0.00	0.00	0.00	0.00	0.00	0.00
		22.581	22.881	13,100	97 ¹	198.70	0.5	14.27	0	0	3	3	0.07	0.00	0.00	41.83	41.83	0.21	0.00
e = 0.083	22 881	22.681	22.981	13,100	97 ¹	198.70	0.5	14.27	0	0	2	2	0.07	0.00	0.00	27.89	27.89	0.14	0.00
e = 0.003	22.001	22.781	23.081	13,100	97 ¹	198.70	0.5	14.27	0	1	2	3	0.07	0.00	13.94	27.89	41.83	0.21	0.00
		22.881	23.181	13,100	97 ¹	198.70	0.5	14.27	0	3	6	9	0.07	0.00	41.83	83.66	125.48	0.63	0.00
		24.617	24.917	7,790	97 ¹	231.57	0.5	21.04	0	1	4	5	0.04	0.00	23.45	93.79	117.23	0.51	0.00
e = 0.083	24 017	24.717	25.017	7,790	97 ¹	231.57	0.5	21.04	0	2	6	8	0.04	0.00	46.89	140.68	187.57	0.81	0.00
e = 0.063	24.917	24.817	25.117	7,790	97 ¹	231.57	0.5	21.04	0	2	6	8	0.04	0.00	46.89	140.68	187.57	0.81	0.00
		24.917	25.217	7,790	97 ¹	231.57	0.5	21.04	1	1	6	8	0.04	23.45	23.45	140.68	187.57	0.81	1.11
		47.117	47.417	19,200	52	114.05	0.8	12.66	0	3	9	12	0.11	0.00	28.54	85.62	114.16	1.00	0.00
e = 0.083	17 117	47.217	47.517	19,200	52	114.05	0.8	12.66	0	4	11	15	0.11	0.00	38.05	104.64	142.69	1.25	0.00
e = 0.063	47.417	47.317	47.617	19,200	52	114.05	0.8	12.66	0	4	15	19	0.11	0.00	38.05	142.69	180.75	1.58	0.00
1		47.417	47.717	19,200	52	114.05	0.8	12.66	0	2	12	14	0.11	0.00	19.03	114.16	133.18	1.17	0.00

¹ Average statewide crash rate for interstates in an urban area

Table 7-6 Superelevation Crash Analysis

VIII. POTENTIAL IMPROVEMENT ALTERNATIVES AND DEVELOPMENT COSTS

This chapter describes a range of alternatives to address the deficiencies identified on the Purchase Parkway. As discussed in **Chapter I**, the use of the existing parkways is a goal for designating as I-69 through Kentucky. Therefore, the development of a new alignment was not among alternatives considered.

A. Potential Improvements and Development Costs

For this study, the range of alternatives under consideration is No Build, Necessary Upgrades and Spot Safety Improvements, and Fully Compliant Reconstruction. These alternatives are discussed further below and represent incremental levels of infrastructure investment needed to implement I-69 along the Purchase Parkway from the Tennessee state line at Fulton to I-24 near Calvert City.

- No Build This alternate would leave a gap in the nationally proposed I-69 route.
 However, the Purchase Parkway would provide the connectivity for the I-69 traffic to travel from Tennessee to I-24.
- Necessary Upgrades and Spot Safety Improvements Key safety and operational concerns would be addressed. Design exceptions or variances would be obtained for the existing conditions that do not meet current AASHTO or KYTC guidelines that are deemed appropriate by the KYTC and the FHWA.
- Fully Compliant Reconstruction This alternate would involve improvements within existing right of way or with minimum right of way acquisition necessary for making the existing Purchase Parkway meet minimum AASHTO criteria for interstate routes.

Figures are provided at the end of the chapter referencing improved interchanges for the cost estimates.

1. No Build

The Purchase Parkway and I-24 would remain as they are currently without the I-69 designation. This alternate would not require any additional funding for the construction related to upgrading the facilities to current interstate standards.

2. Necessary Upgrades and Spot Safety Improvements

Under this alternate the Purchase Parkway would not be upgraded to meet all current interstate standards. Design exceptions and variances would be necessary for those design features that do not meet current criteria or standards and are deemed appropriate by the KYTC and the FHWA. New infrastructure and improvements along the Purchase Parkway would be proposed to upgrade necessary features and improve safety. **Table 8-1** contains a summary of the preliminary costs and design assumptions for implementing improvements proposed in these alternatives.

Item	Unit	2011 Cost		
Correct Vertical / Stopping Sight Distance Deficiencies (MP 25.32)	Total	\$30,000		
Upgrade Crash Worthy Pier Protection	Location	\$30,000		
Upgrade Mayfield Bypass Median	Mile	\$725,000		
Mainline Structures (Upgrade Guardrail/Approaches/Railings)	Foot	\$85		
Overpass Structures (Upgrade Deficiencies)	Total	\$330,000		
Interchanges				
Interchange Ramp Improvements	Interchange	Variable		
Toll Plaza - Exits 14 and 43 (Upgrade Deficiencies)	Interchange	\$21,600,000		
Unique Interchanges				
Exit 21 (Upgrade Deficiencies)	Interchange	\$25,360,000		
I-69 / I-24 (Upgrade Partial Deficiencies)	Interchange	\$15,700,000		
Design and Environmental	15% of Construction Costs			
Right-of-Way and Utilities	30% of Construction Costs			

Table 8-1 Unit Costs - Necessary Upgrades and Spot Safety Improvements

A summary of the recommended improvements for this option are below:

- Maintain the existing mainline along the Purchase Parkway
- Correct vertical curve and stopping sight distance deficiencies at MP 25.32. This improvement is due to a crash rate greater than the statewide average at this location.
- Upgrade for crash worthy pier protection for existing structures
- Upgrade Mayfield Bypass median and inside shoulder
- Retrofit the bridge railing/barriers that do not meet current minimum standards
- Upgrade overpass structures to meet the minimum 16 foot vertical clearance (driving lanes and shoulders)
- Upgrade improvements to substandard interchanges
- Upgrade the previous toll booth interchanges at Exits 14 and 43 to meet interstate standards
- Upgrade the Exit 21 (US 45) Trumpet interchange (Figures 8-3, 8-4) to meet interstate standards
- Upgrade the I-69/I-24 systems interchange. Refer to Figure 8-1 for recommended configuration.

As shown in **Table 8-2**, the preliminary cost associated with this alternate is \$132 million. Almost half of this cost is associated with the upgrading of the Exit 14 (KY 339) and Exit 43 (KY 348) previous toll booth interchanges and upgrading the interchanges at Exits 21 (US 45) and 52 (I-24).

Segment	Length (miles)	Design & Environmental (million)	ROW and Utilities (million)	Const			
				Roadwork	Mainline & Overpass Structures	Interchanges	Total Costs (million)
Fulton MP 0.0 - MP 3.0	3	\$0.24	\$0.48	\$0.00	\$0.22	\$1.36	\$2.30
Fulton to Mayfield MP 3.0 - MP 21.0	18	\$3.32	\$6.64	\$0.00	\$0.52	\$21.60	\$32.08
Mayfield MP 21.0 - MP 25.2	4.2	\$4.30	\$8.59	\$2.18	\$0.27	\$26.16	\$41.50
Mayfield to Benton MP 25.2 - MP 40.0	14.8	\$0.11	\$0.22	\$0.03	\$0.25	\$0.43	\$1.04
Benton MP 40.0 - MP 43.0	3	\$3.25	\$6.50	\$0.00	\$0.03	\$21.63	\$31.41
Benton to Calvert City MP 43.0 - MP 51.4	8.4	\$2.45	\$4.89	\$0.00	\$0.21	\$16.07	\$23.62
Total	51.4	\$13.67	\$27.32	\$2.21	\$1.50	\$87.25	\$131.95

Table 8-2 Necessary Upgrades and Spot Safety Improvement Preliminary Cost Estimate

3. Fully Compliant Reconstruction

The Fully Compliant Reconstruction option would involve improving the Purchase Parkway to meet all the minimum design guidelines for interstate highways. **Table 8-3** contains a summary of the preliminary costs and design assumptions for implementing improvements proposed in this alternate.

Item	Unit	2011 Cost		
Correct Vertical / Stopping Sight Distance Deficiencies	Total	\$250,000		
Upgrade Crashworthy Pier Protections	Location	\$30,000		
Widen Inside Shoulders to 4 foot paved	Mile	\$77,000		
Auxiliary Lane (Interchange Spacing - Exit 41/Exit 43)	Mile	\$4,233,000		
Upgrade Mayfield Bypass Median	Mile	\$725,000		
Mainline Structures (Widen Deficient Bridges)	Foot	\$65		
Mainline Structures (Upgrade Guardrail/Approaches/Railings)	Foot	\$200		
Overpass Structures (Upgrade Deficiencies)	Total	\$330,000		
Interchange Control of Access	Total	\$5,000,000		
Interchanges				
Interchange Ramp Improvements	Interchange	Variable		
Toll Plaza - Exits 14 and 43 (Upgrade Deficiencies)	Interchange	\$21,600,000		
Unique Interchanges				
Exit 21 Upgrade Deficiencies	Interchange	\$25,360,000		
I-69 / I-24 (Replace with fully directional)	Interchange	\$65,800,000		
Design and Environmental 15% of Construction				
Right-of-Way and Utilities 30% of Construction Cos				

Table 8-3 Unit Costs – Fully Compliant Reconstruction

A summary of the improvements for this option are below:

- Maintain the existing mainline along the Purchase Parkway
- Correct any vertical curve and stopping sight distance deficiencies
- Upgrade crash worthy pier protection
- Widen the inside paved shoulder to 4 foot
- Upgrade Mayfield Bypass median and inside shoulder
- Widen the mainline bridges that are deficient in horizontal lateral clearance to 31 feet. All
 of these bridges are greater than 200 feet long.
- Replace the bridge railing/barriers that do not meet current minimum standards
- Upgrade overpass structures to the meet minimum 16 foot vertical clearance (driving lanes and shoulders)
- Upgrade the improvements to substandard interchanges (acceleration/decelerations length, divergence angle, shoulder width, curb removal)
- Reconstruct the toll booth interchanges at Exits 14 and 43
- Upgrade Exit 21 (US 45) deficiencies (**Figure 8-6**)
- Upgrade the I-69/I-24 systems interchange Refer to Figure 8-2 for interchange configuration
- Construct auxiliary lanes between Exit 41 and Exit 43 to comply with interchange spacing.

As indicated in **Table 8-4**, the improvements for this alternate are estimated at \$219 million. At an average cost per mile of \$4.26 million, the Purchase Parkway can be improved to meet the minimum interstate design standard without any design exceptions. The majority of the cost estimate is associated with the reconstructing interchanges on the Purchase Parkway.

Segment	Length (miles)	Design & Environmental (million)	ROW and Utilities (million)	Construction Costs (million)			
				Roadwork	Mainline & Overpass Structures	Interchanges	Total Costs (million)
Fulton MP 0.0 - MP 3.0	3	\$0.32	\$0.63	\$0.24	\$0.49	\$1.36	\$3.04
Fulton to Mayfield MP 3.0 - MP 21.0	18	\$3.58	\$7.16	\$1.67	\$0.51	\$21.68	\$34.60
Mayfield MP 21.0 - MP 25.2	4.2	\$4.31	\$8.62	\$2.27	\$0.24	\$26.22	\$41.66
Mayfield to Benton MP 25.2 - MP 40.0	14.8	\$0.35	\$0.69	\$1.26	\$0.53	\$0.50	\$3.33
Benton MP 40.0 - MP 43.0	3	\$3.98	\$7.96	\$4.47	\$0.10	\$21.94	\$38.45
Benton to Calvert City MP 43.0 - MP 51.4	8.4	\$10.13	\$20.25	\$0.65	\$0.66	\$66.17	\$97.86
Total	51.4	\$22.67	\$45.31	\$10.56	\$2.53	\$137.87	\$218.94 ¹

¹ Cost estimate does not include cost associated with connecting to Segments of Independent Utility (SIU) 5 (I-24 at Western Kentucky Parkway) or SIU 7 (Exits 0,1,2 at Fulton, KY).

Table 8-4 Fully Compliant Reconstruction Preliminary Cost Estimate

4. Summary

The following table provides a cost comparison of the potential alternates provided in this study. The cost per mile estimate is based on the 51.4 miles of the Purchase Parkway. The Necessary Upgrades / Spot Safety Improvements alternative cost is approximately two-thirds the cost of the Fully Compliant Reconstruction alternative. The difference in cost results from inside shoulder improvement, bridge widening, auxiliary lanes, and reconstructing the Purchase Parkway and I-24 interchange to meet full compliance for a systems interchange.

Alternative	Meet Current Standards	Impact on Cost Environment (million)		Cost per Mile ¹ (million)
1. No Build	No	Least	\$0.00 2	\$0.00
2. Necessary Upgrades / Spot Safety Improvements	Yes ³	Minimal	\$131.95	\$2.57
3. Fully Compliant Reconstruction	Yes	More Significant	\$218.94 ⁴	\$4.26

Table 8-5 Cost Comparison of Potential Alternatives

¹ Cost per mile based on 51.4 miles of Purchase Parkway.

² Cost for routine maintenance is not depicted in alternatives.

³ This alternative would include upgrading the design features along the Purchase Parkway that potentially represents the most significant safety and operational issues. This alternative requires design exceptions and variances where safety and operational conditions would not create undue risk to the motorist.

⁴ Cost estimate does not include cost associated with connecting to Segments of Independent Utility (SIU) 5 (I-24 at Western Kentucky Parkway) or SIU 7 (Exits 0,1,2 at Fulton, KY).

5. Potential Interchange Improvements/Reconstruction

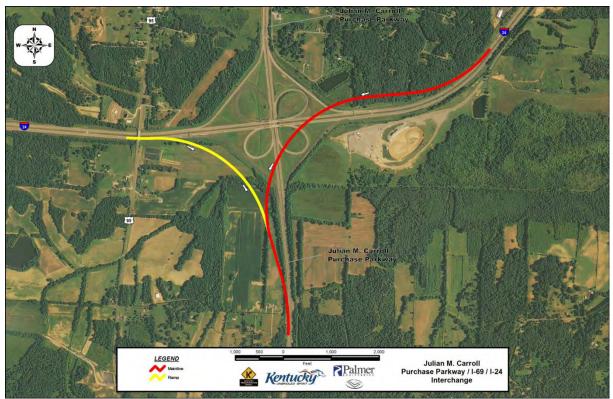


Figure 8-1 I-69 / I-24 Interchange (Upgrade Option)

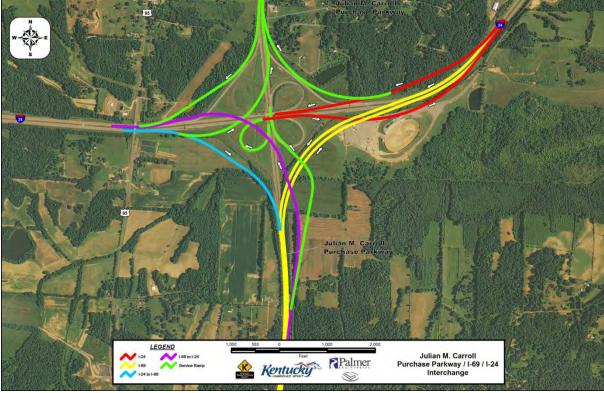


Figure 8-2 I-69 / I-24 Interchange (Reconstruction)

<u>Chapter VIII – Potential Improvement Alternatives and Development Costs</u>



Figure 8-3 I-69 / US 45 Interchange (Upgrade Option 1)



Figure 8-4 I-69 / US 45 Interchange (Upgrade Option 2)

<u>Chapter VIII – Potential Improvement Alternatives and Development Costs</u>



Figure 8-5 I-69 / US 45 Interchange (Reconstruction)

IX. RECOMMENDATIONS

The following chapter includes recommendations for improvements to the Purchase Parkway and related work for future designation as I-69.

As previously discussed in **Chapter I**, the FHWA has identified thirteen design features that are important to the operational and safety performance a highway. These controlling design features compiled are commonly known as the *13 controlling criteria*. A formal written design exception is required when any of the 13 criteria are not met on the National Highway System (NHS). The Interstate System is part of the NHS. The *13 controlling criteria* is listed below. Design features that deviate from common practice but are not included in the *13 controlling criteria* will be termed design variance. There are two categories for design variances. A design variance is a design feature that (1) varies from the current AASHTO criteria but not part of the *13 controlling criteria* or (2) a design feature that varies from common practice but not part of the *13 controlling criteria*. A summary of recommended design exceptions and design variances is provided at the end of this chapter.

- 1. Design speed
- 2. Lane width
- 3. Shoulder width
- 4. Bridge width
- 5. Horizontal alignment
- 6. Superelevation
- 7. Vertical alignment
- 8. Grade
- 9. Stopping sight distance
- 10. Cross slope
- 11. Vertical clearance
- 12. Lateral offset to obstruction
- 13. Structural capacity

A. Recommendations

It is recommended that the Necessary Upgrades and Spot Safety Improvements alternative be chosen for initial advancement based on the following:

- The Purchase Parkway adequately meets AASHTO guidelines for most design elements of an interstate. Of the design element deficiencies, others may be accepted as design exception/variance with agreement by the KYTC and the FHWA.
- Based on the operational and crash analysis included in this study, addressing those repairs identified for Needed Upgrades and Spot Safety Improvements will appropriately address any crash history concerns identified. The entire length of the Purchase Parkway meets the level of service required and only a few locations exhibit potential safety problems.

The following summarizes a strategy for implementing the Necessary Upgrades and Spot Safety Improvement alternative.

1. Geometry/Typical Section

a. Inside paved shoulder (Four foot paved minimum) – It is recommended that a design exception be requested for the minimum inside paved shoulder on the Purchase Parkway that currently has three foot paved inside shoulders with the intention to correct the existing deficient inside shoulder during future pavement rehabilitation or resurfacing projects.

It is recommended to construct a 4 foot paved shoulder for the Mayfield Bypass section of the Purchase Parkway. Currently, there is not a paved inside shoulder on this section.

- b. Vertical Curve / Stopping Sight Distance It is recommended to seek a design exception for the locations mentioned in Chapter IV that do not meet the minimum vertical length of curve and minimum stopping sight distance except one location at MP 25.32. Based on crash history data, Table 7-4, only this location (MP 25.32) has a significant crash history (CRF>1.0). Correction of this vertical curve is recommended. Since there is not a crash history associated with the other locations, it is not considered cost effective to improve. Correction of vertical curvature/stopping sight distance concerns may be addressed for other locations during future pavement rehabilitation or resurfacing projects.
- c. Superelevation Referencing the Federal Highway Administration *Mitigation Strategies for Design Exceptions*, "A formal design exception is required if the State's superelevation policy cannot be met in design of any curve on the NHS." This document advises, "A design exception is also required if a superelevation rate is proposed that is different from the published rate per the State's policy for that curve, regardless of whether the curve is a controlling one (minimum radius for a design speed) or not." The current KYTC geometric policy references the AASHTO Policy on Geometric Design of Highway and Streets, current edition, which provides maximum superelevation rate tables for 4%, 6%, 8%, 10%, and 12%. From review of as-built plans and field inspections, it appears that the Purchase Parkway was constructed on the basis of 10% maximum superelevation.

A crash analysis on horizontal curves with a superelevation greater than 8% is provided in **Chapter VII**. There is one horizontal curve with a superelevation of 8.3% that has a critical crash rate factor greater than one. Based on the analysis, it is not apparent that the crash history is directly related to the superelevation.

Since the Purchase Parkway appears to have been constructed with a maximum superelevation of 10% which is compliant with the AASHTO and KYTC policies, and there are no apparent crash histories related to superelevation, a design exception for superelevation does not appear warranted.

d. Mayfield Bypass Median Width/Type – It is recommended to construct a new median on the Mayfield Bypass section of the Purchase Parkway. Currently the median is defined as non mountable because it has a raised curb along edge of driving lane. The crash history for this section does not correlate with the need of a median barrier. According to the AASHTO Roadside Design Guide, a median barrier is optional for the existing median width (16 feet) and existing Average Daily Traffic (14,300 vpd). However, median barrier is recommended for the design year (2040) forecasted traffic as an interstate (30,000 vpd) on the Mayfield Bypass. It is recommended to construct a new median with barrier for this section based on the need to construct inside paved shoulders, forecasted interstate traffic, and drainage needs in the median after constructing inside paved shoulders.

2. Bridges and Overpasses

- a. Vertical Clearance It is recommended to correct the vertical clearance for the four overpasses bridges that do not meet minimum criteria.
- b. Mainline Bridge Railing It is recommended to retrofit the bridge railing for all mainline bridges to meets current criteria. Retrofitting the bridge railing should be completed prior to interstate designation.
- c. Mainline Bridge Width It is recommended to seek a design exception for mainline bridge width. With the exception of the two mainline bridges at Exit 21, there are 10 bridges with an existing width of 30 feet. The mainline bridges identified as deficient are greater than

200 feet in length and would need to be widened from their existing 30 foot width to 31 feet. Based on the crash analysis, there does not appear to be a crash history related to bridge width at these locations.

d. Crash Worthy Pier Protection – It is recommended to improve the crash worthy protection of overpass bridge piers along the Purchase Parkway. There are eight overpasses that have earthen mound pier protection that do not meet current standards.

3. Interchanges and Ramps

- a. Interchange Spacing It is recommended to seek a design variance for interchange spacing for the rural interchange spacing in Fulton, KY (Exit 0, Exit 1, and Exit 3) and in Benton, KY (Exit 41 and Exit 43).
- b. Interchange Control of Access It is recommended to seek a design variance for the interchange control of access deficiencies at Exit 14, Exit 22, Exit 27, and Exit 47. As future improvements and rehabilitation projects are indicated, control of access at these locations can be initialized.
- c. Interchange Deficiencies
 - Exit 0 Weigh Station Interchange In the future, coordination between the Tennessee Department of Transportation and Kentucky Transportation Cabinet (KYTC) will more clearly define future I-69 connectivity between Kentucky and Tennessee at this location. Specific deficiencies will be more thoroughly addressed at that time.
 - Exit 14 Previous Toll Plaza Interchange It is the recommended to improve the existing interchange at Exit 14 to meet current interstate criteria.
 - Exit 21 Mayfield Bypass Interchange It is recommended to improve the interchange to meet current interstate criteria.
 - Exit 43 Previous Toll Plaza Interchange It is the recommended to improve the existing interchange at Exit 43 to meet interstate standard. This interchange has been identified by KYTC for improvement and is currently under design.
 - I-24 / Purchase Parkway Interchange It is recommended to improve the eastbound I-24 to southbound I-69 ramp and construct a new southbound I-69 flyover ramp from westbound I-24. The following existing ramps will be eliminated with this recommendation:
 - Westbound I-24 to northbound Purchase Parkway ramp
 - o Westbound I-24 to southbound Purchase Parkway loop ramp
 - o Eastbound I-24 to northbound Purchase Parkway loop ramp

The existing northbound Purchase Parkway to westbound I-24 loop ramp also will remain in place and serve as the northbound I-69 to westbound I-24 connector under this scenario.

This option will require a deferral (design exception) for the I-69 northbound movement. The existing northbound Purchase Parkway to eastbound I-24 ramp will serve as the I-69 north movement. This ramp will accommodate the traffic in the near future. However, it is recommended to improve the ramp to meet current criteria once traffic volumes exceed capacity. It also is recommended to construct a new northbound I-69

to westbound I-24 flyover ramp once the traffic volumes exceed the existing loop ramp capacity.

The recommended improvements are shown in **Figure 8-1** and the construction cost is provided in **Table 8-1 Unit Costs-Necessary Upgrade and Spot Safety Improvements**. Additional improvements to the interchange include extending acceleration and deceleration tapers.

- d. Ramp Typical Section It is recommended to improve existing ramp cross section geometry that does not meet current interstate criteria. Currently, a majority of the interchange ramps have rolled curb and deficient shoulder widths. It is recommended to remove the rolled curb on the interchange ramps and construct minimum shoulder width in order to meet minimum interstate criteria.
- e. Ramp Alignment Geometry
 - Divergence Angle It is recommended to correct the deficient ramps with divergence angles that do not meet current criteria.
- f. Speed Change Lanes It is recommended to improve all ramp tapers and acceleration/deceleration lanes to meet current interstate criteria.

4. Design Exception and Variance Summary

The following table summarizes the essential design elements commonly known as the 13 controlling criteria. Following the 13 controlling criteria are design variances which do not meet the AASHTO criteria of an interstate.

13 Design Criteria	Meets Criteria (Yes or No)	Cost to Cure (\$)	Design Exception/Variance should be requested	Explanation
Design Speed	Yes*	\$33,860,000	Yes	The Purchase Parkway meets the design speed criteria except for the new I-69 through route at the I-69/I-24 interchange. It is recommended to improve the eastbound I-24 to southbound I-69 ramp, construct a new I-69 southbound flyover ramp, and seek a design exception for the northbound I-69 through movement.
Lane Width	Yes	-	-	-
Shoulder Width	No	\$3,730,000 ²	Yes	The inside paved shoulders need to be widen from 3 feet to 4 feet for the Purchase Parkway. The paved inside shoulder for the Mayfield Bypass should be widened inconjunction with construction of a new median. It is recommended to seek a design exception for inside paved shoulder of the Purchase Parkway, with the exception of the Mayfield Bypass shoulder improvement.
Bridge Width	No	\$2,370,000 ³	Yes	There are 12 bridges on the Purchase Parkway that do not meet criteria. 10 of these 12 bridges are long bridges (> 200 feet long). The remaining two bridges are part of the Exit 21 interchange that is recommended to be upgraded to meet interstate standards. It is recommended to seek a design exception for bridge width.
Horizontal Alignment	Yes	-	-	-
Superelevation	Yes	-	-	-
Vertical Alignment	No	\$250,000	Yes	There are eight deficient vertical curves on the Purchase Parkway. All of these curves are sag curves and six are close to meeting criteria. There is one deficient curve that is recommended to correct due to crash history.
Grade	Yes	-	-	-
Stopping Sight Distance	No	\$167,000		There are 3 vertical curves that do not meet the minimum stopping sight distance. All of these curves are a sag and 2 are close to meeting criteria. This cost is also included in the cost to cure vertical alignment deficiencies. It is recommended to improve one vertical curve due to crash history.
Cross Slope	Yes	-	-	-
Vertical Clearance	No	\$330,000	No	There are 4 overpasses on the Purchase Parkway that do not meet vertical clearance requirements. It is recommended to improve the overpass bridge clearance to meet current criteria.
Lateral offset to obstruction	-	-	-	-
Structural Capacity	Yes	-	-	•
Design Variances				
Acceleration & Deceleration Lengths / Divergence Angle	No	\$768,000 ¹	No	It is recommended to improve the deficient ramps to meet AASHTO criteria.
Interchange Spacing	No	\$4,233,000	Yes	The interchange spacing between Exit 41 and Exit 43 is less than the 3 mile rural recommended criteria. Cost to cure does not include the interchange spacing deficiency at Exit 0, Exit 1, and Exit 2. The connection of I-69 between Kentucky and Tennessee is still in question, which would include these exits at Fulton, KY and Fulton, TN.
Interchange Control of Access	No	\$5,000,000	Yes	There are four interchanges that have interchange control of access less than the minimum criteria
Interchange Ramp Shoulder Width / Curb	No	\$2,937,000 ¹	No	Many of the interchange ramps do not meet the minimum shoulder width and/or have an existing curb.
Bridge Side railing/barrier	No	\$930,000	No	All of the mainline bridges have brush blocks rather than a barrier slope that meets current standards. It is recommended to retrofit the existing side railing/barrier to meet criteria.
Bridge Pier Protection	No	\$240,000	No	Eight overpass bridges have earthen mound bridge pier protection that does not meet current standard. It is recommended to improve the pier protection to meet current standard.
Median Width / Type	Yes	\$2,175,000 4	-	Construct a new median with barrier and inside shoulder on the Mayfield Bypass. (Cost includes median barrier and inside paved shoulders.)

Table 9-1 Summary of Design Exceptions and Variances

Does not include improvements associated with Exit 14 (Toll Plaza), Exit 21 (Modified Trumpet), and Exit 43 (Toll Plaza)

² Does not include widening Mayfield Bypass inside paved shoulder.

³Cost is associated with widening the deficient mainline bridges to 31 feet curb to curb.

⁴ The median width meets AASHTO criteria. The median type meets AASHTO criteria with current traffic volumes ,but not future traffic projections.

Designation of the Purchase Parkway as I-69 may be accomplished by implementing the recommended improvement strategies in coordination with the Federal Highway Administration. Information presented herein is a *first look* to identify deficiencies and impediments for designation as I-69 and to identify a range of improvement strategies needed to upgrade the Purchase Parkway to satisfy applicable interstate criteria with applicable design exceptions/variances by the Federal Highway Administration. Thus, as projects for implementation of recommended improvement strategies are initiated, additional engineering analysis and studies may be needed to further refine the specifics for improvements. For example, the following may be areas for further analysis.

- Operational Considerations There may be roadway conditions not shown in crash data contributing to crash history. Additional analyses during preliminary engineering may provide additional insight which could refine the scope of needed improvements at a given location.
- Mainline Geometry and Typical Section Analyses for mainline geometry and typical section were evaluated using as-built plans supplemented with field reviews of existing conditions. Actual design features may require further verification with non-detailed field reviews of the roadway cross-section during preliminary engineering for implementing improvement strategies.
- Interchanges and Ramps Most of the interchange ramps are deficient and some design features were illegible on the as-built plans. Therefore, as interchanges are identified for improvement, geometric features (i.e. superelevation rate, horizontal and vertical alignments, design speed, etc.) should be further analyzed.

B. Summary and Conclusion

Based on the findings of this study, it can generally be concluded that the Purchase Parkway is currently providing motorists efficient and safe travel from US 51 in Tennessee to I-24 with operating conditions similar to an interstate. There would be minimal to no impact to the operating characteristics of the Purchase Parkway in the near future if it was designated as I-69 under the current conditions. The operation characteristics of the I-69 corridor would not be expected to be altered until more sections of I-69 are completed across the country especially in Tennessee and Indiana. As sections of I-69 are completed and thus provide continuity at a regional and national level, additional truck traffic volume will likely grow on the Purchase Parkway to the point that estimated truck traffic and congestion along the existing Purchase Parkway may eventually alter the operational characteristics.

Intuitively, there may be sections of interstate in Kentucky and around the United States that do not meet the current design standards. Some design features on these other interstates may be very similar to the existing design features on the Purchase Parkway. Based on the impact to other sections of Parkways that are designated as future interstate corridors and existing interstates with similar design feature deficiencies, designation of the Purchase Parkway as I-69 under the Parkway's existing conditions appears realistic.

There are two broad based potential improvement alternatives recommended for improving the Purchase Parkway to meet interstate standards. The Necessary Upgrades and Spot Safety Improvement alternative includes upgrading the Purchase Parkway to meet all current interstate standards but with design exceptions/variances. The Fully Compliant Reconstruction alternative would upgrade the Purchase Parkway to meet interstate standards with no design exceptions or variances. Right of way acquisitions will be needed for interchange improvements.

In general, improvements related to bridge deficiencies, Mayfield Bypass median, interchange acceleration and deceleration lanes, and toll plaza interchange improvements are recommended. It is also recommended that initially, minimal improvements should be made to the Purchase

Parkway and I-24 interchange and US 45 interchange in Mayfield. The minimal improvements should be designed to provide continuity and capacity for the forecasted traffic, while maintaining consideration for crash history and safety for the traveling public. Ultimately, as traffic operations change and traffic volumes increase, additional improvements to these interchanges may be needed to improve safety and meet current interstate criteria.



Environmental Overview for I-69 from Fulton to Eddyville

Introduction

This environmental overview covers the proposed I-69 corridor from the Kentucky–Tennessee state line in Fulton, Kentucky, to Knob Creek Bridge near Eddyville, Kentucky extending approximately 67 miles. The corridor lies within the Coastal Plain physiographic region (also known as the Jackson Purchase or Mississippi Embayment) which is dominated by flat, low plains dissected by a dendritic drainage network of low-gradient streams and small rivers flowing into the Mississippi and Tennessee Rivers. Uplands are underlain by sand, gravel, silt, and clay deposited by the last inland invasion of the seas more than 65 million years ago. It is part of the oldest northern extension of today's Coastal Plain of the southeastern United States. A silty mantle covers much of the region, but it is deepest along the Mississippi River where bluffs formed by this windblown material (loess).

Ecological

Potential ecological impacts were researched using available literature and internet-based searches. Some of the federal and state agencies from which information was sought were US Fish and Wildlife Service (USFWS), Kentucky Department of Fish and Wildlife Resources (KDFWR), Kentucky Division of Water (KDOW), Kentucky State Nature Preserve Commission (KSNPC), Kentucky Transportation Cabinet (KYTC), US Geological Service (USGS) topographical maps, and National Wetland Inventory (NWI) maps. Additional ecological information is located at the end of this overview.

Surface Waters

The USGS 7.5 minute topographic maps covering the project area were reviewed to determine the number and type of stream crossing. This is a partial list of all stream crossings since not all streams are labeled on topographical maps. Notable stream crossings include: Bayou du Chien and all its tributaries are Outstanding State Resource Waters because they are known habitat for the federally endangered relict darter; Panther Creek is a state Exceptional Quality and Reference Reach stream; Tennessee and Cumberland Rivers are Outstanding State Resource Water because they are known habitat for federally endangered mussel species; Clarks River because it is part of the Clarks River National Wildlife Refuge.

Number of blue-line streams crossed by county

Number of Blue-line Perennial	Number of Blue-line Intermittent	Lake	Rivers
27	52	1	4

Wetlands

Since much of the Lower Cumberland and Purchase area is alluvial floodplain with rich soils ideal for farming, agriculture is a large part of the local economy. The alluvial floodplains are also rich in wetlands. NWI maps show an abundance of wetlands scattered throughout and in proximity to the corridor. The following is a list of wetlands that the corridor bisects or is adjacent to as indicated by NWI mapping. The actual number of wetlands can only be determined by field reconnaissance.

Number of wetlands by type

	-7 F -
Type of Wetland	Number of Occurrences
Emergent	5
Ponded Emergent	1
Ponded Scrub Shrub	1
Ponded Forested	10
Riverine	4
Lacustrine	1
Total	22

Threatened and Endangered Species

The 2008 USFWS list of threatened and endangered species was referenced for the counties involved. The KSNPC species database for each county was also referenced. Individual federally listed species and a quantitative listing of state species is presented by each county in project corridor. Notable species on the USFWS list are the Indiana bat, gray bat, relict darter, and mussels. Communications with USFWS and KSNPC will refine the number of listed species potentially affected by this project.

After reviewing USGS topographic maps, aerial maps, and field reconnaissance the several potential bat habitat and travel corridors were identified: creeks (Bayou du Chien, Mayfield, West Fork Clarks); rivers (Clarks, Tennessee, Cumberland); and wildlife management areas (Land Between the Lakes, Clarks River National Wildlife Refuge). Other natural features were determined throughout the project area as potential areas suitable for sustaining Indiana bats and gray bats.

The relict darter is endemic to Bayou du Chien drainage basin and because of this the Bayou du Chien and its tributaries are listed as Outstanding State Resource Waters by the KDOW. The current project bisects known relict darter habitat in the Bayou du Chien, roughly 5,200 feet downstream of the species' primary breeding area in Jackson Creek. Given the relict darter's limited distribution and apparent dependence on one spawning area (Jackson Creek), the relict darter is extremely vulnerable to anthropogenic activities (Warren et al. 1994).

The project corridor crosses the Tennessee and Cumberland Rivers, known habitat areas for federally and state endangered mussels. Little impact, if any, is anticipated because no known modifications are expected for bridges that cross these rivers.

Socioeconomic

Environmental Justice Populations

Based on the current level of information available, no significant adverse social or economic impacts are anticipated from proposed project; however, these preliminary findings will require validation through appropriate environmental Baseline studies required in subsequent project phases.

Relocations

Based on the current level of information available, no residential, commercial, or non-profit organization relocations are anticipated from the proposed project; however, these preliminary findings will require validation through appropriate environmental Baseline studies required in subsequent project phases.

Cultural Historic Resources

Historic Architectural Resources

If the project advances using federal funds, a historical baseline analysis will be required. Since a reconstruction project has the potential to have adverse impacts to historic resources, Section 106 of the National Historic Preservation Act of 1966 initiation would begin once the environmental documentation and design of any future project started.

Archaeological Resources

Even though a low potential for intact archaeological sites has been projected, the project area has not been subjected to a Phase I archeological investigation, and the presence of currently unidentified archaeological sites within the project area are possible.

Air

The U. S. Environmental Protection Agency (EPA) has identified seven air pollutants of national concern, including carbon monoxide (CO), nitrogen oxides (NO_x), ozone (O₃), particulate matter (PM₁₀ and PM_{2.5}), sulfur oxides (SO_x), and lead (Pb). FHWA requires, by the development of air quality base studies, the modeling of CO, if needed, to determine and compare calculated existing and future concentrations with the National Ambient Air Quality Standards (NAAQS) and, if required, a qualitative hot spot analysis for PM_{2.5}. A CO analysis will not be require because traffic projections will not exceed the 80,000 average daily traffic (ADT) Jefferson County is currently designated by the U.S. Environmental Protection Agency (EPA) to be in maintenance for O₃. Carbon Monoxide is not considered a concern for this project. Projects within Jefferson County increasing roadway capacity will be required to comply with the fine particulate, PM2.5, hotspot consideration requirements. In addition, a Mobile Source Air Toxics (MSATs) analysis will be required for the proposed project.

A project-specific air quality impact assessment will be required for this undertaking as a part of the NEPA-phase project activities. This will be necessary in order to insure that the selected alternative does not adversely affect air quality programs currently in place and maintains conformity with the Transportation Improvement Program (TIP). This assessment will take into consideration the affects of local climate and topography and include a microscale dispersion analysis. Project impacts on future air quality conditions through air quality modeling will be used to compare the relative effects of each project alternative and to determine whether or not CO emissions attributable to the project would cause or contribute to an exceedance of the NAAQS. Since the project area consists of residential and commercial land uses, representative air quality receptors will need to be identified in conjunction with the existing facility and with specific alignment alternatives and considered as a part of the assessment. Depending on the results of the assessment, air quality impacts may be important in determining the constructability of the various alternatives and in selecting a preference among them.

Construction-period air quality impacts will need to be evaluated to expose the potential short-term effects of site preparation, demolition, material storage, construction actions, and to determine if any appropriate mitigation commitments will be incorporated into the project plans.

Noise

To determine potential noise impacts from construction and operation of the proposed project, each representative noise-sensitive land use will need to be identified in conjunction with specific alignment alternatives and existing measured ambient noise levels. The procedure for conducting

field monitoring will be based on FHWA requirements and KYTC Noise Abatement Policy. Noise levels will be measured in terms of $L_{\rm eq}$, which reflects the average equivalent steady state sound level; in a stated time period, usually one hour, it would contain the same acoustic energy as the time-varying sound level during the same time period. For future noise level predictions, FHWA TNM (Traffic Noise Model) 2.5 will be used for noise impact analysis.

Given the location of the project area, the vehicle mix, patterns and volumes of traffic, and the general absence of sensitive receptors, highway noise impacts are not expected to influence project feasibility or location decisions; however, a project-specific noise impact analysis will be required to verify noise impact conditions.

Fulton County - There are no federal or state natural areas

USFWS

Group	Species	Common Name	Legal Status	Occurrence
Mammals	Myotis sodalis	Indiana bat	Endangered	Potential
Mussels	Potamilus capax	Fat pocketbook	Endangered	Potential
Fishes	Scaphirhynchus	Pallid sturgeon	Endangered	Potential
	albus			
	Etheostoma	Relict darter	Endangered	Potential
	chienense			
Birds	Sterna antillarum	Interior least tern	Endangered	Known
	Haliaeetus	Bald eagle	Delisted	Known
	leucociphalus			

KSNPC

Group		Total number of species in each group	Number of state endangered species	Number of state threatened species	Number of state Special Concern species
Terrestrial	Vascular Plants	26	7	14	5
	Snail	1		1	
	Insects	2		1	1
	Birds	16	6	5	5
	Mammals	3	1		2
	Reptiles	6	2	2	2
Aquatic	Amphibians	3	1		2
	Fishes	20	8	6	6
	Crustaceans	3	1	1	1
	Mussels	3	2		1
Totals		83	28	30	25

Natural communities in need of protection include: Bottomland hardwood forest, Bottomland marsh, Coastal plain slough, and Cypress (tupelo) swamp

Hickman County – Obion Creek State Nature Preserve is a 1,601 acre scientific research facility protecting a mosaic of wetland communities, upland slopes, and Murphy's Pond. It is located in the Obion River watershed and is part of one of the largest remaining tracts of wetland in Kentucky. This nature preserve is located approximately 8 miles east of the corridor. BMP to prevent siltation downstream will have to be in place to protect this resource.

USFWS

Group	Species	Common Name	Legal Status	Occurrence
Mammals	Myotis sodalis	Indiana bat	Endangered	Known
Mussels	Potamilus capax	Fat pocketbook	Endangered	Potential
	Lampsilis abrupt	Pink mucket	Endangered	Known
Fishes	Scaphirhynchus	Pallid sturgeon	Endangered	Known
	albus			
	Etheostoma	Relict darter	Endangered	Known
	chienense			
Birds	Sterna antillarum	Interior least tern	Endangered	Known
	Haliaeetus	Bald eagle	Delisted	Known
	leucociphalus			

KSNPC

Group		Total number of species in each group	Number of state endangered species	Number of state threatened species	Number of state Special Concern species
Terrestrial	Vascular Plants	24	7	10	7
	Snail	1		1	
	Insects	2		2	
	Birds	11	3	3	5
	Mammals	4	2		2
	Reptiles	5	1	1	3
Aquatic	Amphibians	4	1		3
	Fishes	17	9	5	3
	Crustaceans	4	2	1	1
	Mussels	4	3		1
Totals		46	28	23	25

Natural communities in need of protection include: Bottomland hardwood forest, Shrub swamp, Coastal plain slough, and Cypress (tupelo) swamp

Graves County – Bayou du Chien River Drainage (Hickman and Graves Counties) – The relict darter is endemic to this drainage and is listed as endangered by the USFWS. The current project bisects known relict darter habitat in the Bayou du Chien, roughly 5,200 feet downstream of the primary breeding area, Jackson Creek, for the species. Given the relict darter's limited distribution and apparent dependence on one spawning area (Jackson Creek), the relict darter is extremely vulnerable to anthropogenic activities (Warren et al. 1994). This entire drainage is listed as sensitive waters by the KYTC. **Panther Creek** is listed as a sensitive water area. Panther Creek is considered an exceptional water and reference reach for Graves County.

USFWS

Group	Species	Common Name	Legal Status	Occurrence
Mammals	Myotis sodalis	Indiana bat	Endangered	Potential
Fishes	Etheostoma chienense	Relict darter	Endangered	Known

KSNPC

Group		Total number of species in each group	Number of state endangered species	Number of state threatened species	Number of state Special Concern species
Terrestrial	Vascular Plants	13	4	3	6
	Insects	2	1	1	
	Birds	3	2		1
	Mammals	2	1		1
	Reptiles	4		2	2
Aquatic	Amphibians	4		1	3
	Fishes	17	12	4	1
	Crustaceans	3	2	1	
	Mussels	2	1		1
Totals		50	23	12	15

No natural communities in need of protection include:

Marshall County – Clarks River National Wildlife Refuge is located along the Clarks River north of Benton. It is the first national refuge established wholly within the state. The majority of this refuge is located west of the corridor but there is a small tract of land located in the corridor at the Clarks River Bridge crossing.

USFWS

Group	Species	Common Name	Legal Status	Occurrence
Mammals	Myotis sodalis	Indiana bat	Endangered	Potential
	Myotis grisescens	Gray bat	Endangered	Potential
Mussels	Pleurobema clava	Clubshell	Endangered	Known
	Lampsilis abrupt	Pink mucket	Endangered	Known
	Plethobasus	Orangefoot pimpleback	Endangered	Known
	cooperianus			
	Obovaria retusa	Ring pink	Endangered	Known
	Cumberlandia	Spectaclecase	Candidate	Potential
	monodonta			
	Plethobasus cyphyus	Sheepnose	Candidate	Potential
	Cyprogenia stegaria	Fanshell	Endangered	Potential
Plants	Apios priceana	Price's potato-bean	Threatened	Potential
Birds	Sterna antillarum	Interior least tern	Endangered	Known
	Haliaeetus	Bald eagle	Delisted	Known
	leucociphalus			

KSNPC

Group		Total number of species in each group	Number of state endangered species	Number of state threatened species	Number of state Special Concern species
Terrestrial	Vascular Plants	22	9	7	6
	Insects				
	Birds	10	3	3	4
	Mammals	2	1		1
	Reptiles	5		2	3
Aquatic	Amphibians	4			4
	Fishes	11	4	4	3
	Snail	3			3
	Crustaceans	1		1	
	Mussels	12	10	1	1
Totals		60	24	15	21

Natural communities in need of protection include: Bottomland hardwood forest, Acidid sub-xeric forest, and Xerophydric flatwoods

Livingston County – Land Between the Lake Natural Recreational Area is located on each side of the I-24 corridor.

USFWS

Group	Species	Common Name	Legal Status	Occurrence
Mammals	Myotis sodalis	Indiana bat	Endangered	Known
	Myotis grisescens	Gray bat	Endangered	Known
Mussels	Pleurobema clava	Clubshell	Endangered	Known
	Lampsilis abrupt	Pink mucket	Endangered	Potential
	Plethobasus	Orangefoot pimpleback	Endangered	Known
	cooperianus			
	Obovaria retusa	Ring pink	Endangered	Known
	Cumberlandia	Spectaclecase	Candidate	Potential
	monodonta			
	Plethobasus cyphyus	Sheepnose	Candidate	Potential
	Potamilus capax	Fat pocketbook	Endangered	Known
	Pleurobema plenum	Rough pigtoe	Endangered	Potential
Plants	Apios priceana	Price's potato-bean	Threatened	Known
Birds	Sterna antillarum	Interior least tern	Endangered	Known
	Haliaeetus	Bald eagle	Delisted	Known
	leucociphalus			
Reptiles		Copperbelly water snake	SCA	

SCA = Species covered by a State Conservation Agreement

KSNPC

KON C					
Group		Total number of species in each group	Number of state endangered species	Number of state threatened species	Number of state Special Concern species
Terrestrial	Vascular Plants	27	13	7	7
	Non-vascular plant	1	1		
	Insects	1		1	
	Birds	11	1	3	7
	Mammals	3	2	1	
	Reptiles	4		1	3
Aquatic	Amphibians	4			4
	Fishes	12	4	4	4
	Snail	4			4
	Crustaceans	3	1	2	
	Mussels	14	11	2	1
Totals		84	33	21	30

Natural communities in need of protection include: Limestone slope glade, Sandstone barrens (open woodland), and Shawnee Hills sandstone glade

Lyon County

USFWS

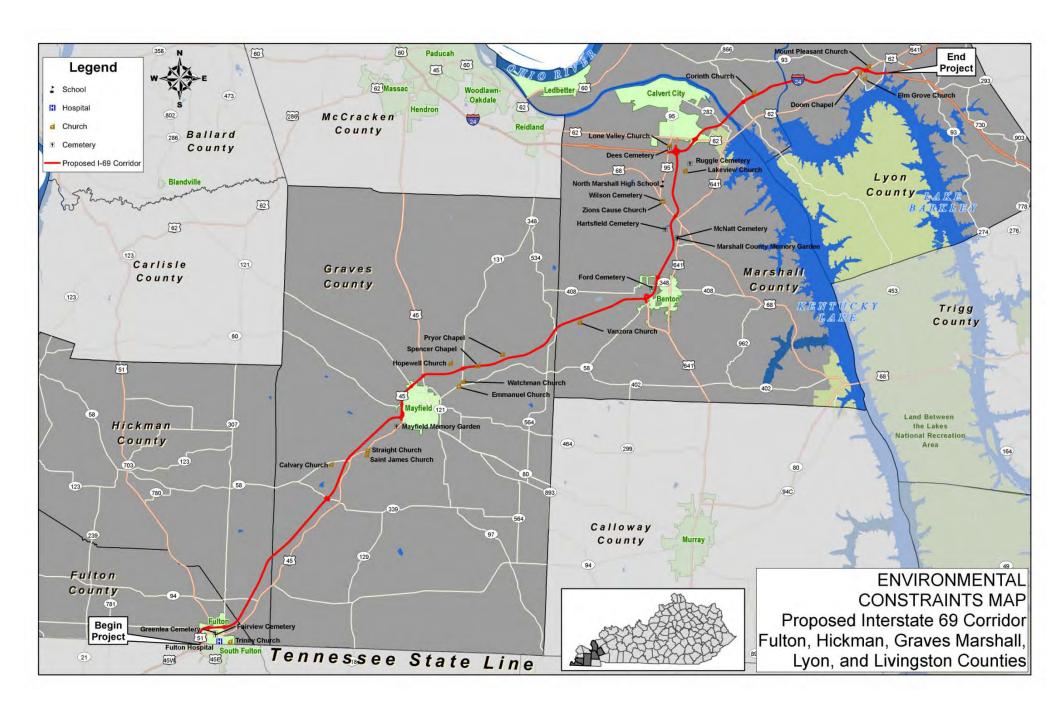
Group	Species	Common Name	Legal Status	Occurrence
Mammals	Myotis sodalis	Indiana bat	Endangered	Potential
	Myotis grisescens	Gray bat	Endangered	Potential
Mussels	Pleurobema clava	Clubshell	Endangered	Known
	Lampsilis abrupt	Pink mucket	Endangered	Known
	Plethobasus	Orangefoot pimpleback	Endangered	Known
	cooperianus			
	Obovaria retusa	Ring pink	Endangered	Known
	Plethobasus cyphyus	Sheepnose	Candidate	Potential
	Cyprogenia stegaria	Fanshell	Endangered	Known
Plants	Apios priceana	Price's potato-bean	Threatened	Known
Birds	Sterna antillarum	Interior least tern	Endangered	Known
	Haliaeetus	Bald eagle	Delisted	Known
	leucociphalus			
Insects	Nicrophorus	American burying	Endangered	Considered
	americanus	beetle		extirpated

SCA = Species covered by a State Conservation Agreement

KSNPC

Group		Total number of species in each group	Number of state endangered species	Number of state threatened species	Number of state Special Concern species
Terrestrial	Vascular Plants	24	9	8	7
	Insects	1	1 extirpated		
	Birds	11	4	3	4
	Mammals	1			1
	Reptiles	5		1	4
Aquatic	Amphibians	1			1
	Fishes	5	1	1	3
	Snail	1			1
	Mussels	11	10	1	
Totals		60	24	14	21

No natural communities in need of protection





Local Officials Meeting Minutes

I-69 Strategic Corridor Planning Study, Fulton to Eddyville, KY
Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties
Purchase Area Development District Office
Mayfield, Kentucky
2:00 p.m. (CST), November 15, 2010

This meeting was held with local officials from Fulton, Hickman, Graves, Marshall, Livingston, Lyon, and surrounding counties to discuss the I-69 Strategic Corridor Planning Study from Fulton to Eddyville, KY. The project was introduced and the attendants were able to discuss potential project issues and provide input on specific local concerns. Those in attendance included:

Dan Voegeli Mayor of Fulton

Kenney Etherton City Manager of Fulton
Lee McCollum Mayor of Kuttawa
John C. Mahre Christian County

William M. "Bill" Corum Madisonville-Hopkins County EDC

Ken Winters State Senator – District 1

Will R. Coursey

State Representative – District 6

Mike Miller

Marshall County Judge Executive

Larry Kelley Ballard County Chamber of Commerce

Greg Terry Carlisle County Judge Executive
Vickie Viniard Ballard County Judge Executive

John Anderson Purchase ADD Staff

Marty Wiles Field Representative for Senator McConnell

Mickey Beck Clinton City Councilman

Tony Smith Graves County Judge Executive

Arthur Byrn Mayor of Mayfield
Jim LeFevre KYTC – District Office
Jill Asher KYTC – Central Office
Tonya Higdon KYTC – Central Office
Steve Ross KYTC – Central Office
Shane Tucker KYTC – Planning

David Martin KYTC – Highway Design Stacey Courtney Purchase ADD Staff Mark Davis Purchase ADD Staff

Will Conkin Palmer Engineering
David Lindeman Palmer Engineering
Gary Sharpe Palmer Engineering
Chuck Wood Palmer Engineering

Lee Kileman

Bernardin, Lochmuller, and Associates

David Isley

Bernardin, Lochmuller, and Associates

As the attendants arrived, they were asked to sign the attendance sheet and were given a project brochure and questionnaire. Several exhibits were on display for the local officials to look at. KYTC and consultant staff personnel were on hand to answer any questions at this time. Copies of the questionnaire, project brochure, and exhibits are attached to the end of Appendix C.

The meeting was opened by Jim LeFevre of KYTC welcomed the attendants and provided an introduction to the project. David Lindeman of Palmer Engineering then gave a project overview and presented a slide show to the attendants. The presentation included background information on previous studies, scope of work for this project, overview of existing conditions, and discussion of interstate design standards. The slides from the presentation are attached to the end of Appendix C.

Following the slide show presentation, the floor was opened for attendants to ask question and make comments concerning the information presented. Questions from attendants (<u>underlined</u>) and responses from the consultants and KYTC staff during the meeting included:

Will there be work completed on the Mayfield Bypass?

In the current state, the median does not meet interstate standards. The median width is acceptable, but the median type does not meet interstate standards and separation will have to be provided with a concrete barrier.

• Will the power line at the Purchase Parkway and the I-24 interchange be affected by the new interstate design?

It is not known what exactly will be affected at the I-24 and Purchase Parkway interchange, but it is believed that the power line will not be affected.

• What is the time frame?

There is not a set time frame for final designation of the Purchase Parkway as I-69. Due to the fact that there is no longer an interstate program, funding will have to come from normal federal and state funds. KYTC is going to take an incremental approach to final designation of I-69. For example, KYTC has identified projects on the Parkways where the roadway is known not to meet interstate standards. The toll booth plazas are known interchanges that do not meet interstate standards. The KY 348 interchange in Benton has been let for design. KYTC is working with the FHWA to identify design exceptions and variances for the Wendell H. Ford and Edward T. Breathitt Parkways. These exceptions and variances will set precedence for designating the Purchase Parkway as I-69. KYTC has many larger projects that are consuming the current budget, including the Kennedy Interchange project, the Louisville Bridges project, the Lake Bridges project, and the Milton Madison Bridge project. In addition, the transition of the parkways to an interstate designation has to begin at an existing interstate and end at a logical terminus. It is believed that the first segment to be completed will be from I-24 east towards Edward T. Breathitt Parkway on the Wendell H. Ford Parkway.

Are any preliminary costs available?

There are not any preliminary cost available at this time. The final report will provide a planning level cost estimate.

• Can you expound upon the issues at the Tennessee state line and Fulton?

The current Kentucky project will extend to the state line because the width is available to do so. Tennessee has currently stopped their I-69 design approximately two (2) miles south of the state line. There is currently no coordination with Tennessee. It is presumed that there will be coordination at the state line once Tennessee begins to look seriously at making the connection.

• Will a median barrier be required?

A Jersey style concrete barrier will probably be used instead of a cable barrier on the Mayfield Bypass. The rest of the Purchase Parkway meets the minimum interstate median width.

What are the current plans for the I-24 interchange?

Yes, a fully-directional interchange would be required, but specifics are not yet known. KYTC is looking at providing an interim solution for short-term conversion to I-69. In the future, the I-24 interchange will probably need to be redesigned to a fully-directional interchange. KYTC is looking at providing an interim solution for short-term conversion to I-69.

• Are there other known interchanges where improvements will be required?

The former toll booth plazas at Benton and Wingo are known to not meet interstate standards.

How will any bridge height problems be fixed?

The bridge can be torn down and replaced, the existing bridge can be raised, or the pavement under the bridge can be lowered as long as there are no drainage issues.

What is the plan for the approach?

Kentucky has to work from the existing interstate out to get interstate designation on work that has been previously completed. The redesign and improvements to the I-24 interchange will be a big job. At this interchange all ramps may be replaced with two-lane ramps. Improvement can be completed anywhere on the Purchase Parkway, but the I-69 designation cannot be achieved until the roadway is contiguous with I-24.

• What effect will the progress for the I-69 connection in Henderson across the Ohio River have on the project?

The lack of bridges in Henderson will probably not have an effect on this project.

Appendix C. Public Meeting Minutes and Material

- 1. Public Meeting Minutes, Graves County, November 15, 2010
- 2. Public Meeting Questionnaire Handout
- 3. Public Meeting Questionnaire Summary
- 4. Public Meeting Handout
- 5. Public Meeting Exhibits

Public Involvement Meeting Minutes

I-69 Strategic Corridor Planning Study, Fulton to Eddyville, KY Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties

Purchase Area Development District Office Mayfield, Kentucky 5:30 p.m. to 7:30 p.m. (CST), November 15, 2010

A public involvement open house meeting was held on Monday, November 15, 2010 from 5:30 p.m. to 7:30 p.m. (CST) at Purchase Area Development District (PADD) Office, 1002 Medical Drive, Mayfield, Kentucky 42066. The following Kentucky Transportation Cabinet (KYTC) and consultant staff personnel were in attendance:

Jill Asher KYTC – Central Office
Tonya Higdon KYTC – Central Office
Steve Ross KYTC – Central Office
Shape Tucker KYTC – Planning

Shane Tucker KYTC – Planning

David Martin

Jim LeFevre

Will Conkin

David Lindeman

Gary Sharpe

Chuck Wood

KYTC – Highway Design

KYTC – District Office

Palmer Engineering

Palmer Engineering

Palmer Engineering

Palmer Engineering

Lee Kileman Bernardin, Lochmuller, and Associates

Employees of PADD and Pennyrile Area Development District (PEADD) were also in attendance. A total of 59 people registered their attendance during the public involvement open house (this number excludes those 9 individuals listed above and the employees of PADD and PEADD). Two members of the media, a local newspaper and a television news reporter, were on-hand to cover the meeting.

As attendees arrived, they were asked to sign-in and were given a project brochure and questionnaire. Attendees were invited to view the exhibits and ask questions to KYTC and consultant staff personal. Copies of the questionnaire, project brochure, and exhibits are attached to the end of this appendix.

After a time of informal gathering, Jim LeFevre of KYTC formally welcomed all attendants and provided an introduction to the project. David Lindeman of Palmer Engineering then gave a project overview and presented a slide show to the attendants. The presentation included background information on previous studies, scope of work for this project, overview of existing conditions, and discussion of interstate design standards. The slides from the presentation are attached to the end of this appendix.

Following the slide show presentation, the floor was opened for attendants to ask question and make comments concerning the information presented. Questions from attendants (<u>underlined</u>) and responses from KYTC and consultant staff during the meeting included:

What are the bridge rehabilitation requirements?

There are four (4) bridges that do not meet the interstate standards. The bridge can be torn down and replaced or raised to the appropriate elevation. The pavement below the bridge can also be lowered to obtain minimum clearance if the drainage issues can be addressed.

• Will seismic retro-fitting be a part of the rehabilitation?

It is currently unknown if seismic retro-fitting will be a part of the raising of bridges on this project, but would be included as a part of new bridges.

Will work have to be completed to the Purchase Parkway and I-24 interchange? Yes, a fully-directional interchange would be required, but specifics are not yet known. KYTC is looking at providing an interim solution for short-term conversion to I-69.

• Is the proposed alignment totally along with the existing parkway?

Yes, with some localized exceptions. For instance, the former toll booth interchanges will have to be redesigned, which will require some right-of-way acquisition. Also, the I-24 interchange will probably require some right-of-way work. The area near Fulton will have to be studied further to incorporate the existing road network at the Purchase Parkway with the integration of I-69.

What will happen at the Tennessee/Kentucky state line?

Currently KYTC is not sure how the situation will be handled. The area around the state line is very busy with many roadways and access points. KYTC will have to work with Tennessee Department of Transportation (TDOT) to find a solution. Tennessee has recently elected a new governor and when the administration changes, projects tend to sit in limbo until everything is settled. Kentucky will have to wait until Tennessee is ready to talk about the I-69 connection.

What is the timing of completion?

The project must first get into the 6-Year Highway Plan. The 6-Year Highway Plan already has about 15 years worth of projects in it. KYTC is also already planning to build several other bridges throughout the state. The timing of the funding for I-69 is unknown. KYTC hopes to identify and proceed in incremental steps along the Purchase Parkway. KYTC currently has one interchange on the Purchase Parkway that does not meet interstate standards in the design phase A roadway cannot get interstate designation on sections that do not connect to an existing interstate and stop at a logical terminus.

- Why is Tennessee so far ahead of Kentucky in terms of the construction of I-69?
 KYTC can't say why Tennessee is ahead of Kentucky in terms of construction of I-69, but does know why Indiana has proceeded and talked about leasing the toll way.
- Will there be federal funds to build the interstate if the "interstate program is over?"

 Yes, but it will probably have to come from federal funds allotted to Kentucky, not from a new or separate funding source.
- What happens if Kentucky and Tennessee disagree about how I-69 will meet up at the state line?

The states will have to work together to find a solution.

 Will the railroad track bridge have to be removed for the new interchange design at KY 348 in Benton?

It is not yet known if the railroad track bridge will have to be removed for the new interchange design at KY 348 in Benton. A final decision has not yet been made and may be subject to funding. The new interchange will not be built until 2013 or 2014. The new interchange will need design, right-of-way acquisition, utility relocation, etc. It is currently in the design phase.

- Will the number of current interchanges in Fulton change?
 Currently, KYTC does not intend to remove or add any interchanges to the Purchase Parkway for the designation as I-69. Future studies will be conducted in the Fulton area to determine access to I-69.
- Has the amount of traffic on I-24 once the two interstates (I-69 and I-66) are built been looked at?

Yes, the projections have been performed out to 2040. I-66 probably will not get constructed in the foreseeable future, but the state's priorities can change especially when politics are involved.

- Will there be any realignment of the weigh station at Fulton, or will it stay the same?
 KYTC does not see how the weigh station could stay the same. If the weigh station is replaced, then it will have to be done at a more northern location and will probably involve some advanced technology that may allow for a weigh-in-motion setup.
- Is there a website where the public can track developments for the project?
 KYTC will add it to the Division of Planning portion of the KYTC website. The presentation shown today will be added to the PADD website.
- What is the time frame for purchasing right-of-way for the KY 348 interchange in Benton?

There is not a set schedule for the purchase of right-of-way for the KY 348 interchange in Benton. Property owners may use their property any way they wish. Construction is not expected within the next two years.

At the close of the meeting attendants could turn in any completed questionnaires or were given the option of mailing them back by December 1, 2010. A total of 26 public comment questionnaires were completed at the meeting. An additional 7 public comment questionnaires were received from individuals in attendance at the meeting at a later date.

The meeting closed at 7:30 p.m. (CST).

QUESTIONNAIRE

November ____, 2010

I-69 Strategic Corridor Planning Study Purchase Parkway / I-24 - Fulton to Eddyville, KY Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties

(Please Print)

Name:	Phone:
Address	· ·
City, State, 2	Zip
County	
e-mail	(optional)
	n do you use the Julian M. Carroll (Purchase) Parkway? Weekly Monthly
another)?	usage considered local (travel within a county) or regional (from one county/city to Regional
	re any specific safety issues along the study area? Where and what problems exist?
•	
	ements to the corridor may include improving existing interchanges. Which e(s) do you think have the highest priority of improving?
5. Are there	e sensitive locations or issues that you know of within corridor?

Name:	November, 2010
	NNING STUDY ADDITIONAL COMMENTS we not been addressed with this questionnaire, please use the s or express concerns.
Comments:	
.	
-	

Your answers and comments will be given to the Project Team for further development of strategies, options and recommendations for improvements to the Julian M. Carroll (Purchase) Parkway and ultimately I-69. Please turn them in tonight at the registration desk or mail them by December 1, 2010 to:

Jim LeFevre, P. E.

Kentucky Transportation Cabinet – District 1 5501 Kentucky Dam Road Paducah, KY 42003

Public Questionnaire Summary

I-69 Strategic Corridor Planning Study, Fulton to Eddyville, KY Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties

Distribution of Responses by County:

Fulton	4
Graves	18
Groves	1
Hopkins	1
Marshall	1
McCracken	2
Obion (TN)	3

1. How often do you use the Julian M. Carroll (Purchase) Parkway?

Daily	9
Weekly	19
Monthly	1
Yearly	2

2. Is your usage considered local (travel within a county) or regional (from one county/city to another)?

Local	13
Regional	24

3. Any there any specific safety issues along the study area? Where and what problems exist?

Exit 14	3
Exit 21	5
Exit 43	1
Access for Emergency Vehicles	1
Fulton Exits	1
I-24/Purchase Parkway Interchange	3
Lighting	1
Mayfield Bypass	1
Minimal Needed	1
Old Toll Both	1
Ramp and Taper Length	2
Short Access Ramps in Mayfield	1
West Broadway (KY 80)	1

4. Improvements to the corridor may include improving existing interchanges. Which interchange(s) do you think have the highest priority of improving?

Calvert City (I-24)	1
Exit 0	6
Exit 14	5
Exit 21	7
Exit 22	2
Exit 24	2
Exit 43	6
Fulton	3
I-24/Purchase Parkway Interchange	4
Mayfield (south)	1

5. Are there sensitive locations or issues that you know of within the corridor?

Cell Tower at Exit 14	1
Exit 21	2
Exit 24	1
Exit 43	1
Exit 52	1
Guardrails at Exit 14 and Mayfield Bypass	2
KY 166 Curve	1
Emergency Vehicle Access from Mile	
Marker 2 to 9	1
Old Toll Booth	1
State Line	1

Additional Questionnaire Questions and Comments:

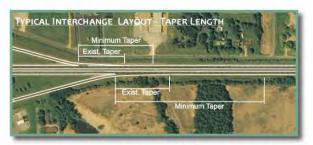
- Why was stimulus funding not used for this project?
- A man was killed because he missed Exit 21.
- This project will improve safety and open up the opportunity for new commerce.
- Kentucky stands to gain much from this investment.
- When and how will the new KY 80 tie to the Mayfield Bypass southwest of Mayfield?
- Try to avoid business disruption, utility relocation, and traffic congestion during construction.
- I request widening of connector from 121-Bypass to US 45.
- I hope I-69 can be moved forward as quickly as possible and using as much existing roadway as possible.
- I am concerned about the impact the new corridor will have on existing improvements along the south-bound leg of the interstate near the Mayfield-Fulton exit.
- Traffic is funneled down to one lane in a curve and then widens back to four lanes

- Let's get started!
- Consider using noise walls in residential areas near road.
- KY 58/80 needs to be improved significantly on both sides. We need curb and gutter and better lighting.
- Good informational session!
- I have been on the board for many years and the time for planning is over.
- We want to have the necessary changes made to open up western Kentucky and put people back to work.
- Turn this road into a toll road.
- The guardrails are too close to traffic at the Mayfield Bypass exit and Exit 14 (Wingo).
- I'm excited about this project. The sooner it becomes I-69 the better.
- At Exit 14, traffic comes from both directions when entering and exiting. The guardrails on the ramp are too close to the traffic.
- The Obion County Commission would like to see the I-68 project enter into Tennessee at the present location.
- We are a concerned fire department in southern Graves County (Water Valley). We cover approximately eight to nine miles of what is going to be I-69. Our concern is the unavailability of an entrance/exit ramp near our station. At the present time we must drive to Fulton or Wingo to access the Parkway. This is a seven to 10 mile drive just to get to the Parkway and does not count travel time to the scene. People on the new interstate deserve a quick response no matter what section of I-69 they are traveling. We are presently looking at a minimum approximate response time of 15 minutes. If the fire or motor vehicle accident is at the end of our district it could be 20 minutes or longer. Our biggest concern is safety for the public driving on our roads. We are sure you would agree. We would greatly appreciate your consideration of an entrance ramp to enable us to respond and serve more efficiently and effectively.
- The entrance and exit ramps at The Wingo exit need to be lengthened.
- I am concerned with the area between the mile marker 2 and the mile marker 9. The Water Valley Fire Department responds to this area and in order for them to get to an emergency call they have to go all the way to Fulton at Exit 2. If there is a wreck or someone is entrapped in a vehicle that is on fire that is a very long trip for the fire department to make. I know if your family was having an emergency at the 8 mile marker and it took the fire department 20 to 30 minutes to get there you wouldn't be too happy. They are a volunteer department and have to drive from a location to the fire department, so that extends the response time as well. I feel the access of a ramp would greatly help the fire department and the safety of the drivers on I-69. I know there are bridges over the Parkway for Highway 1529 and Highway 1283 just outside of Water Valley. It would be greatly appreciated if you could consider one of those overpasses for an entrance.

PROJECT DESCRIPTION

The Kentucky Transportation (KYTC) has undertaken a Strategic Corridor Planning Study for a portion of proposed Interstate 69 (I-69). I-69 is proposed to extend from the Mexican border in Texas to the Canadian border in Michigan. This project involves a study of the Julian M. Carroll (Purchase) Parkway north from the Tennessee state line at Fulton through Fulton, Graves, Marshall, Livingston, and Lyon Counties to the interchange with I-24. The study corridor continues east on I-24 to just west of the Wendell H. Ford Parkway and I-24 interchange. Evaluation of the remaining segments of I-69 in Kentucky have been addressed by another study. The primary purposes of this project are:

- to review the existing conditions along the Julian M. Carroll (Purchase) Parkway and I-24 to identify locations that do not meet current highway design guidelines for Interstate routes
- b to evaluate the degree to which these guidelines are not met
- to identify options for making improvements to address identified deficiencies
- to make recommendations regarding the suitability of routing this segment of I-69.



Recommendations for improving taper lengths to meet minimum interstate standards will be included in the report.

PROJECT FOCUS

This project will focus on evaluating existing conditions in the context of the following:

- roadway geometry (lane, shoulder, and median widths; horizontal and vertical clearance)
- bridge geometry, structural condition, load rating, and functional attributes
- ▶ interchange geometry and access control.

PROJECT SCHEDULE

Notice to Proceed April 2010

Complete Inventory of Existing Conditions August 2010

Public Meeting November 2010

Interdisciplinary Meeting January 2011

Final Report February 2011



PROCEDURE FOR SUBMITTING COMMENTS

Representatives of the Kentucky Transportation Cabinet and their engineering consultants are available to answer questions you may have regarding this project. In addition, exhibits and displays are available to assist you in understanding the various facets of this project. You are encouraged to make an official comment that will be incorporated into the project summary.

To make a written statement, complete one of the comment sheets provided and leave it tonight with one of the representatives or mail it by December 1, 2010, to the address listed below.

Jim LeFevre, P.E.
Department of Highways, District 1
5501 Kentucky Dam Road
Paducah, KY 42003

I-69 Strategic Corridor Planning Study Fulton to Eddyville, KY

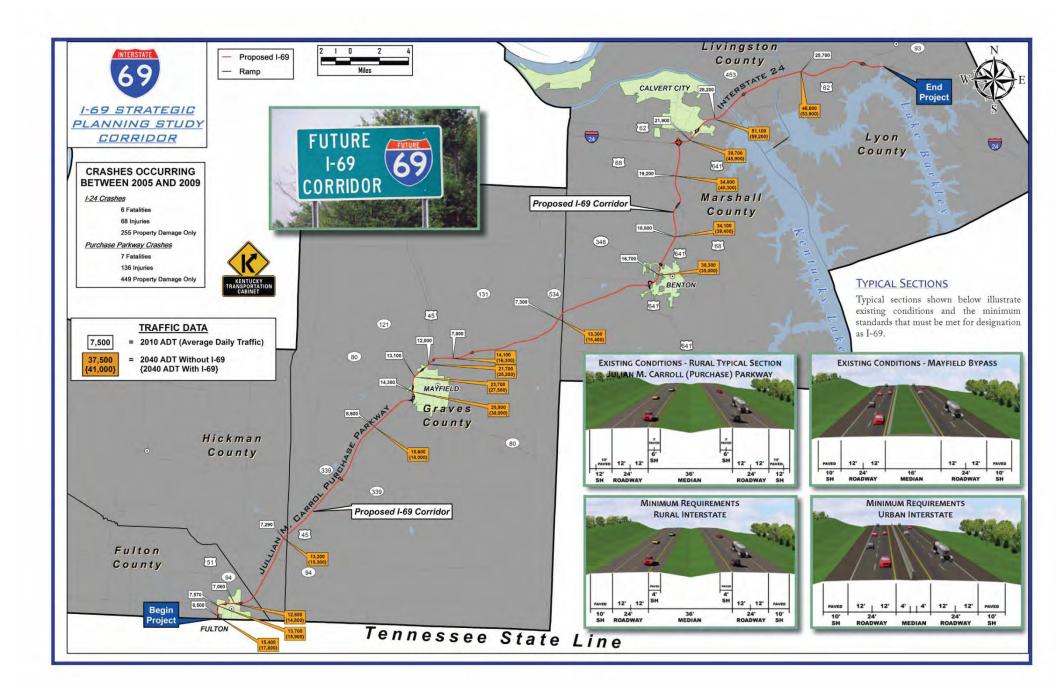


November 15, 2010 5:30 p.m. - 7:30 p.m. CT

Purchase Area Development District (PADD) 1002 Medical Drive Mayfield, KY 42066









I-69 Strategic Planning Corridor STUDY ISSUES

The Project Corridor will be studied to identify needed changes to meet Interstate Highway Standards.

Examples of items that need to be addressed are shown below:







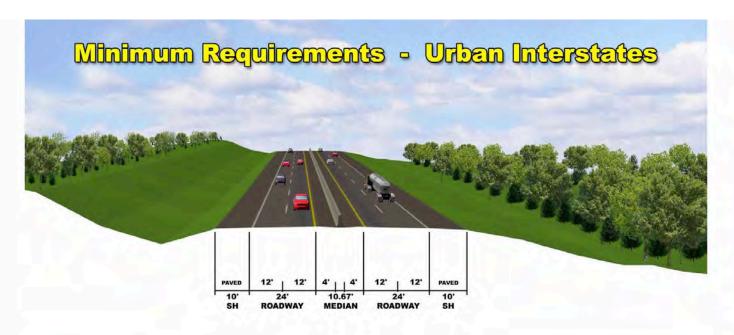


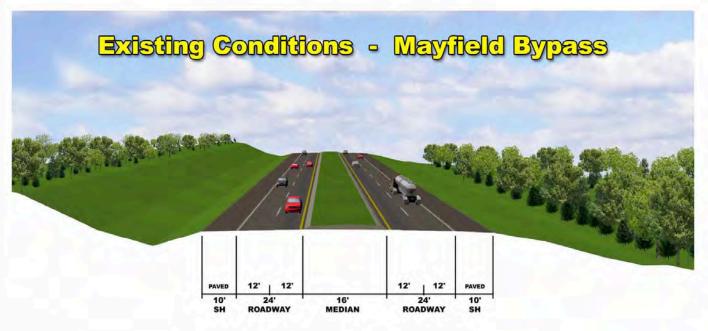


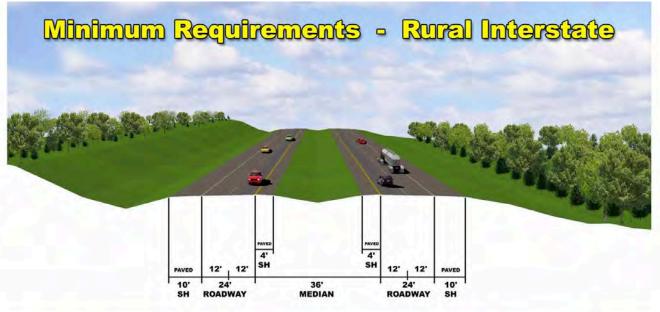
- ► Acceleration & Deceleration Taper Lengths
- ► Ramp Curvature
- ► Access Control

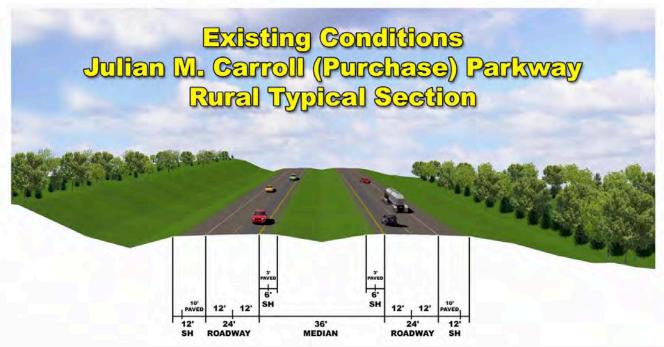


- ► Rural 36' Minimum
- ► Urban 10' Minimum



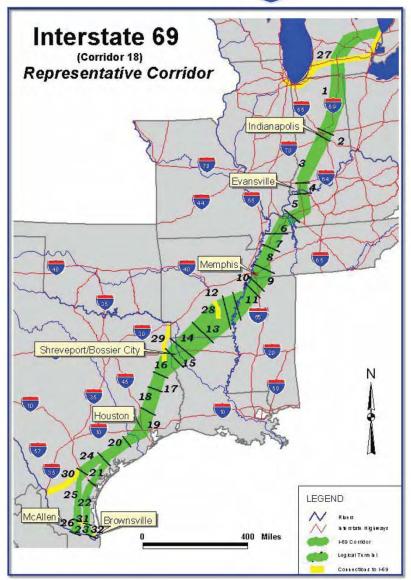


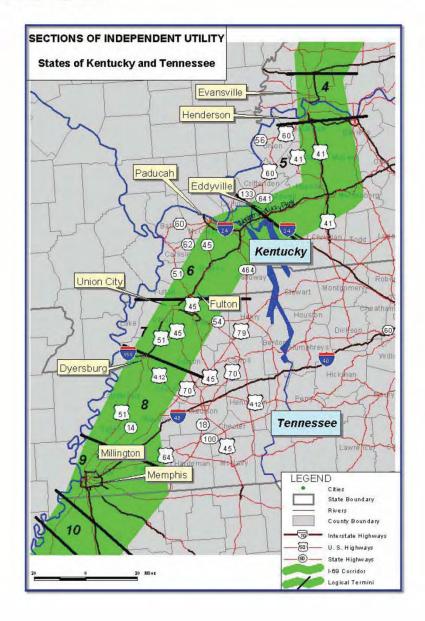


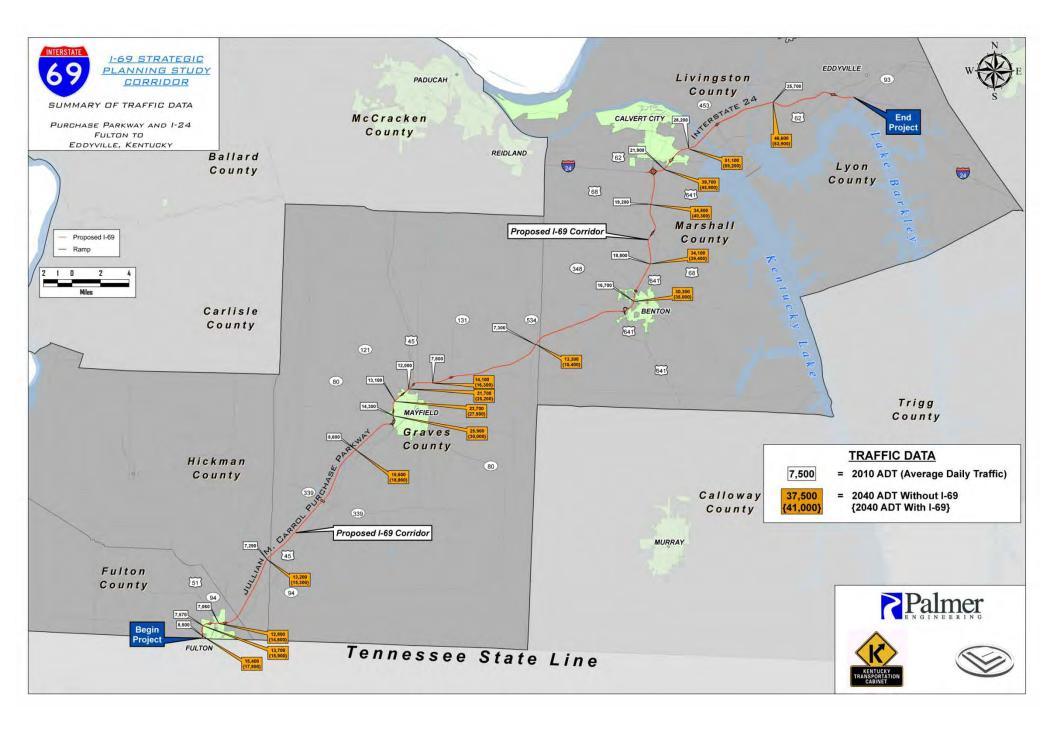


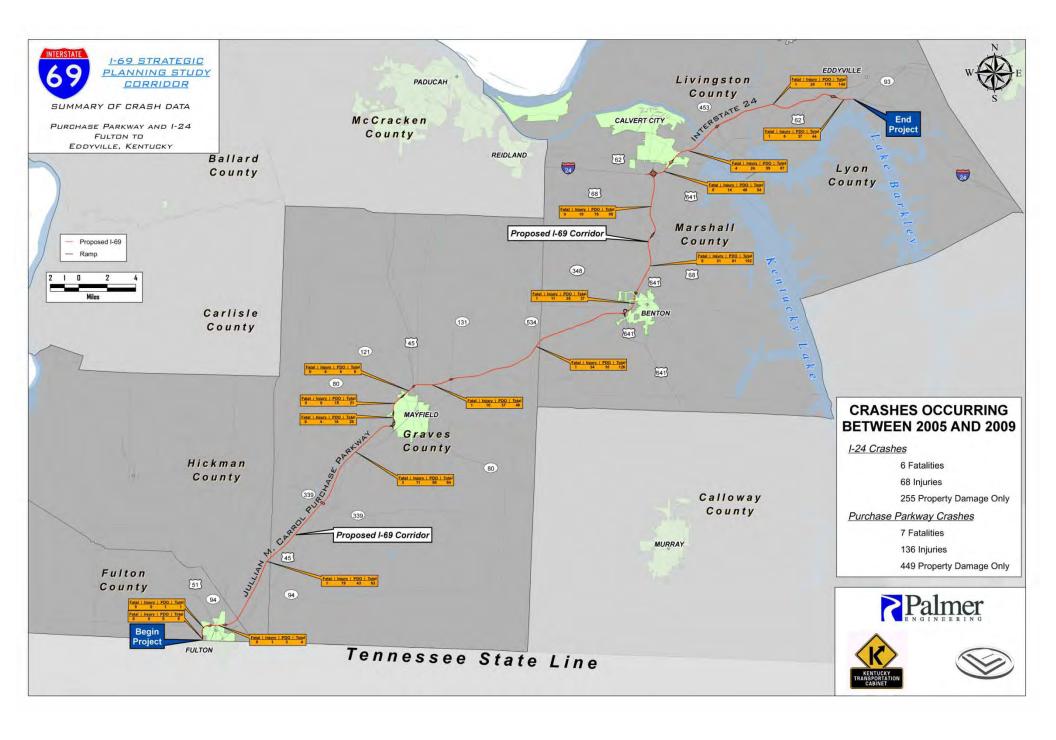


I-69 CORRIDOR











I-69 Strategic Planning Corridor Study: Fulton To Eddyville, KY

Fulton, Hickman, Graves, Marshall, Livingston, Lyon Counties

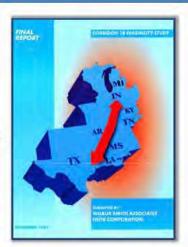
Public Meeting November 15, 2010





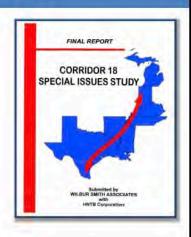
Project Background: Corridor 18 Feasibility Study

- Produced information regarding cost, economic efficiency, impacts on economic development, financial viability and other relevant features of this large scale highway project.
- Completed November 1995



Project Background: Corridor 18 Special Issues Study

- Redefined the corridor from Indianapolis, IN to the Lower Rio Grande Valley
- □ Completed July 1997



Project Background:

Strategic Corridor Planning Study for I-69: Eddyville to Henderson, KY

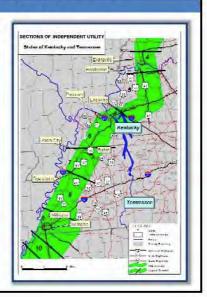
- □ Segment of Independent Utility (SIU) 5
- Overview of Existing Conditions
- Wendell H. Ford and Edward
 T. Breathitt Parkways
- □ Completed March 2005



This Project: Strategic Planning Study

Purchase Parkway / I-24
Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties

- □ SIU 6
- □ Beginning Tennessee State
 Line in Fulton County
- □ Ending West of I-24 and Western Kentucky Parkway Interchange in Lyon County



Strategic Planning Study: 1-69 Corridor Purchase Parkway / 1-24

Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties

□ Study Beginning - April 2010



Anticipated Completion - February 2011

Strategic Planning Study: 1-69 Corridor Purchase Parkway / 1-24

Fulton, Hickman, Graves, Marshall, Livingston, and Lyon Counties

- □ Scope of Work
 - Inventory existing conditions
 - Define Interstate criteria
 - Determine and evaluate deficiencies
 - Identify options and strategies for needed improvements
 - Develop recommendations and potential cost
 - Document findings

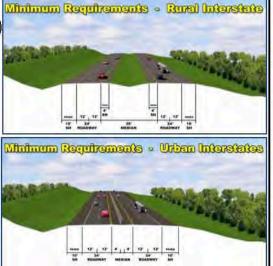
Strategic Planning Study: 1-69 Corridor Interstate Design Standards

- □ Fully Controlled Access
 - At Ramp Terminals
 - Minimum 100 foot urban
 - Minimum 300 foot rural
- □ Design Speed
 - 70 MPH Rural
 - 50 MPH Urban
- □ Four Lanes
 - 12 feet wide



Strategic Planning Study: 1-69 Corridor Interstate Design Standards

- Shoulder Widths (paved)
 - Inside 4 foot min
 - Outside 10 foot min
- □ Median
 - Rural 36 foot min
 - Urban 10 foot min



Strategic Planning Study: 1-69 Corridor Interstate Design Standards



Minimum Horizontal Curvature

- Rural 1810 foot radius
- Urban 758 foot radius
- Minimum Stopping Sight Distance
 - Rural 730 feet
 - Urban 425 feet

Strategic Planning Study: 1-69 Corridor Interstate Design Standards

Bridges

- All lanes and shoulders at least 16 foot vertical clearance
- Full paved shoulder width
- Crashworthy barrier railing
- Structurally adequate
- □ Sign Trusses 17 foot vertical clearance



Strategic Planning Study: 1-69 Corridor Interstate Design Standards

□ Interchange

- Provide all traffic movements
- Spacing between interchanges
 - Minimum 1 mile Urban
 - Minimum 3 mile Rural
- Adequate Acceleration/ Deceleration Tapers



Strategic Planning Study: 1-69 Corridor Overview of Existing Conditions



- □ Traffic 2010 Vehicles per day (vpd)
 - Purchase Parkway
 - Ranges from 7,060 vpd to 19,200 vpd
 - · I-24
 - Ranges from 21,900 vpd to 28,200 vpd

Strategic Planning Study: 1-69 Corridor Traffic Forecasts



- □ Traffic 2040 Vehicles per day (vpd)
 - Purchase Parkway
 - Ranges from 14,800 vpd to 40,300 vpd
 - · I-24
 - Ranges from 45,900 vpd to 59,200 vpd

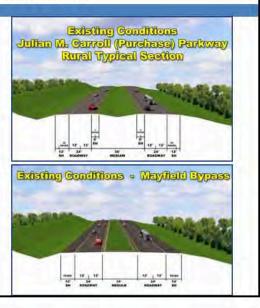
Strategic Planning Study: 1-69 Corridor Overview of Existing Conditions

- □ Crash History (2005-2009)
 - Purchase Parkway
 - 7 Fatalities
 - 136 Injuries
 - 449 Property Damage Only
 - · I-24
 - 6 Fatalities
 - 68 Injuries
 - 255 Property Damage Only



Strategic Planning Study: 1-69 Corridor Overview of Existing Conditions

- □ Roadway Geometry
 - Lane Widths
 - 12 feet wide
 - Shoulder Widths
 - Inside 0 6 feet
 - Outside 10-12 feet
 - Median Widths
 - Urban 16 feet
 - Rural 36 feet



Strategic Planning Study: 1-69 Corridor Overview of Existing Conditions



□ Bridges

- Width
 - 30 38 feet
- Vertical Clearance
 - 15.12 19.48 feet
- Bridge Railing
 - Upgrade to current standards

Strategic Planning Study: 1-69 Corridor Environmental Overview

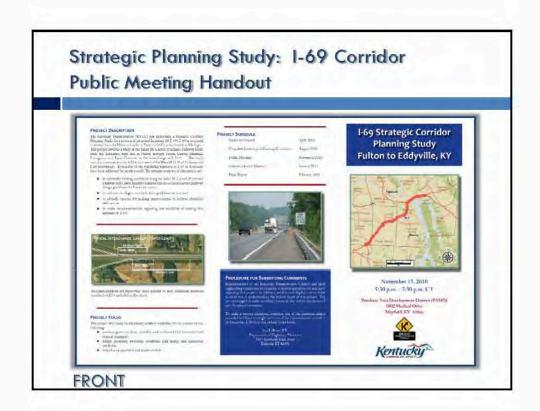
- Overview of critical environmental (NEPA) considerations
- □ Environmental Justice
 - Review of the Socioeconomic characteristics along the project area
 - 2000 U.S. Census Bureau
 - Findings of no impact to the studied area populations since corridor is with-in existing Right of Way



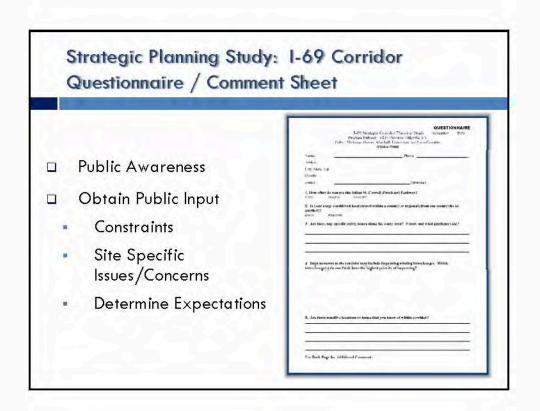
Strategic Planning Study: 1-69 Corridor Geotechnical Overview

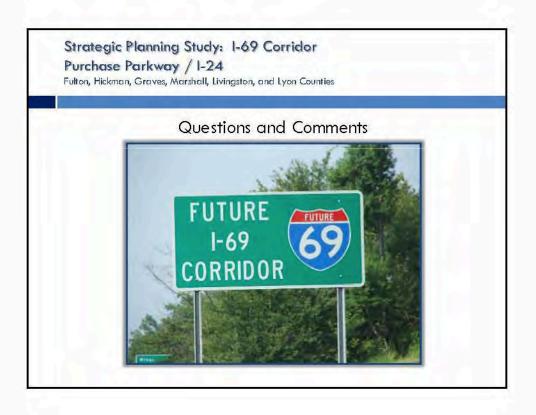


- Overview of anticipated improvements and geotechnical considerations
- To be completed in conjunction with final report



Strategic Planning Study: I-69 Corridor Public Meeting Handout TOTAL PROPERTY OF THE PROPER









I-69 Strategic Planning Corridor Study from Fulton to Eddyville, Kentucky Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties, Kentucky

Stantec Consulting Services Inc. One Team. Infinite Solutions

1409 North Forbes Road Lexington, KY 40511-2050 Tel: (859) 422-3000 • Fax: (859) 422-3100 www.stantec.com Prepared for: Palmer Engineering Winchester, Kentucky

February 14, 2011



Stantec Consulting Services Inc. 1409 North Forbes Road Lexington, KY 40511-2050 Tel: (859) 422-3000 Fax: (859) 422-3100

February 14, 2011

rpt_001_175561002

Mr. Gary Sharpe, PE, PLS Palmer Engineering 400 Shoppers Village P.O. Box 747 Winchester, Kentucky 40392-0747

Re: Report of Geotechnical Overview

I-69 Strategic Planning Corridor Study from Fulton to Eddyville, Kentucky

Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties, Kentucky

Dear Gary:

Stantec Consulting Services Inc. (Stantec) is pleased to submit this geotechnical overview for the proposed Interstate 69 corridor situated between the cities of Fulton and Eddyville, Kentucky in Fulton, Hickman, Graves, Marshall, Livingston and Lyon counties. The overview is based upon research of available published data and input from various Project Team meetings.

Palmer Engineering provided Stantec with preliminary locations for the study area. The scope of work performed and results of the overview are presented in the accompanying attachment. Stantec appreciates having the opportunity to provide these engineering services and would be happy to answer any questions and further assist you concerning this project.

Sincerely,

STANTEC CONSULTING SERVICES INC.

T. Craig Barnett, PE Geotechnical Engineer

Mark A. Litkenhus, PE Senior Principal

/rws

Adam Crace, PE Project Manager

I-69 Strategic Planning Corridor Study from Fulton to Eddyville, Kentucky Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties, Kentucky

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I-69 Strategic Planning Corridor Study from Fulton to Eddyville, Kentucky Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties, Kentucky

Prepared for:
Palmer Engineering
Winchester, Kentucky

February 14, 2011

I-69 Strategic Planning Corridor Study from Fulton to Eddyville, Kentucky Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties, Kentucky

Project Description

The Kentucky Transportation Cabinet (KYTC) is evaluating a corridor for the proposed Interstate 69 (I-69) that would include the Purchase Parkway from the Tennessee state line to the Interstate 24 (I-24) interchange, and then east along I-24 to west of the Western Kentucky Parkway. This overview will be utilized to identify possible improvements needed to bring the corridor up to current interstate standards. It is anticipated the current alignment of the Purchase Parkway will be utilized and that significant improvements will be needed at the major interchanges. In addition, it is anticipated 10 bridge structures will require some type of widening effort to meet current interstate criteria.

The Project Team prepared an environmental footprint of the area to be studied. This area incorporates portions of Fulton, Hickman, Graves, Marshall, Livingston and Lyon Counties. The proposed I-69 corridor is approximately 68 miles in length.

Scope of Work

The scope of work for this study consists of performing a geotechnical overview for the proposed corridor based upon research of available published data and Stantec's experience with highway design and construction within the region. General geotechnical and geologic characteristics of the study area have been identified and are discussed in this report. Stantec personnel, using a variety of sources, performed a literature search that included reviews of the following sources:

- Available topographic and geologic mapping of the project area published by the United States Geological Survey (USGS) and the Kentucky Geological Survey (KGS);
- The Geologic Map of Kentucky, published by the USGS and the KGS (1988);
- KYTC Geotechnical Data, published by the KGS and KYTC, http://kgs.uky.edu/kgsmap/kytcLinks.asp;
- United States Department of Agriculture, Soil Conservation Service (SCS) Soil Survey Publications for affected counties;
- Physiographic Regions, published by KGS, http://kgs.uky.edu/kgsweb; and
- KY Lakes, published by Kentucky Division of Geographic Information (GIS), http://kymartian.ky.gov

3. Physiographic and Stratigraphic Setting

3.1. Topography and Drainage

The proposed I-69 corridor for this study is located in Western Kentucky, situated on portions of ten USGS 7.5-minute topographic quadrangle maps. They are the Calvert City (1993), Briensburg (1969), Eddyville (1967, revised 1973), Hickory (1994), Westplains (1993), Oak Level (1969, revised 1987), Hardin (1969, revised 1986), Dublin (1969), Mayfield (1994) and Water Valley (1969, revised 1981) Quadrangles.

The study area is situated within the Mississippi Embayment and the Mississippian Plateaus Physiographic Regions of Kentucky. The eastern portion of the project alignment is situated within the Mississippian Plateaus Region. The Mississippian Plateaus contains gently to moderately rolling topography that has been partially influenced by karst weathering in some areas. The karst areas in the Mississippian Plateaus Region are characterized by the existence of sinkholes, ridgetop ponds, sinking streams, springs, and various subterranean channels. The remaining portions of the proposed project alignment are located within the Mississippi Embayment Region which consists of broad, flat flood plains along the Mississippi River to gently rolling uplands of low relief. The alluvial areas are relatively level. The limits of each Region is detailed in Figure 1.

Surface drainage within the region is directed towards numerous swales, ditches, creeks, and streams, including the Cumberland River, Tennessee River, Clarks River, Bayou du Chien River and Mississippi River, as well as karst features in the area. The Kentucky Lake and Lake Barkley reservoirs are located on the eastern side of the study area.

3.2. Stratigraphy

Corresponding USGS geologic quadrangles are available for the Calvert City (1968), Briensburg (1964), Eddyville (1963), Hickory (1965), Westplains (1967), Oak Level (1968), Hardin (1968), Dublin (1972), Mayfield (1965) and Water Valley (1963) Quadrangles.

Based on the various geologic mapping and literature reviewed, the proposed corridor is primarily underlain by alluvium, loess and continental deposits of the Tertiary and Quaternary age. The Mississippian Plateaus Region on the eastern end of the proposed corridor is primarily underlain by limestone, shale and possibly some sandstones. The bedrock consists of Mississippian age limestones, shales and isolated sandstones. The limestones are predominantly gray, medium to coarse grained, zones argillaceous, with fossiliferous and cherty zones. The shales are gray, clayey to silty, with calcareous zones. The sandstones are generally light to yellowish gray, and very fine to medium grained.

The Mississippi Embayment Region is predominantly underlain by alluvium, loess and continental deposits. Less common deposits include the Clayton and McNairy, Porters Creek Clay and the Coastal Plain deposits. The overburden soils are unconsolidated and easily eroded. Alluvial deposits of silt, sand, and gravel are also present along major streams and rivers in the area. The loess deposit consists of silt. The continental deposits consist of gravel, sand and clay. The Clayton and McNairy deposits consist of sand. The Porters Creek Clay consists of clay and sand. The Coastal Plain deposits consist of sand and clay.

The overburden soils are greater than 100 feet thick along the proposed corridor within the Mississippi Embayment Region. Bedrock is not mapped on the Dublin, Hardin, Hickory, Mayfield, Oak Level or Westplains Quadrangles.

3.3. Faulting in the Area

Based on USGS Geologic mapping, several unnamed faults are near the footprint studied on the eastern part of the corridor near the Western Kentucky Parkway. USGS Geologic mapping also indicates a series of concealed probable faults near the proposed roadway between Calvert City and Benton Kentucky. A series of hypothetical faults are noted between Benton and Mayfield Kentucky on the USGS Geologic mapping. Faults are not noted on the section of the proposed corridor between Mayfield and Fulton Kentucky. These faults are mapped within portions of the western portion of the studied corridor. The study area is near the New Madrid Fault Zone and discussed further in 3.5.

3.4. Soils and Unconsolidated Materials

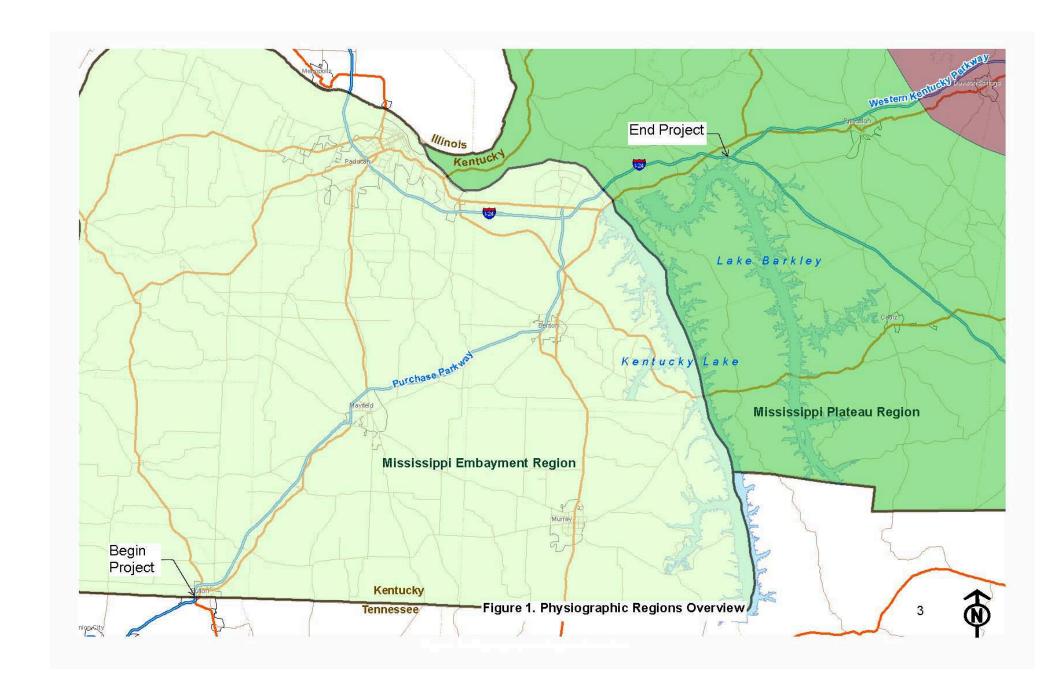
Alluvial materials are the predominate soil type along the proposed alignment within the Mississippi Embayment Region and are comprised of sands, silts, clays and gravels covering the floodplains of major streams and tributaries in the study area. Major streams within the environmental footprint include Cumberland River, Tennessee River, Clarks River, Bayou du Chien River and Mississippi River. The Kentucky Lake and Lake Barkley reservoirs are located on the eastern side of the study area.

Residual soils are the predominate soil type within the eastern portion of the study area near the Western Kentucky Parkway. Soil descriptions contained herein are based upon SCS soil surveys and on Stantec's knowledge of the study area. Soils within the Mississippian Plateaus Region along the corridor are predominantly a clayey to silty clay and range in depths from four feet to greater than twelve feet. Soils can become very thin to very deep in karst areas within a relatively short distance.

3.5. Regional Seismicity

Seismicity within the Commonwealth of Kentucky varies widely depending on location. The western portion of the state is dominated by the New Madrid and Wabash Valley source zones. In general, these zones are fairly active with many documented historical seismic events. Central and eastern portions of the state experience less frequent earthquakes because the source zones are quite distant from these areas. To assist designers in the Commonwealth of Kentucky, the KYTC began a research project in conjunction with the University of Kentucky and the Kentucky Transportation Center (KTC) in 1996. The products of this effort are documents in the publication "Source Zones, Recurrence Rates, and Time Histories for Earthquakes Affecting Kentucky", Research Report KTC-96-4, by Ron Street, et al., (1996). This document and other information available from the Kentucky Geological Survey (KGS) were reviewed in relation to the Interstate 69 Corridor.

An Earthquake Epicenters and Magnitudes Map for the Central and Eastern United States from 1568 to 1987 is presented in Figure 2. This map indicates that the I-69 Corridor area could be affected by earthquake events, particularly the New Madrid Seismic Zone (NMSZ). The NMSZ lies within the Central Mississippi Valley, extending from northeast Arkansas, southeast Missouri, western Tennessee, western Kentucky, and southern Illinois. The NMSZ is the most seismically active region in the United States east of the Rocky Mountains.



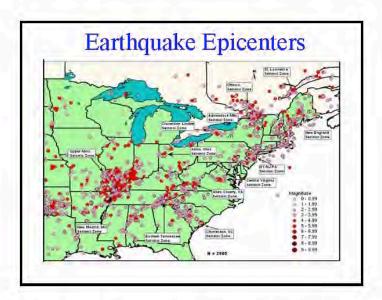


Figure 2. Earthquake Epicenters and Magnitudes in the Central and Eastern United States from 1568 - 1987

The KTC-96-4 research report indicated that a Central Kentucky earthquake event occurred on February 28, 1854 and assigned a Modified Mercalli intensity of V. The most severe effects of that earthquake was reported in Lebanon, where dishes and windows rattled. The earthquake was felt at numerous other locations in Kentucky including Bardstown and Harrodsburg.

4. Geotechnical Considerations

4.1. General

Based on discussions with the Project Team, it is not anticipated that there will be many new cuts or fills required along the existing roadways for the proposed I-69 alignments. However, it does appear that several interchanges will need to be reworked / realigned to meet the current interstate standards. The revisions to the interchanges will include lengthening ramps and changing horizontal / vertical alignments. As the interchanges are reworked, the Project Team should consider the geotechnical considerations that are included in Section 4 as they pertain to karst activity, erosion, cut slopes, embankments, widened structures and seismicity.

4.2. Karst Activity in the Area

Karst activity exists over portions of the project area near the Western Kentucky Parkway. Based on existing occurrences of known karst in the area, bedrock in the Mississippian Plateaus Region is considered to have a moderate to high potential for karst development. The potential for moderate to high karst activity is greater near the Western Kentucky Parkway and I-24.

An inventory of karst features is recommended during the next phase of study in areas where there is potential for karst activity. The inventory may be utilized to refine alignments and account for environmental related concerns such as water runoff into such features.

4.3. Erosion

The Mississippi Embayment Region consists of broad, flat flood plains along the Mississippi River to gently rolling uplands of low relief. The gently rolling terrain formed as streams gradually eroded the loess and continental deposits. The areas of erosion have exposed the less common Porters Creek Clay formation. Erosion concerns will affect the cut slope configuration and is further discussed in Section 4.4.

4.4. Cut Slope Considerations

The majority of the roadway cuts within the Mississippian Plateaus Region are likely to be shallow cuts in soils and bedrock. As previously discussed, rock types will consist of limestones, shales, and possibly sandstones. Cut slope configurations in rock are generally controlled by bedrock lithology, bedrock quality, results of Slake Durability Index (SDI) tests in shales and siltstones, and by the presence of any fractures and/or joints. In general, if joint/fracture angles are high (as measured from horizontal), steeper cut slopes can be constructed and an acceptable level of stability can be maintained. If discontinuities exhibit low angles and steep cut slopes are utilized, large block failures may occur along the open cut face.

Slope configurations for rock cuts in durable or Type I non-durable rock generally range from 1H:4V to 1H:2V pre-split slopes on approximate 30-foot intervals of vertical height. These types of cuts could be anticipated within the Mississippian Plateaus Region. Shallow cuts in bedrock may be best handled on 2H:1V slopes, covered with a soil layer and vegetated.

Slope configurations for soil cuts in alluvial deposits are generally constructed on a 2H:1V or flatter. Due to erosion concerns within the Mississippi Embayment Region, soil cuts in alluvium may be best handled on a 3H:1V or flatter and covered with vegetation.

4.5. Embankment Considerations

Embankments constructed of durable rock materials generally exhibit adequate stability at 2H:1V slope configurations. However, flatter embankment slopes may be required for tall embankments or in areas where embankments are founded on alluvial materials. Alluvial soils can be expected throughout the majority of the study area from the Tennessee state line to I-24. Since most of the improvements will be focused at interchanges, it is anticipated the embankments will be constructed from borrow and offsite sources.

Low shear strengths and high settlement potentials are generally associated with alluvial deposits. Consolidation settlements and short-term embankment stability problems are common for roadway embankments in alluvial floodplains, and controlled embankment construction rates and/or flatter embankment side slopes should be anticipated for these areas.

4.6. Structures

Based on discussions with the Project Team, it is anticipated that approximately 10 mainline structures will need to be widened to meet horizontal clearance requirements for an interstate. At this time, it is unknown as to whether the widening would require new and/or widened substructure elements. Based on Stantec's knowledge of the area, it can be anticipated that the majority of the structures within the project corridor are likely supported

on deep foundation friction elements. It can be problematic at times to install new deep foundation elements for widened structures due to access and existing battered pile elements. In addition, deep foundation elements may be difficult to install because of the presence of dense chert or gravel layers. Therefore, piles may need to be outfitted with pile points or pre-drilling may be warranted. Therefore, the Project Team should be prepared to study the structures on a case-by-case basis and anticipate special foundation designs in the next phase of the design process.

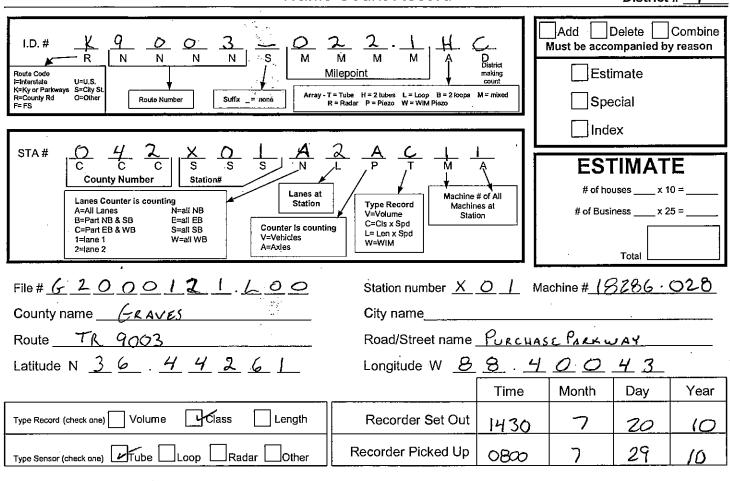
4.7. Seismic Concerns

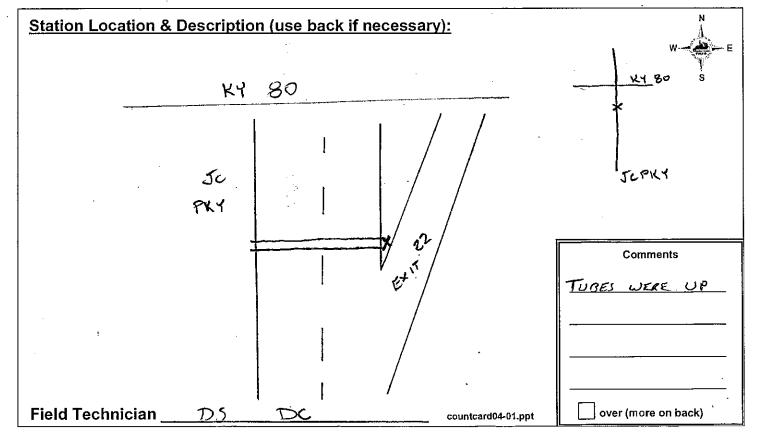
The seismic hazard at a bridge site shall be characterized by the acceleration response spectrum for the site and the site factors for the relevant site class. A comprehensive geotechnical investigation will be required to determine the site class. The 2010 AASHTO LRFD Bridge Design Specifications provides guidelines for selecting a seismic performance category and a soil profile type for bridge sites. This information establishes the elastic seismic response coefficient and spectrum for use in further structural design and analyses. Refer to Section 3.10.2 for specifications.

Appendix E. Traffic Counts and Directional Design Hourly Volumes (DDHV)

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District #





KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF TRANSPORTATION PLANNING

PORTABLE TRAFFIC RECORDER REPORT

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NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
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4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 26 26 26 TOTAL HOURS

н 24

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COUNTY: GRAVES

DISTRICT: 1

ROUTE:
MILEPOST: 22.100

ROAD DESIGNATION: U

LATEST ADT COUNT: 4358 IN 2010

LOCATION INFORMATION: PURCHASE PKWY NB JUST SOUTH OF KY 80

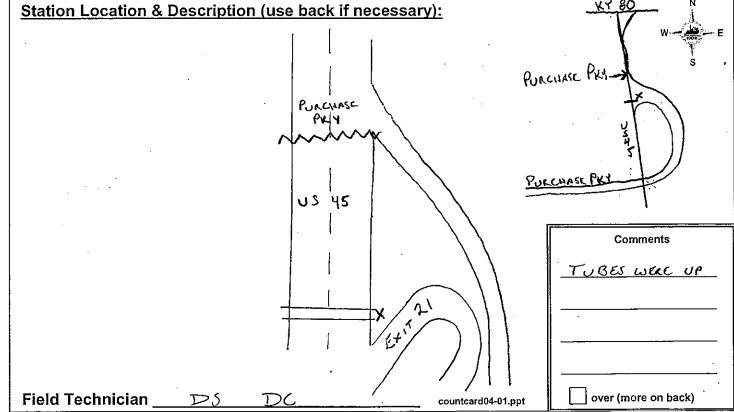
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT
2010
STATION: STATION: X01 BOTH N-S ROUTE: KY9003 TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District #

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NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 *** TOTAL HOURS

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COUNTY: GRAVES

DISTRICT: 1

ROUTE: US 45

MILEPOST: 15.400

ROAD DESIGNATION: U

ATEST ADT COUNT: 2472 IN 2010

LOCATION INFORMATION: US 45 NORTHBOUND AT PURCHASE PKWY INTERCHANGE

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT

7: GRAVES

2010

STATION:) STATION: XOZ BOTH N-S
ROUTE: US 45
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 16

Kentucky Transportation Cabinet - Division of Planning

over (more on back)

countcard04-01.ppt

Traffic Count Record			District	#
I.D. #	District making count	Must be acco	mpanied by	Combine y reason
STA# 10 4 2 X 0 3 A 2 A 1 1 M		Inde	IMAT	F
County Number Station# Lanes Counter is counting A=All Lanes N=all NB B=Part NB & SB	es at	# of ho	uses x 1 ness x 2 Total	0 =
File # <u>62000141.200</u> Station number <u>**</u>	<u>0 3</u> Ma	ichine# <u>/</u>	3236	<u>: 25</u>
County name City name			· .	· · · · · · · · · · · · · · · · · · ·
Route Road/Street name		·- ^	1 5	
Latitude N 3 6 . 4 4 3 3 6 Longitude W 3		1	<u> </u>	Voor
	Time	Month	Day	Year
Type Record (check one) Volume Class Length Recorder Set Out	1450	7	ొర	<u>ا</u>
Type Sensor (check one) Tube Loop Radar Other Recorder Picked Up	0900	7	29	10
Station Location & Description (use back if necessary):			₹ Ş Wa	N E S
De Contraction de la contracti	F		Comments	
	775			

Field Technician

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS: 48	DATE: 12-11-11-11-11-11-11-11-11-11-11-11-11-1		ROUTE: KY9003
: 97 : 100 : 168 : 295 414		4854	10000000000000000000000000000000000000		
BETWEEN BETWEEN	AVERAGE	4999	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	WEEK OF	
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MONDAY FRIDAY	:: 4398	3302	SUN 508 1134 1137 1137 1137 1137 1137 1137 1137	JULY	COUNTY
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POINT FED BY SOURCE		4772	1177 127 127 127 127 127 127 127 127 127		
: 22.1 : CEN OFF : CLASS : 2 TUBES : ALL LANES		31746	TOTALS 370 296 271 322 365 794 1091 1420 1448 1524 1637 1816 1988 2024 2439 24439 2461 1238 905		STATION XO3

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 ***

7 %%% A % M D M O M	PERCENT	TOTAL VE	07-07AM 08-09AM 08-09AM 09-10AM 11-12AM 12-01PAM 01-02PAM 02-03PAM 05-06PAM 06-07PAM 09-10PAM 11-12PAM 11-12PAM 11-02AAM 02-03AAM 03-04AAM	ıom	
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. 76 251/ 21/ 90/	0.4	20	NO0000-N000N	5 OR LESS AXLE	MULT
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36 36 36	0.0	0 *	000000000000000000000000000000000000000	OTHERS	
		4580	122 1788 1788 1788 1788 1788 1788 1788 1	VEHICLES	

COUNTY: GRAVES DISTRICT: 1 MILEPOST: 22.100 ROAD DESIGNATION: U LATEST ADT COUNT: 4398 IN 2010 LOCATION INFORMATION: PURCHASE PKWY SB JUST SOUTH OF KY 80

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
STATION: STATION: XO3 BOTH N-S ROUTE: KY90C3 TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record District # Add Delete Combine I.D.# Must be accompanied by reason Milepoint Estimate Roule Code t=interstate U=U.S. K=Ky or Parky Array - T = Tube ' H = 2 tubes 'L = Loop 'B = 2 loops R = Radar P = Piezo W = WIM Piezo R=County Rd F= FS Special Index STA# **ESTIMATE County Number** Station# # of houses Lanes at Machine # of Ali Lanes Counter is counting
A=All Lanes N=all NB Station Type Record V=Volume Machines at # of Business x 25 = Station B=Part NB & SB E=all EB C=Cls x Spd Counter is counting S=all SB C=Part EB & WB L= Len x Spd W=WIM V=Vehicles W=all WB 1=lane 1 A=Axles 2=lane 2 Machine #__1829 File # 62000 151.L00 Warz 5 County name ____ City name Road/Street name Longitude W 💆 Time Month Day Year :505 Volume Class Length Recorder Set Out Type Record (check one) / 🗢 Recorder Picked Up **0966** Type Sensor (check one) Tube LLOop JRadar ˈ[10Station Location & Description (use back if necessary):

Field Technician

Comments

over (more on back)

countcard04-01.ppt

TUBES WELL UP

MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR: PM HIGH HOUR: 2		TOTALS: 2728	12- 1 1- 2 1- 2 3- 4 4- 5 5- 6 6- 7 7- 8 8- 9 10- 10 11- 12 11- 12 11- 12 11- 12 11- 12 11- 13 12- 1 130 130 130 130 130 130 130 13	<u>'</u>		ROUTE: US0045	
97 100 24 148 BETWEEN 10-11 AM 0 244 BETWEEN 4- 5 PM 0	AVERAGE DAILY TRAFFIC			22 23 THU FRI 9	WEEK OF JULY	GRAVES	**************************************
ON WEDNESDAY	AFFIC: 2646			24 25 SAT SUN	21 TO JULY	ES COUNTY	יור אחריכאכחא אחדיכאיי
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNTED				26 27 MON TUE	27 2010		ייני קייני קייני
: 15.3 : CEN OFF : CLASS : 2 TUBES D: ALL LANES		2728	3588810882221773314633336833284 35888108833284 358883 35888 358883 358886883 358886 35886 358886 358886 35888 35888 35888 35888 35888 358886 35888 36888 36888 36888 36888 36888 36888 36888 36888 36888 36888 36888	TOTALS		STATION X04	

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00
OTHER 2 AXLE 4 TIRE VEHICLES 00
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 % % %

TP%%≯% Om ×××	PERCENT OF	TOTAL VEHICLES	07-08AM 08-09AM 09-10AM 11-13AM 11-13AM 11-13AM 11-03PM 01-02PM 03-04PM 05-06PM 08-07PM 08-07PM 09-10PM 11-12PM 11-12PM 11-12PM 01-03AM 03-04AM 03-04AM	TIME	
% HEAVY TRUCKS = AXLES / TRUCK = % TRAILER TRUCKS = % TRAILERS apk HR= PEAK HOUR BETWEEN 04 TOTAL HOURS = 2	OF TOTAL	HICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	DATE	
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98/ 352/ 53/ 4/	56.2	334	1-8575323 80507 800 800 700 700 700 700 700 700 700 7	PASSENGER CARS	
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594 54 ==	0 0	0	000000000000000000000000000000000000000	6 AXLE	I-TRAILER
" 7.6 " 9.3	0.2	_	000000000000000000000000000000000000000	7 OR MORE AXLE	I LER
% % %	0.0	Q	000000000000000000000000000000000000000	OTHERS	
		594	001-0000000000000000000000000000000000	TOTAL VEHICLES	

COUNTY: GRAVES DISTRICT: 1 ROUTE: US 45 MILEPOST: 15.300 ROAD DESIGNATION: U LATEST ADT COUNT: 2646 IN 2010 LOCATION INFORMATION: US 45 SOUTHBOUND AT PURCHASE PKWY INTERCHANGE

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT

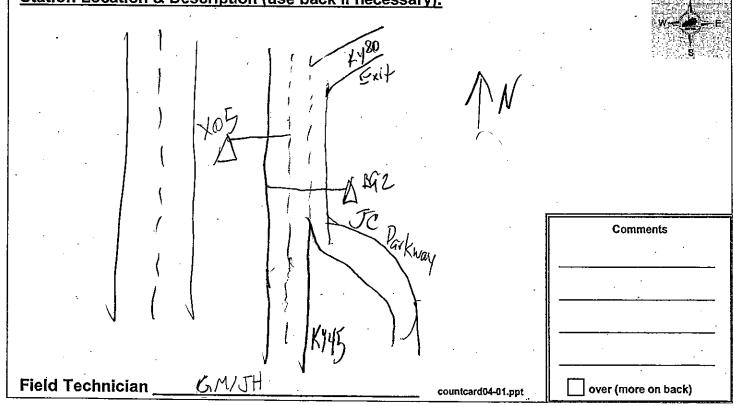
7: GRAVES

2010

STATION:) STATION: X04 BOTH N-S
ROUTE: US 45
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 16

Kentucky Transportation Cabinet - Division of Planning

Traffic	Count Record		u ii iii ig	Distric	t#				
Firstersace U=U.S. K=Ky or Pankways S=City St R=County Rd O=Cither Rouse Mumber Suffix 7 and Array-1	M M M Afilepoint T=Tube H=2 tubes L=Loop B=2 loops R=Radar P=Piezo W=WIM Plezo	District making count	Must be acc	imate ecial	Combine by reason				
STA# C C C S S S N L P T M A County Number Station# Lanes Counter is counting A=All Lanes N=all NB B=Part NB & SB E=all EB C=Part EB & WB S=all SB 1=fane 1 W=all WB 2=lane 2 Index ESTIN # of houses Counter is counting V=Volume C=Cls x Spd L=Len x Spd W=WIM Tota Tota									
File # G 2 0 0 0 / 8 / . L 0 0 County name	Station number X City name Road/Street name Longitude W			<u>5 a</u>	······································				
Type Record (check one) Volume Class Length	Recorder Set Out	15:a8	0.7	Day බර	Year 2010				
Type Sensor (check one)	Recorder Picked Up	9.725	01	29	2010				
Station Location & Description (use back	if necessary):	\ <i>\</i>			And the second s				

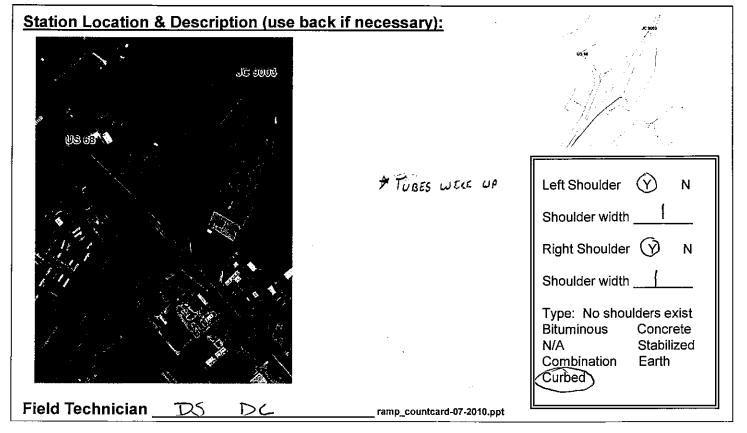


MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: KY9003
: 93 : 168 : 138 128		968	88 118 8 118 8 8 8 8 8 8 8 8 8 8 8 8 8		
BETWEEN BETWEEN	AVERAGE	1035	252 172 172 173 173 174 175 175 175 175 175 175 175 175 175 175	WEEK OF	
7- 8 AM 4- 5 PM	DAILY	996	FRI 34 8 4 4 8 8 77 71 71 64 63 63 64 65 65 64 65 65 65 65 65 65 65 65 65 65 65 65 65	JULY	GRA
ON THE	TRAFFIC:	577	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	21 TO	GRAVES COUNTY
MONDAY Thursday	671	381	79223346888845888888888888888888888888888888	JULY	ΥTY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT		1006	M M M M M M M M M M M M M M M M M M M	27 2010	
OINT :: OURCE :: COUNTED:		922	1270 1270 1270 1270 1270 1270 1270 1270		s
21.4 CEN OFF VOLUME TUBE ALL LANES		5885	TOTALS 18 18 18 31 12 31 158 380 614 383 343 343 343 343 355 376 376 550 385 127 132 101 86		STATION X05

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #

1.D.# K 9 0 0 3 R 0	4 7 O H	<u>_</u>	Ramp#	311	
Route Code -Interstate	Ölstrict making count M = mixed ter	EXIT 47 US 68			
STA# C C C S S S N County Number Lanes Counter is counting A=All Lanes B=Part NB & SB E=all EB C=Part EB & WB S=all SB 1=lane 1 W=all WB 2=lane 2 A N Lanes at Station Counter is counting V=Vehicles A=Axles	# of All		641 FEANIL		
File# 6200011.L00	Station number 3		chine#	1161 - 0	2029
# of Lanes \ \ Lane Width \ \frac{1.5}{	Traffic light	Stop sign	Merg	ge	
County Marshall	Route <u>079-JC-</u>	9003 -311	11 079350		
Latitude N <u>3 6 . 5 5 5 . 6</u>	Longitude W $\underline{\mathcal{S}}$	<u>8.2</u>	<u>08</u>	7 9	
		Time	Month	Day	Year
Type Record (check one) Volume Class Length	0805	7	20	10	
Type Sensor (check one)	1300	7	28	10	



MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	12-11-12-13-13-13-13-13-13-13-13-13-13-13-13-13-	DATE:		ROUTE: KY9003R
FACTOR: 97 TOR: 100 JURS: 120 HOUR: 203 HOUR: 260		2532	300 4 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	21		003R
BETWEEN	AVERAGE	2704	300 50 50 50 50 50 50 50 50 50 50 50 50 5	22	WEEK OF	
11-12 AM 5- 6 PM	SE DAILY TRAFFIC:	2808	34	23	F JULY	MARS
22	TRAFF	2254	SAT 1120 120 120 120 120 120 120 120 120 12	24	21	HALL
SATURDAY FRIDAY	IC: 2328	1703	78 78 78 78 78 78 78 78 78 78 78 78 78 7	25	YJUL 0T	MARSHALL COUNTY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT			S S S S S S S S S S S S S S S S S S S	26	27 2010	
OINT D BY OURCE			TO	27		
: 47.0 : CEN OFF : CLASS : 2 TUBES D: ALL LANES		12001	TOTALS 96 49 41 575 575 659 776 820 820 820 8772 8772 8778 978 978 978 978 978 978			STATION 311

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 % 3K 3K

*

7 % A X L A X L T T T T T T T T T T T T T T T T T T	PERCENT	TOTAL VE	05-07AM 08-09AM 09-10AM 11-11AM 11-12AM 11-01PM 01-02PM 01-02PM 02-03PM 05-06PM 06-07PM 06-07PM 08-09PM 09-10PM 11-12PM 11-12PM 11-12PM 01-03AM 01-05AM 01-05AM		
RAIL RAIL	OF TOTAL	VEHICLES	7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10	DATE	
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197 585 71 705P	66.8	1773	430 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	PASSENGER CARS	
7 2655 = 7.4 7 197 = 2.9 7 2655 = 2.7 7 239 = 2.1 M 239 VEHI	24.9	661	200700,00700000000000000000000000000000	OTHER 2 AXLE 4 TIRE VEHICLES	
770 CLES	0.4	≟	00000000000000000000000000000000000000	BUSSES	SINGLE
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ION FACTOR I TRUCKS I @ PK HR PEAK HOUR	0.9	24			TRUCKS
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	0.0	0			TRUCKS
	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	RAILER
26 36 36	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		2655	22 22 23 24 24 24 24 24 24 24 24 24 24 24 24 24	TOTAL VEHICLES	

TO US 68

COUNTY: MARSHALL 2010
DISTRICT: 1
MILEPOST: 47.000
ROAD DESIGNATION: R
LATEST ADT COUNT: 2328 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

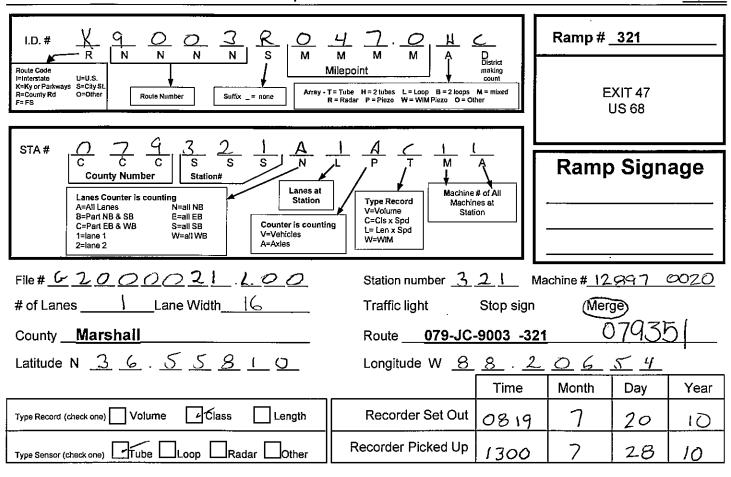
ZO10

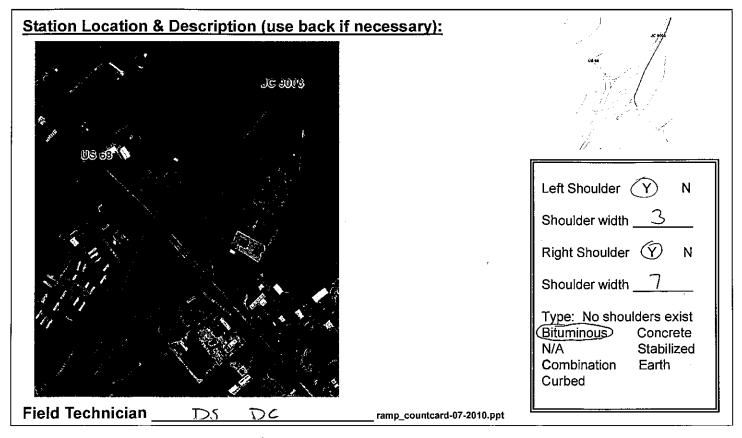
ZO10

ZO10 STATION: 311 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District # /





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: 12-1 12-1 13-4 4-5 4-5 4-5 4-5 4-5 4-7 7-8 8-7 7-8 7-8 8-7 7-8 7-8 7-8 7-8 7		ROUTE: KY9003R
TOR: 97 : 100 : 168 R: 255 R: 234		2802	M 2021-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1		03R
BETWEEN	AVERAGE	2853	THU 122 101 6 8 10 122 101 6 8 10 10 10 10 10 10 10 10 10 10 10 10 10	WEEK C	
7- 8 12- 1	GE DAILY	2992	FRI 23	OF JULY	MA
AM ON WE	/ TRAFFIC	2695	SAT 22 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	21 TO	MARSHALL C
WEDNESDAY	C: 2657	2397	SUR SUR 21 115 115 115 115 115 115 115 115 115	O JULY	COUNTY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT	7	2732	MO	27 2010	
OINT D BY OURCE		2715	1007 TENTON TO THE TENTON THE TENTON TO THE		"
47.0 CEN OFF CLASS 2 TUBES ALL LANES		19186	TOTALS 87 40 40 64 572 1186 1186 1187 1189 1219 1219 1219 1219 1219 1219 1219		STATION 321

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00
OTHER 2 AXLE 4 TIRE VEHICLES 00
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 36 34 34 TOTAL HOURS

II 24

% ≪% & ∪ × ×	PERCENT OF	TOTAL VEHICLE	06-07AM 08-09AM 08-09AM 09-10AM 11-12AM 11-12AM 12-03PM 03-05PM 03-05PM 06-07PM 06-07PM 07-08PM 08-09PM 09-10PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-13AM 03-05AM		
% HEAVY TRU AXLES / TRU % TRAILER T % TRAILERS PEAK HOUR B	OF TOTAL	HICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	DATE	
TRUCKS = 131/ TRUCK = 458/ R TRUCKS = 80/ RR S	0.8	21	N-000000-0-0N	MOTOR- CYCLES	
131/ 27 458/ 1 80/ 27 5/ 1	73.3	2010	22 5 5 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	PASSENGER CARS	
2743 = 4.8 131 = 3.4 2743 = 2.9 196 = 2.6 196 VEHI	21.2	581	25 25 25 25 25 25 25 25 25 25		
.6 % .6 % .6 %	0.3	8	000000000000000000000000000000000000000	BUSSES	SIN
÷	1.1	31	M00000000777000M0M0M0000	2 AXLE 6 TIRE	SINGLE UNIT
%%%>	0.4	12	000000000000000	3 AXLE	TRUCKS
AXLE CORREC % SINGLE UN % SINGLE UN % TRUCKS AT	0.0	0		4 OR MORE AXLE	KS
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2743 196 196	0.0	0	000000000000000000000000000000000000000	6 AXLE	I-TRAILER
= 1.9 = 2.0	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
34 34 34	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		2743	779 1796 1796 1796 1771 1771 1771 1771 1	TOTAL VEHICLES	

COUNTY: MARSHALL

DISTRICT: 1

ROUTE: KY
MILEPOST: 47.000

ROAD DESIGNATION: R

LATEST ADT COUNT: 2657 IN 2010

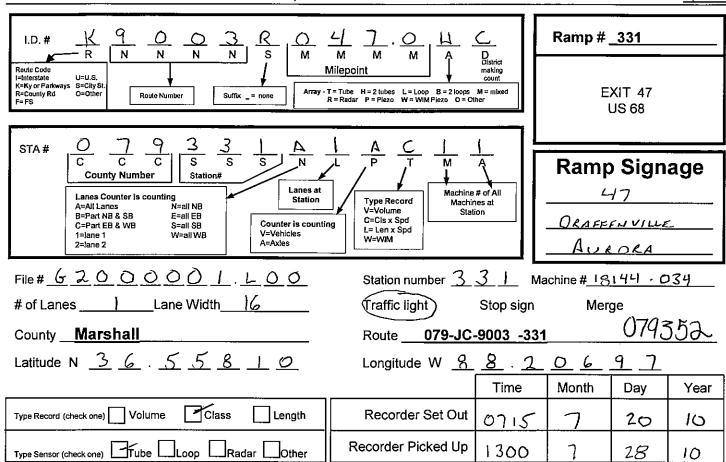
LOCATION INFORMATION: RAMP FROM US 68 ONTO PURCHASE PARKWAY NB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

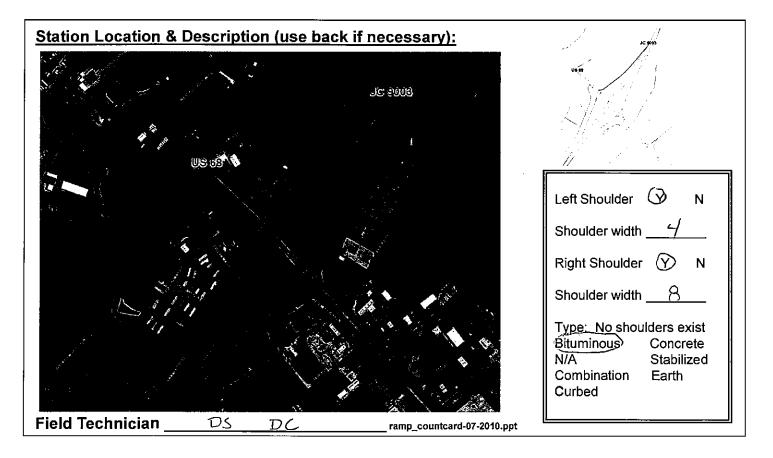
VEHICLE CLASSIFICATION COUNT
2010

STATION: STATION: 321 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AN HIGH HOUR PM HIGH HOUR		TOTALS:	DATE 122 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2		ROUTE: KY9003R
TOR: 97 : 100 : 168 : 168 JR: 176 JR: 307		2532	## 27 27 27 27 27 27 27 27 27 27 27 27 27)03R
BETWEEN	AVERAGE	2846	11122223333333333333333333333333333333	WEEK OF	
11-12 AM 3- 4 PM	GE DAILY	3064	FREE TARKS BEET TARKS	F JULY	MAR
22	TRAFFIC	2586	S 2 4 4 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	21 TO	MARSHALL CO
SATURDAY Friday	: 2499	1853	007444500000000000000000000000000000000) JULY	COUNTY
MILE F COUNTE DATA S ARRAY LANES	Ĭ	2557	MON 125 108 108 108 108 108 108 108 108 108 108	27 2010	
POINT FED BY SOURCE C COUNTED		2602	TUE 27 11 12 25 14 1 8 6 6 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
: 47.0 : CEN OFF : CLASS : 2 TUBES : ALL LAN					STATION
S F		18040	TOTALS 187 545 45 45 45 45 45 47 47 105 105 105 105 105 105 105 105 105 105		331

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 %
OTHER 2 AXLE 4 TIRE VEHICLES 00 %
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

TOTAL HOURS

Ii 24

% % % G I	PERCENT OF	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 11-02PA 03-05PA 03-05PA 03-05PA 06-07PA 07-08PA 08-09PA 11-12PA 11-12PA 11-12PA 11-27AM 07-03AM 07-03AM 07-03AM 07-03AM	TIME PERIOD	
% HEAVY TRU AXLES / TRU X TRAILER T X TRAILERS PEAK HOUR B	OF TOTAL	HICLES	77777777777777777777777777777777777777	DATE	
TRUCKS = 141/ TRUCK = 495/ R TRUCKS = 90/ RS @PK HR = 7/ RS @PK HR = 7/	0.7	17	0000000	MOTOR- CYCLES	
141/ 25 495/ 1 90/ 25 7/ 7	73.1	1886	24 4 7 4 2 4 2 4 2 4 2 4 2 4 2 4 2 4 2 4	PASSENGER CARS	
/ 2579 = 5.5 / 141 = 3.5 / 2579 = 3.5 / 248 = 2.8 M 248 VEHI	20.7	535	1 2 2 2 2 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3	OTHER 2 AXLE 4 TIRE VEHICLES	,
E 8 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0.2	4	0-00000000000000-00	BUSSES	SINGLE
,	1.3	34	NN4NMNM4MN	2 AXLE 6 TIRE	GLE UNIT
**** >	0.5	13		3 4 AXLE M	T TRUCKS
AXLE CORR & SINGLE & SINGLE	0.0	0		OR ORE XLE	S
ECTI UNIT AT F	2.7	70	といしのののししとなるなりとクノノイヤヤケンシー	4 OR LESS AXLE	SINGLE
TRUCKS a PK HR EAK HOUR	0.8	20	00000 0		LE TRAIL
0 0 0 0	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	Ę
0.96 51/ 2579 = 2.0 5/ 248 = 2.0 12/ 248 = 4.8	0.0	0	000000000000000000000000000000000000000		MULT
	0.0	0	000000000000000000000000000000000000000	6 AXLE	I - TRAI TRUCKS
	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	I L ER
****	0.0	0 *		OTHERS	
		2579	28 11 8 5 11 8 7 12 7 13 1 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	TOTAL	

COUNTY: MARSHALL

DISTRICT: 1

ROUTE: KY9

MILEPOST: 47.000

ROAD DESIGNATION: R

LATEST ADT COUNT: 2499 IN 2010

LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB TO TO US 68

STATION: 331 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

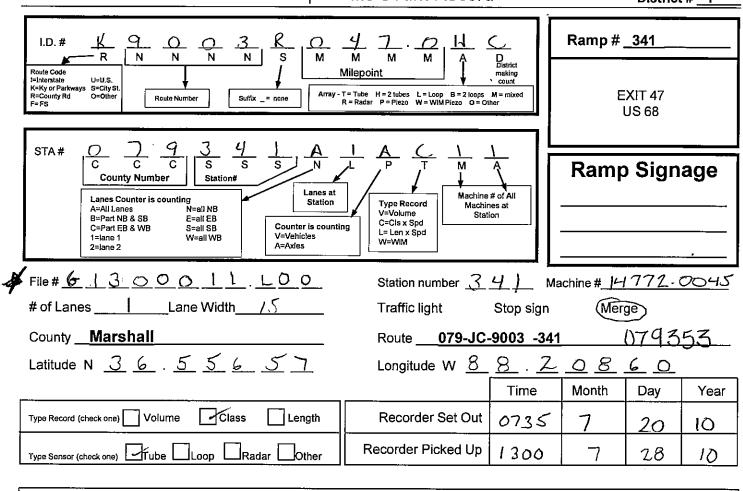
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

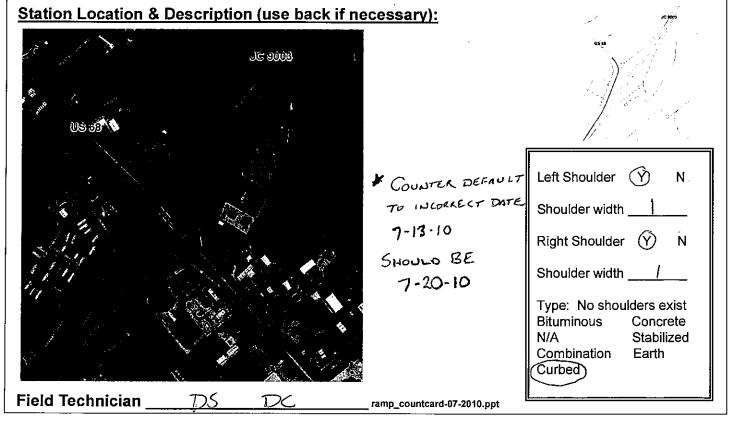
VEHICLE CLASSIFICATION COUNT

ZIATION:

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	11-12 PM	74272	3- 4 AM 4- 5 AM 5- 7 AM 7- 8 AM 8- 9 AM 9-10 AM	DATE: DAY: 12- 1 AM 1- 2 AM 2- 3 AM	ROUTE: KY9003R
TOR: 97 : 100 : 168 : 176 R: 210		2511	55611880 56811880	157 169 175 175	158 117 117 117 117	21 11 6	03R
0 BETWEEN	AVERAGE	2595	553366835 55366835	152 163 164 193	136 136 136 136 136 136 136	122 120 5	WEEK OF
7- 8 AM 3- 4 PM	3E DAILY	2621	189 1138 1118 1118 1118 1118 1189 1189 1	161 150 167 171 210	1359 1427 1427	FRI 27 8	Ę
22	TRAFFIC:	2216	166 1132 118 1108 110	160 129 141	113 113 118 118	24 31 12	MARSHALL C
THURSDAY FRIDAY	C: 2332	1952	146 145 137 125 82 20	109 136 119 145	1118332 1107 117	25 SUN 19	COUNTY TO JULY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT	10	2485	182 112 123 77 77 78	149 157 172 151	134 134 155	MON 14 6	27 2010
POINT : ED BY : SOURCE : COUNTED:		2459	196 112 112 117 117 56	120 138 158 202	1128 128 149	10E 13	
47.0 CEN OFF CLASS 2 TUBES ALL LANES		.16839	1266 1196 909 786 818 589 476 281	1008 1084 105 <i>7</i> 1086 1239	72 98 301 943 924 924 945	TOTALS 141 60 65	STATION 341

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 % % %

TOTAL HOURS

п 24

	PE	TO:	0000011100076 000011100076	₹ PET	
% 4 % & 0 1	PERCENT OF	TOTAL VEHICLE	07-08AM 07-08AM 09-10AM 09-10AM 09-10AM 01-02P 01-02P 01-02P 01-02P 01-03P 01-03P 01-03P 01-03P 01-03P 01-03AM 03-04AM		
% HEAVY TRU AXLES / TRU % TRAILER T % TRAILERS T PEAK HOUR B	7	HICLES	77777777777777777777777777777777777777	DATE	
CKS =	0.7	18	-0000000N0-N	MOTOR- CYCLES	
130/ 398/ 55/ -05PM	68.9	1657	28888888888888888888888888888888888888	PASSENGER CARS	
2404 = 5.4 130 = 3.1 2404 = 2.1 182 = 2.1 182 VEH	24.9	599	↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑	4	
5.4 % 3.062 2.3 % 2.2 % VEHICLES	0.1	и	0-00000000000000-000-	BUSSES	
	2.6	62	424533445~53332~200000~~	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
****	0.4	9	000000000000	3 AXLE	TRUC
AXLE CORR % SINGLE % SINGLE % TRUCKS	0.0	_	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
ASSE	1.6	38	- ผลผลผลผล+พ4ผลผล-0000000	4 OR LESS AXLE	SINGLE
TION FACTOR T TRUCKS T @ PK HR PEAK HOUR	0.7	17	0000000-00NN		LE TRAILER
	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.97 75/ 2404 = 3.1 5/ 182 = 2.7 9/ 182 = 4.9	0.0	0		5 OR LESS AXLE	MULT
	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
36 36	0.0	o, .	000000000000000000000000000000000000000	OTHERS	
		2404	550 50 50 50 50 50 50 50 50 50 50 50 50	TOTAL	

COUNTY: MARSHALL
DISTRICT: 1
ROUTE:
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 2332 IN 2010
LOCATION INFORMATION: RAMP FROM US 68 ONTO PURCHASE PKWY WB

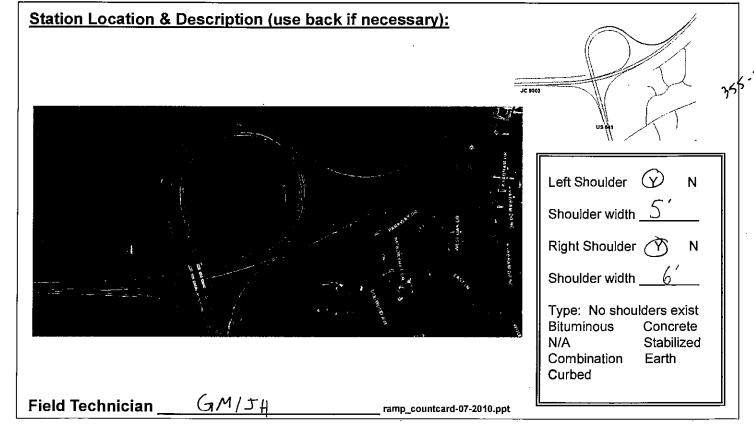
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION: STATION: 353 BOTH E-W ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #

I.D.# <u>K 9 0 0 3 r 0 0</u>	<u>O. I. H.</u>	<u>C</u>	Ramp #	131	
Route Code =Interstate	District making count M = mixed	EXIT 41 US 641			
STA# C C C S S S N L Lanes at	A # of All	Ramp Hardl	Sign	age	
Lanes Counter is counting A=All Lanes B=Part NB & SB	Type Record V=Volume C=Cls x Spd L= Len x Spd W=WIM Machines at Station Machines at Station Machines at Station Machines at Station Murray Murra				
# of Lanes Lane Width \(\frac{1}{4} \)	Station number Traffic light	3 } M Stop sign	achine # <u>11</u> Mer		011
County <u>Marshall</u>	Route079-JC-	9003 -13	1		
Latitude N <u>3 6</u> . <u>9 9 7 3 3</u>	Longitude W 🦠	§. <u>a</u>	8 7	16	
		Time	Month	Day	Year
Type Record (check one) Volume Class Length	Recorder Set Out	7:1 9	07	20	2010
Type Sensor (check one)	Recorder Picked Up	14:08	07	28	2010
		1			



MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR: PM HIGH HOUR:	TOTALS: 4445	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: KY9003R
97 100 168 263 BETWEEN 1: 474 BETWEEN 3	46	00000000000000000000000000000000000000	WEEK O	*P0R
IGE DAILY	4872	12200116888833336029011683324444444556655566556655666666666666666	OF JULY	*PORTABLE TR MAR
TRAFFI		0200288324243444444444444444444444444444	21 TO	TRAFFIC RI MARSHALL CO
C: 4153 RIDAY HURSDAY	324	SUN 50 50 50 50 50 50 50 50 50 50	A JULY	RECORDER F
MILE P COUNTE DATA S ARRAY LANES	4363	MON	27 2010	REPORT*
POINT TED BY SOURCE C	4460	1127 127 127 127 127 127 127 127 127 127		
CEN OFF CLASS TUBES	29974	TOTALS 346 184 187 147 147 147 129 1178 1178 1178 1178 1178 1179 1557 2066 2723 2723 2743 1478 1478 1412 1209 935		355 Station 437

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 % OTHER 2 AXLE 4 TIRE VEHICLES 00 % 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

TOTAL HOURS

= 24

. <u> </u>	PERCENT OF	TOTAL VEHICLES	07-05PM 07-08AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12PM 01-02PM 03-05PM 03-05PM 06-07PM 08-09PM 08-09PM 08-09PM 08-09PM 08-07PM		
% HEAVY TRI AXLES / TRI X TRAILER % TRAILERS PEAK HOUR I	OF TOTAL	SHICLES	77777777777777777777777777777777777777	DATE	
JCKS = JCK =	0.8	35	-0000-N-N-NNWNWNNNNNW	MOTOR- CYCLES	
502 1818 271 271 19	60.1	2578	200 200 200 200 200 200 200 200 200 200	PASSENGER CARS	
/ 4292 = 11. / 502 = 3. / 4292 = 6. / 390 = 4. M 390 VEH	27.4	1177	10774 2774 2774 2774 2774 2774 2774 2774	0TH 2 A 4 T 4 T	
1.93622 1.9322 1.0522 1.0522	0.6	25	0000000NWWNWN-N	BUSSES	SINGLE
% % % >	3.5	151	\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	2 AXLE 6 TIRE	GLE UNIT
	0.8	35	とろくろろろろろくくくしょしょしょしょ	3 AXLE	TRUCKS
AXLE CORRECT % SINGLE UNI % SINGLE UNI % TRUCKS AT	0.5	20	55555555555555555555555555555555555555	4 OR MORE AXLE	S
70-1-1-	1.7	74	ろろ4054505704423112110111	4 OR LESS AXLE	SINGLE
ON FACTOR TRUCKS a PK HR EAK HOUR	4.5	195	00000000000000000000000000000000000000		LE TRAILER TRUCKS
	0.0	0		6 OR MORE AXLE	LER
0.91 231/ 4292 = 5.4 21/ 390 = 5.4 40/ 390 = 10.3	0.0	N	000-00000000-0000000	5 OR LESS AXLE	MULI
	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
94 96 96	0.0	0 *	000000000000000000000000000000000000000	OTHERS	
		4292	1172 1257 1257 1257 1257 1257 1257 1257	TOTAL VEHICLES	

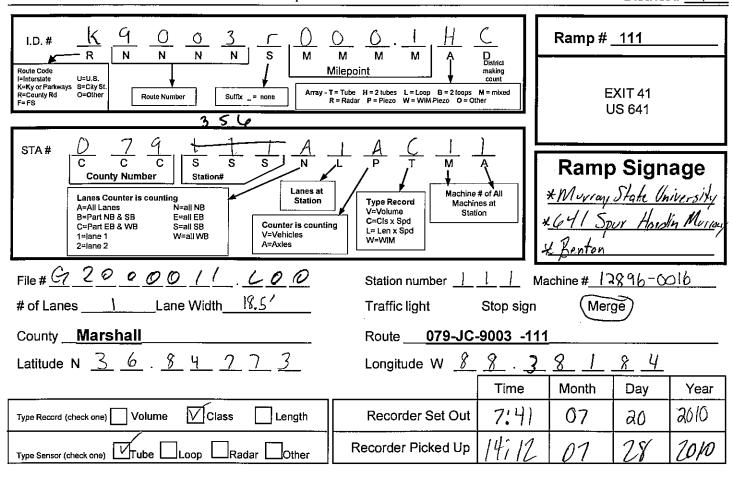
COUNTY: MARSHALL
DISTRICT: 1
ROUTE:
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 4153 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PKWY WB ONTO US 641

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
2010

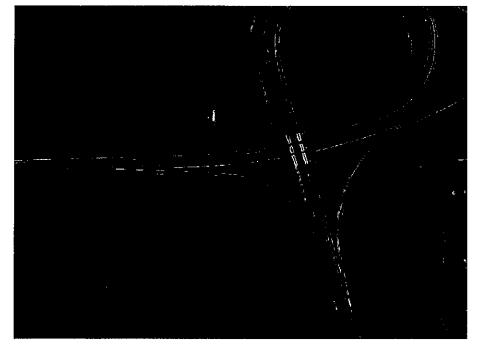
STATION: STATION: 355 BOTH E-W ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

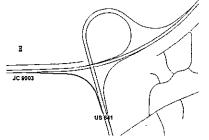
District #



<u>Station Location & Description (use back if necessary):</u>



Field Technician GM/JH ramp_countcard-07-2010.ppt



MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	120 DD A A A A A A A A A A A A A A A A A A		ROUTE: KY9003R	
		359	E 23 E 23 E 24 E 24 E 24 E 24 E 24 E 24 E 24 E 24)3R	
97 00 68 29 BETWEEN 50 BETWEEN	AVERAGE	375	77 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	WEEK OF		*PORTABLE
11-12 AM 5- 6 PM	DAILY	420	TR 23	. JULY	MAR	
22	TRAFFIC:	249	SAT 41122223771188111119883200271188477	21 10	MARSHALL CO	TRAFFIC RE
FRIDAY FRIDAY	: 308	186	22 S S S S S S S S S S S S S S S S S S	AJUL	COUNTY	RECORDER R
MILE POINT COUNTED BY DATA SOURC ARRAY LANES COUN		307	820 820 820 820 820 820 820 820 820 820	27 2010		REPORT*
POINT ED BY SOURCE COUNTED:		333	TUE 7 127 127 127 127 127 127 127 127 127 1			
CEN OFF CLASS Z TUBES ALL LANES		2229	TOTALS 117 127 133 141 141 141 141 141 141 141 141 141		STATION 444	456

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 ***

17%%A% OM 11	PERCENT OF	TOTAL VEHICLES	07-08AM 07-08AM 07-08AM 07-08AM 07-012AM 07-012AM 07-012AM 08-012AM	TIME PERIOD	
RAILES /	OF TOTAL	HICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	i m	
CKS = CKS = RUCKS = OPK HR=ETVEEN	0.0	0	000000000000000000000000000000000000000	ES -	
41/ 3 135/ 19/ 3 1/ 04-05PM	53.8	171	7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	PASSENGER CARS	
318 = 12.9 % 41 = 3.293 318 = 6.0 % 34 = 2.9 % 34 VEHICLE	33.3	106	₩ ₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	OTHER 2 AXLE 4 TIRE VEHICLES	
C %%3%	0.0	0		ES	SINI
	6.3	20	000000000000000000000000000000000000	2 AXLE 6 TIRE	SINGLE UNIT
≪ %%≫	0.6	~	0000-000000-0000000	m	TRUCKS
AXLE CORI % SINGLE % SINGLE % TRUCKS	0.0	0	000000000000000000000000000000000000000		S
ONIII UNIII AT F	1.9	6	000000000000000000000000000000000000000	4 OR LESS AXLE	SING
ON FACTOR TRUCKS OPK HR PEAK HOUR	4.1	13	~0000000		SINGLE TRAILE
ווווווו	0.0	0			LER
.92 22/ 3/	0.0	0	000000000000000000000000000000000000000	L	MUL 1
318 = 34 = 34 =	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
8.00	0.0	o.	000000000000000000000000000000000000000	7 OR MORE AXLE	ER
26 26 26	0.0	Q.	000000000000000000000000000000000000000	OTHERS	
		318	00100000000000000000000000000000000000	TOTAL VEHICLES	

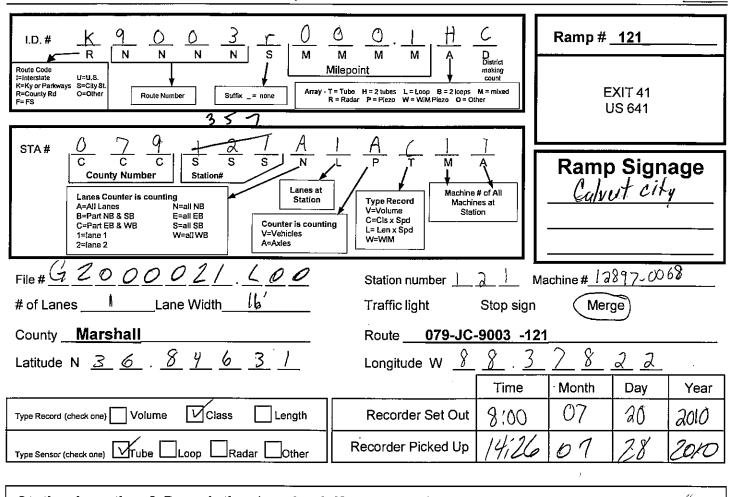
COUNTY: MARSHALL STATION:
DISTRICT: 1
ROUTE:
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 308 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PKWY EB ONTO US 641

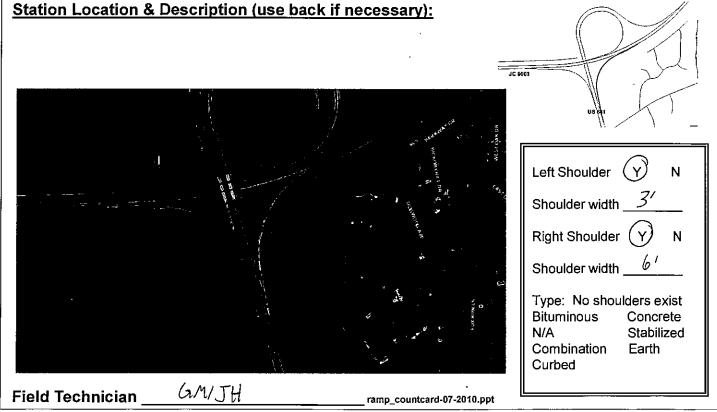
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION: STATION: 356 BOTH E-W ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAYE: DAYE: 12-1 1-2-5 2-4-5 3-4-5 3-4-5 4-5 4-5 4-7 4-7 4-7 10-11 11-12		ROUTE: KY9003R	
TOR: 97 : 100 : 168 R: 461 R: 386		4760	88 67 71 70 70 70 70 70 70 70 70 70 70 70 70 70)03R	
BETWEEN	AVERAGE	4999	THU 122 123 124 124 124 124 124 124 124 124 124 124	WEEK 0		*POF
7- 8 A	GE DAILY	5259	FRI 27 27 27 27 27 27 27 27 27 27 27 27 27	OF JULY	MA	*PORTABLE TI
AM ON	TRAFFIC	4088	SAT 117 333 34 25 25 25 25 25 25 25 25 25 25 25 25 25	21 Т	MARSHALL C	TRAFFIC R
MONDAY FRIDAY	C: 4450	3555	SUN SUN 32 32 32 32 32 32 32 32 32 32 32 32 32	TO JULY	COUNTY	RECORDER
MILE PO COUNTED DATA SO ARRAY LANES O	0	4723	MON 1133 126 1133 126 1133 126 126 126 126 126 126 126 126	27 2010		REPORT*
POINT FED BY SOURCE C		4736	1122 2422 2522 2523 2523 2523 2523 2523 25			
0.1 CEN OFF CLASS 2 TUBES ALL LANES		32120	TOTALS 193 99 147 1947 1947 1947 1947 2000 2000 2015 2000 2000 2000 2000 2000		STATION 124	ب ری

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00
OTHER 2 AXLE 4 TIRE VEHICLES 00
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

TOTAL HOURS

11 24

% HEAVY TRU AXLES / TRU % TRAILER T % TRAILERS: PEAK HOUR B	PERCENT	TOTAL VEHICLE	07-07 08-09 110-110 11-120 01-02P 01-02P 01-03P 01-03P 01-03P 01-03P 01-10P 01-110P 01-010P 01-020 01-020 01-020 01-030 0		
	OF TOTAL	HICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 10/20/10 10/20/10 10/20/10	DATE	
PPK HR	0.7	31	0-10000-10-1-1-WNNNNNNNN-N-WNNN	MOTOR- CYCLES	
504 1834 283 15 07-08A	62.7	2888	7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	PASSENGER CARS	
/ 4606 = 10.9 % / 504 = 3.639 / 4606 = 6.1 % / 363 = 4.1 % M 363 VEHICL	25.7	1183	51562345502350777788777887	OTHER 2 AXLE 4 TIRE VEHICLES	
CL%%9%	0.4	20	-N-0000000N-NNN	BUSSES	SIN
	3.3	153	พพ¬oo¬¬ททกรงกบทงจตจตจ ¬¬¬	2 AXLE 6 TIRE	SINGLE UNIT
××××	1.0	45	N47455777777777777777777777777777777777	3 AXLE	T TRUCKS
AXLE CORRECT % SINGLE UNI % SINGLE UNI % TRUCKS AT	0.1	ы	0000000000000000	!	Š
RRECTI E UNIT S AT P	1.6	74	ころみごみでとなるなるなんととしてしてしていると	4 OR LESS AXLE	SING
TION FACTO	4.5	207	8042012111008100814484040	5 AXLE	SINGLE TRAILER
א א	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	Ë
0.92 221/ 19/ 34/	0.0	2	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
4606 363 363	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	TRAILER UCKS
% 3% 3%	0.0	°	000000000000000000000000000000000000000	OTHERS	
		4606	165114655555555555555555555555555555555	TOTAL VEHICLES	

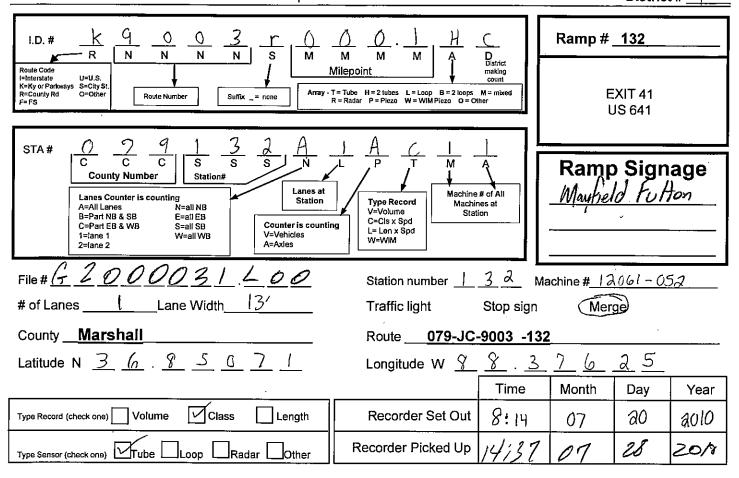
COUNTY: MARSHALL
DISTRICT: 1
ROUTE:
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 4450 IN 2010
LOCATION INFORMATION: RAMP FROM US 641 ONTO PURCHASE PKWY EB

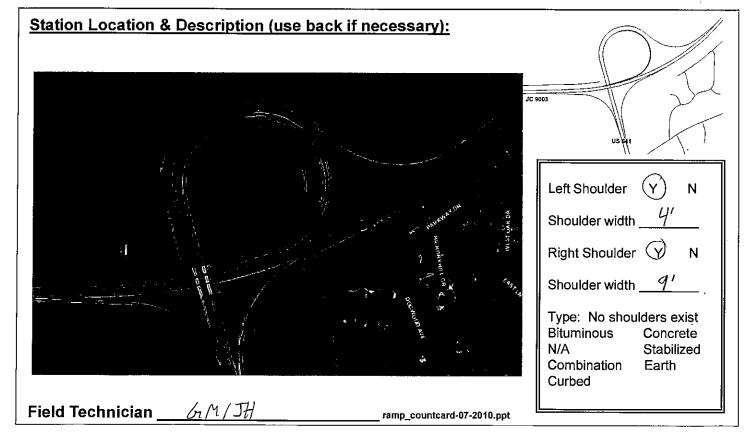
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**
2010

TATION: STATION: 357 BOTH E-W ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #

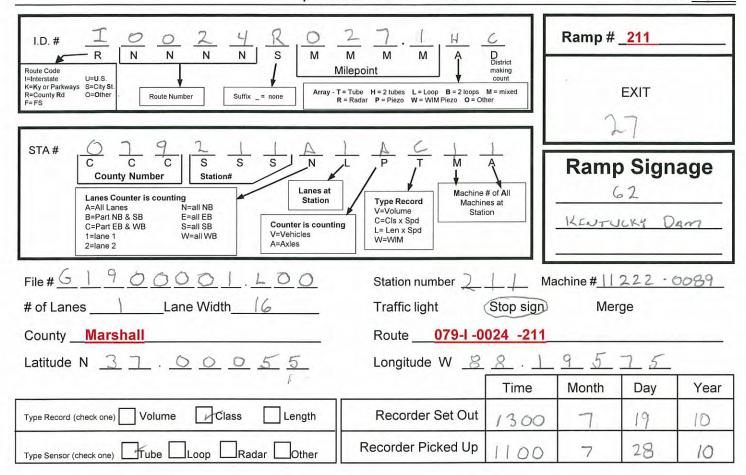


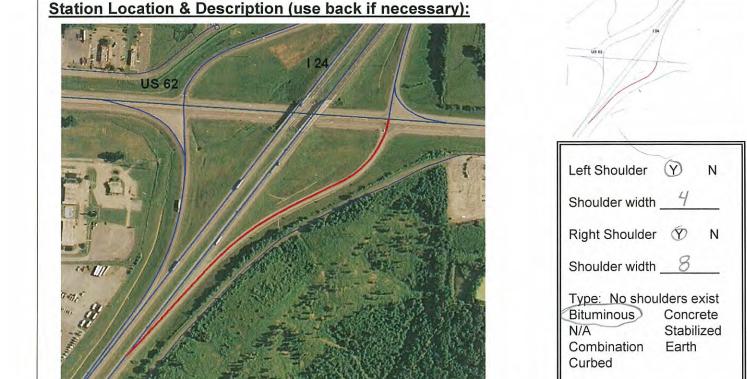


MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DAY 12-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-		ROUTE: KY9003R
TOR: 97 : 100 : 168 R: 40 R: 32		284)03 _R
BETWEEN	AVERAGE	282	1700 271 271 271 271 271 271 271 271 271 271	WEEK OF	
6- 7 AM	SE DAILY	304	22 T L C C C C C C C C C C C C C C C C C C	F JULY	MARS
ON THE	DAILY TRAFFIC:	187	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	21 10	MARSHALL CO
MONDAY THURSDAY	: 253	195	0749755555555555555555555555555555555555	AJUL	COUNTY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNTED		288	-642722337333332252573573333332535733333333	27 2010	
** ** ** ** **		293	27 L L L L L L L L L L L L L L L L L L L		s
0.1 CEN OFF CLASS 2 TUBES ALL LANES					STATION 1
E		1833	TOTALS 707ALS 1158 1158 1158 1158 1158 1158 1158 115		132

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





ramp_countcard-07-2010.ppt

Field Technician

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: 12.7 12.7 13.7 14.7 15.4 16.5 16.5 17.7 17.7 17.7 17.7 17.7 17.7 17.7 17	ROUTE: 100	
		2330	105 20 10	10024R	
93 100 168 193 BETWEEN 279 BETWEEN	AVERAC	2458	£63777777777777777777777777777777777777	WEEK OF	
11-12 5- 6 I	AVERAGE DAILY	2704	THU	Ē	
PM ON	Y TRAFFIC:	2908	FRI 1221223 11264 1221221 1263 1263 1263 1263 1263 1263 1	MARSHALL LY 20	
SATURDAY FRIDAY	IC: 2317	2529	59775 59775	COUNTY TO JULY	
MILE P COUNTE DATA S ARRAY LANES	7	2139	3537884 11381 1138 1138 1138 1138 1138 1138	26 2010	
POINT TED BY SOURCE		2378	MON 26 21 21 31 31 31 31 31 31 31 31 31 31 31 31 31		
CEN OFF				STATION 211	
ö		7446	OTALS 208 158 157 166 158 157 1158 1157 1159 1159 11181 1197 1197 1197 1197 1197 1197 11		

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

TOTAL HOURS

= 24

IP%%A%	PERCENT	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-11AM 11-11AM 11-12PM 01-02PM 02-03PM 03-04PM 06-07PM 07-08PM 08-09PM 07-10PM 11-12PM 11-12PM 11-12PM 11-12PM 07-03AM 07-03AM 07-03AM 07-03AM	TIME	
% HEAVY TRU AXLES / TRU % TRAILER TI % TRAILERS (PEAK HOUR B	PERCENT OF TOTAL	HICLES	77/19/10 77/19/10 77/19/10 77/19/10 77/19/10 77/19/10 77/19/10 10/10/10 77/19/10 10/10/10 10/10/10 10/10/10 10/10/10 10/10/10 10/10/10/10 10/10/10/10/10/10/10/10/10/10/10/10/10/1	DATE	
TRUCKS = TRUCK = R TRUCKS = RS @PK HR= RS @PK HR=	1.3	33	0000-00000000-0000000000000000000000	MOTOR- CYCLES	
RUCKS = 812/ RUCK = 3328/ TRUCKS = 551/ S apk HR = 28/ BETWEEN 05-06PM	39.2	966	1877671015337464883376866773377677101523377677	PASSENGER CARS	
2463 = 33.0 812 = 4.0 2463 = 22.4 180 = 15.6 180 VEHI	26.5	652	16553345544445832222 16553345544445832222		
33.0 % 4.099 22.4 % 15.6 % VEHICLES	1.2	29	NN0000-00-WN-NNN-NN-N-	BUSSES	SIN
	5.6	138	トイトタクののでのできょうとしてローロント	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
20 20 20 To	3.8	93	0-0-00000	3 AXLE	T TRUC
AXLE CORR % SINGLE % SINGLE % TRUCKS	0.0	_	00000000000000	4 OR MORE AXLE	KS
AT F	3.0	73	4WW4DVADVADWCUVCOCC4	4 OR LESS AXLE	SINGLE
ON FACTOR TRUCKS a PK HR PEAK HOUR	18.2	449	8669011111000000000000000000000000000000		TRUCKS
חחווו	1.0	25	000-NNNNN	6 OR MORE AXLE	ER
0.74 261/ 11/ 39/	0.1	2	000000000000000000000000000000000000000	5 OR LESS	MUL
2463 180 180	0.0	0	000000000000000000000000000000000000000	6 AXLE	TI-TRAILER TRUCKS
= 10.6 = 6.1 = 21.7	0.1	2	00000000-00000000-000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		2463	56 22 22 24 4 5 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	TOTAL	
					V

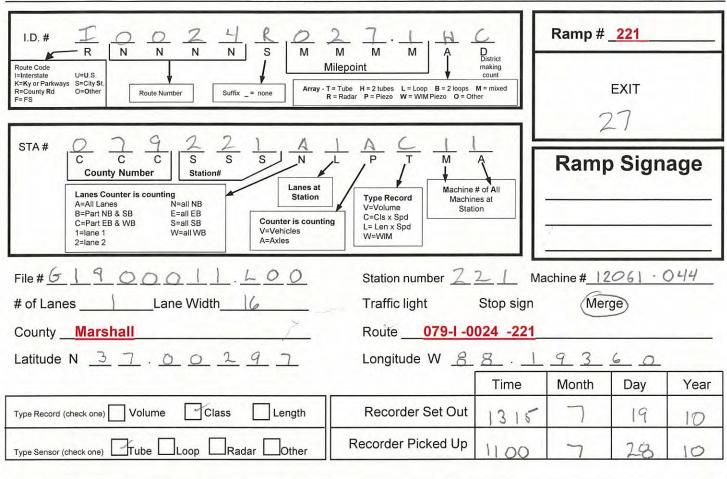
62

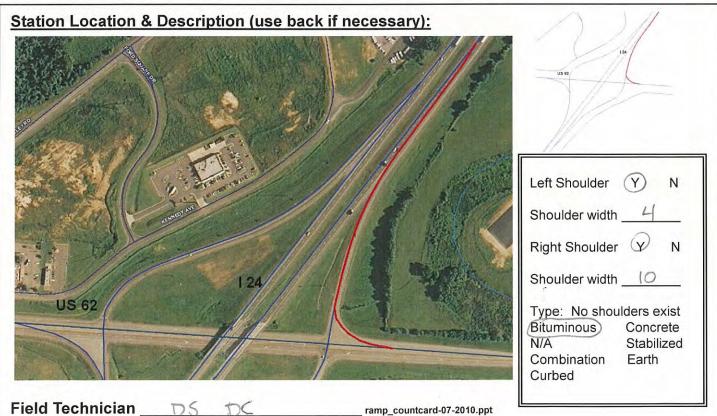
COUNTY: MARSHALL
DISTRICT: 1

MILEPOST: 27.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 2317 IN 2010
LOCATION INFORMATION: RAMP FROM I 24 EASTBOUND TO US STATION: 211 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
2010 STATION:

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: 12-1 12-2 1-2 2-3 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3		ROUTE: 100
CTOR: 93 100 168 168 167 JR: 236		2456	TUE 20 110 110 110 110 110 110 110 110 110		10024R
BETWEEN	AVERAGE	2440	2000 2000 2000 2000 2000 2000 2000 200	WEEK C	
10-11 3- 4	GE DAIL	2712	T 22 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	OF JULY	MA
AM ON	DAILY TRAFFIC	2823	FRI 331 120 23 23 23 23 23 23 23 23 23 23 23 23 23	20	MARSHALL COUNTY
THURSDAY	IC: 2273	2233	22 22 22 22 22 22 22 22 22 22 22 22 22	TO JULY	COUNTY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT	3	2194	SUN 51 117 117 117 117 117 117 117	26 2010	
OINT D BY OURCE		2258	MON MON MON MON MON MON MON MON MON MON	Ĩ	
: 27.1 : CEN OFF : CLASS : 2 TUBES		17116	TOTALS 251 150 142 145 223 453 453 547 740 875 983 1037 1037 1037 1037 1132 1132 1132 1132 11249 1249 1249 1249 1249 1249 1249 12		STATION 221

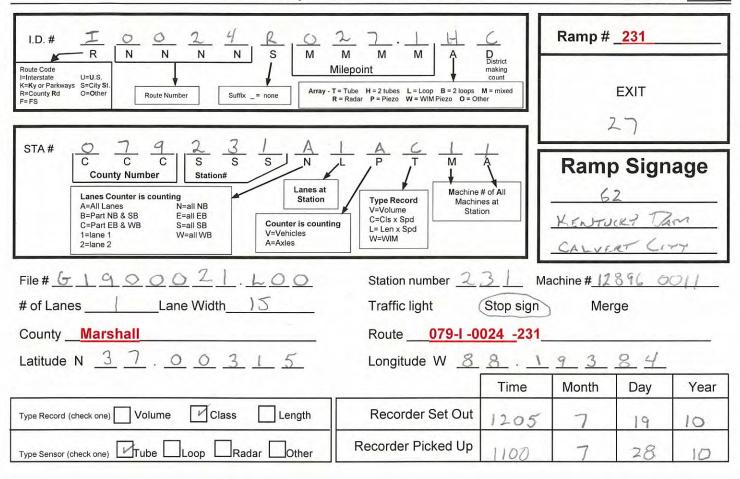
* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

TP%%A%	PERCENT	TOTAL VEHICLES	07-08AM 07-09AM 09-10AM 11-12AM 11-12PM 01-02PM 01-02PM 02-03PM 03-04PM 06-07PM 06-07PM 07-08PM 09-10PM 10-11PM 11-12PM 11-12PM 11-02AM 03-04AM 05-06AM	TIME	
% HEAVY TRUCKS = AXLES / TRUCK = % TRAILER TRUCKS = % TRAILERS apk HR= PEAK HOUR BETWEEN 0: TOTAL HOURS =	PERCENT OF TOTAL	HICLES	777777777777777777777777777777777777777	DATE	
TRUCKS = TRUCK = R TRUCKS = RS @PK HR= R BETWEEN = URS =	0.9	21	00-000-0	MOTOR- CYCLES	
623 2761 490 24 3-04P	45.6	1098	21-666327788377866555555555555555555555555555555	PASSENGER CARS	
/ 2410 = 25.9 % / 623 = 4.432 / 2410 = 20.3 % / 182 = 13.2 % M 182 VEHICLE	27.7	668	2522248877708845554444468864676 62544444686864444468864676	VEHI 2 A	
432 432 3 % 1CLES	0.7	17	-00-000-00NN	BUSSES 6	SING
	4.1	99	4-00-0-10000000000000000000000000000000	AXLE TIRE	SINGLE UNIT TRUCKS
%%% >	0.7	17	-00000d00NN	3 AXLE	T TRUC
AXLE CORR % SINGLE % SINGLE % TRUCKS	0.0	0			KS
UNIT P	2.0	47	-0000440000040	4 OR LESS AXLE	SING
ON FAC TRUCK a) PK EAK HC	15.9	383	11078867711668789822222277	5 AXLE	SINGLE TRAILE
~~ S	1.7	41	070077777007070000000000000000000000000	6 OR MORE	LER
0.76 133/ 2 10/ 34/	0.1	2	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
410 182 182	0.0	0	000000000000000000000000000000000000000	AXL 6	TRUCKS
185.5	0.7	17	0000000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	RS	
		2410	1205 1205 1205 1205 1205 1205 1205 1205	TOTAL	

COUNTY: MARSHALL
DISTRICT: 1
ROUTE
MILEPOST: 27.100
ROAD
ROAD
ROAD DESIGNATION: R
LATEST ADT COUNT: 2273 IN 2010
LOCATION INFORMATION: RAMP FROM US 62 ONTO I 24 EASTBOUND KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
2010

STATION: 2 STATION: 221 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District #



Station Location & Description (use back if necessary): Left Shoulder Shoulder width Right Shoulder Shoulder width US 62 Type: No shoulders exist Bituminous) Concrete Stabilized N/A Combination Earth Curbed Field Technician DS ramp_countcard-07-2010.ppt

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAY: 12-1 AM 12-1 AM 4-5 AAM 5-6 AAM 11-12 AAM 11-	ROUTE: 10	
g		2601	UND 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	10024R	
93 00 68 99 BETWEEN	AVERAGE	2657	00700000000000000000000000000000000000	WEEK C	-
6-7 AM	GE DAILY	2721	00 00 00 00 00 00 00 00 00 00 00 00 00	MAR OF JULY	17000
28	TRAFFIC	2825	FRI 1755 1755 1755 1755 1755 1755 1755 175	MARSHALL LY 20	21.10
THURSDAY SATURDAY	IC: 2355	2344	S 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	COUNTY TO JULY	VECOVOEN I
MILE P COUNTE DATA S ARRAY LANES	0.	2177	80529111111111111111111111111111111111111	26 2010	VEL 021
POINT TED BY SOURCE Y S COUNTED		2407	MON 32 20 20 20 20 20 20 20 20 20 20 20 20 20		
: 27.1 : CEN OFF : CLASS : 2 TUBES D: ALL LANES		17732	TOTALS 210 149 130 153 280 153 280 514 1066 846 876 796 796 1101 1111 1111 1112 1109 1116 1109 1116 1123 778 528 259	STATION 231	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 ***

Om ×8	PERCENT	TAL VE	09-10PM 10-11PM 11-12PM 12-01AM 01-02AM 02-03AM 02-03AM	02-03PM 02-03PM 04-05PM 04-05PM 05-06PM 07-08PM 07-08PM	07-08AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 11-03BW	TIME	
% HEAVY TRUCKS = 2 AXLES / TRUCK = 2 % TRAILER TRUCKS = % TRAILERS @PK HR= PEAK HOUR BETWEEN 04- TOTAL HOURS = 24	OF TOTAL	TOTAL VEHICLES	7777777	7/19/10 7/19/10 7/19/10 7/19/10 7/19/10	7/19/10	DATE	
UCKS =	0.8	20	.0000-00-	NN-	3-NO-3-3-	MOTOR- CYCLES	
538/ 458/ 467/ 25/	59.2	1478	22868112349	5638211935 561825	0998777 99516	PASSENGER	
165	18.5	461	ั ของงานจะว่า	12225 10047 10047	2222222 24227 24227 24327	£ 600	
100 ES	0.4	11	0000000	-000-	*0	BUSSES	SINGLE
	1.9	47		ンの480800	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	2 AXLE 6 TIRE	GLE UNIT
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	1.5	37	00000-1	0-W-W00	ผนผงผงง	4 OR LESS AXLE	SINGLE
TION FACTOR	17.0	425			11111111111111111111111111111111111111		TRUCKS
~	0.0	_	000000-0	0000000	0000000	MORE AXLE	LER
2 77	0.0	_	0000-000	0000000	0000000	5 OR LESS AXLE	MUL
2497 165 165	0.1	3	0000-000	000000	00000	6 AXLE	TRUCK
= 2.8	0.0	0	0000000	000000	0000000	7 OR MORE AXLE	RAILER
%%%	0.0	0*	0000000	000000		OTHERS	
		2497	1427 1427 1427 1427 1427 1427 1427 1427	103 103 103 103 103 103 103	11211111 12161 1500	TOTAL	

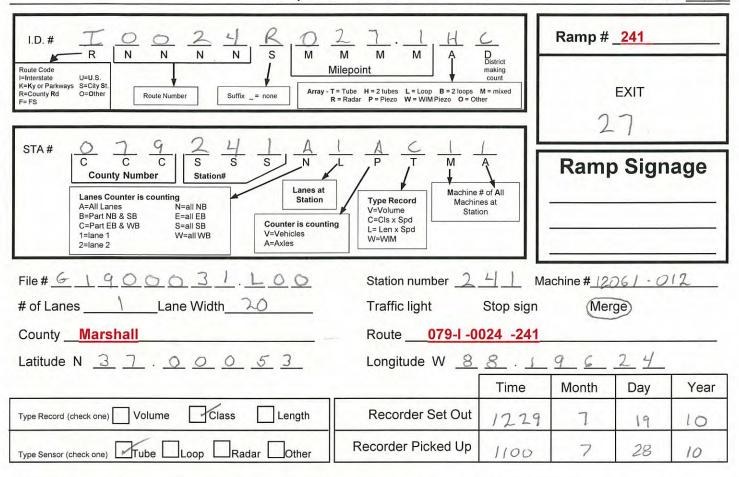
COUNTY: MARSHALL
DISTRICT: 1
MILEPOST: 27.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 2355 IN 2010
LOCATION INFORMATION: RAMP FROM I 24 WESTBOUND TO SI 62

STATION: 231 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION: 7

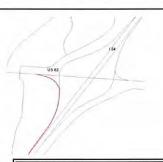
District #



Station Location & Description (use back if necessary):



Field Technician DC ramp_countcard-07-2010.ppt



Type: No shoulders exist
Bituminous Concrete
N/A Stabilized
Combination Earth
Curbed

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: 10	
70R:		1923	T Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z		10024R	
93 00 68 71 BETWEEN 83 BETWEEN	AVERAGE	2144	WED 1 122 1 1 1 2 2 2 1 1 2 2 2 1 1 2 2 2 1 1 2 2 2 2 1 2	WEEK		*P0
11-12 6- 7	AGE DAIL	2176	THU 26 27 28 28 29 29 29 29 29 29 29 29 29 29 29 29 29	OF JULY	MAI	*PORTABLE TI
AM ON	DAILY TRAFFIC:	2257	284 287 272 272 272 273 273 273 273 273 273 27	20 1	MARSHALL (TRAFFIC F
SUNDAY	IC: 1949	2245	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	10 זחרג	COUNTY	RECORDER
MILE P COUNTE DATA S ARRAY LANES	9	2058	SUN	26 2010		REPORT*
E POINT NTED BY N SOURCE NY ES COUNTED		1872	MON			
: 27.1 : CEN OFF : CLASS : 2 TUBES : ALL LANES		14675	TOTALS 127 127 111 111 111 111 111 111 111 111		STATION 241	

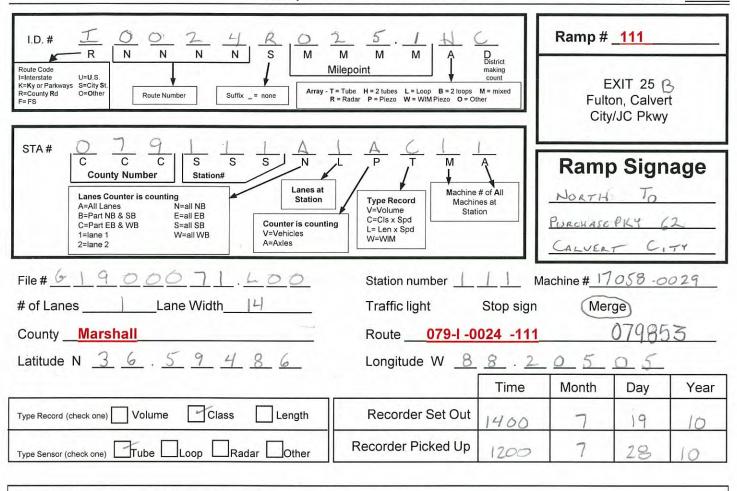
* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 28%

TIME PERIOD DATE CYCLES CARS AXLE BUSSES & TIRE AXLE MORE LASS AXLE MORE CARS AXLE BUSSES & TIRE AXLE MORE LASS AXLE MORE CARS AXLE CARS AXLE CARS AXLE CARS AXLE CARS AXLE MORE CARS AXLE CARS	TP%%A%
PASSENGER 2 AXLE 2 AXLE CARS VEHICLES VEHICLES 3 4 OR 4 OR 5 6 OR	HEAVY TR LES / TR TRAILER TRAILERS AK HOUR I
PASSENGER 2 AXLE 2 AXLE CARS VEHICLES VEHICLES 3 4 OR 4 OR 5 6 OR	UCKS = TRUCKS = APK HR= BETWEEN S =
OTHER 2 AXLE 2 AXLE 2 AXLE MORE LESS AXLE MORE AXLE AXLE AXLE AXLE AXLE AXLE AXLE AXL	598/ 2578/ 472/ 27/ 02-03PM
BUSSES 6 TIRE AXLE MORE LESS AXLE MORE AXLE MORE LESS AXLE MORE AXLE MORE AXLE AXLE AXLE AXLE AXLE AXLE AXLE AXL	41 122 25
TRUCKS OR RE LESS AXLE MORE LESS AXLE MORE LESS AXLE MORE 2 2 3 1 1 5 2 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 6 2 2 3 1 1 6 2 2 1 1 7 0 0 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 1	0 % 311 9 % 1 % 1 CLES
TRUCKS OR RE LESS AXLE MORE LESS AXLE MORE LESS AXLE MORE 2 2 3 1 1 5 2 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 6 2 2 3 1 1 6 2 2 1 1 7 0 0 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 1	
TRUCKS OR RE LESS AXLE MORE LESS AXLE MORE LESS AXLE MORE 2 2 3 1 1 5 2 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 5 2 3 1 1 6 2 2 3 1 1 6 2 2 1 1 7 0 0 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 5 2 1 1 1 1	%%% A
FRUCKS 5 6 OR AXLE MORE 223 1 221 0 223 1 221 0 222 0 223 1 221 0 221 0 222 0 221	SINGL SINGL
FRUCKS 5 6 OR AXLE MORE 223 1 221 0 223 1 221 0 222 0 223 1 221 0 221 0 222 0 221	RRECTI E UNIT E UNIT S AT P
1 N 000000000000000000000000000	
0	инин
5 OR LESS AXLE 000000000000000000000000000000000000	126/ 126/ 35/
TRUCKS 6 7 0 AXLE MOR 0 0 0	466
7 7 0 N N N N N N N N N N N N N N N N N	24.5
%%% O THE RS	% % %
TOTAL VEHICLES 116 113 123 123 123 123 123 124 138 119 127 127 127 127 127 127 127 127 127 127	

COUNTY: MARSHALL
DISTRICT: 1
ROU
MILEPOST: 27.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 1949 IN 2010
LOCATION INFORMATION: RAMP FROM US 62 TO I 24 WESTBOUND

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
7: MARSHALL 2010 STATION: 241 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District #



Station Location & Description (use back if necessary):



Field Technician DS DC ramp_countcard-07-2010.ppt



Left Shoulder	(Y)	N
Shoulder width	_6	_
Right Shoulder	(4)	N
Shoulder width	_7	_
Type: No shou	ılders e	xist
1 100.		
Bituminous	Concr	
		ete

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: 10024R	
FOR: 93 : 100 : 168 R: 7 R: 12		68	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		4R	
BETWEEN	AVERAGE	76	1000c0cccccccccccccccccccccccccccccccc	WEEK OF		The same of the sa
8- 9 AM	DAILY	66	22000000000000000000000000000000000000	JULY	MARSHALL	
ON SAT	TRAFFIC:	75	FRI 7000 - 1000	20 10	HALL COUNTY	
SATURDAY	65	82	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	JULY	NTY	
MILE PO COUNTED DATA SO ARRAY LANES C		67	70002777777777777777777777777777777777	26 2010		
POINT TED BY SOURCE S COUNTED:		57	MON 000000000000000000000000000000000000			
: 25.1 : CEN OFF : CLASS : 2 TUBES : ALL LANE					STATION 111	
NES F		491	TOTAL 3 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		111	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

TP%%A%	ERCENT	TOTAL VEHICLES	07-08AM 07-08AM 08-09AM 09-10AM 11-12AM 12-112AM 01-02PM 03-04PM 04-05PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08AM 07-08AM	77	
% HEAVY TRUCKS : AXLES / TRUCK : X TRAILER TRUCKS : X TRAILERS APK HR PEAK HOUR BETWEEN TOTAL HOURS :	PERCENT OF TOTAL	HICLES	77777777777777777777777777777777777777	DATE	
	0.0	0	000000000000000000000000000000000000000	MOTOR- CYCLES	
10/ 40/ 7/ 05-06PM	42.3	22	00000-0	PASSENGER CARS	
52 = 19.2 % 10 = 4.000 52 = 13.5 % 7 = 14.3 % 7 VEHICLI	38.5	20	00000000WN-NNNN00	OTHER 2 AXLE 4 TIRE VEHICLES	
CLES	0.0	0	000000000000000000000000000000000000000	BUSSES	SIN
	5.8	u	0000000000	2 AXLE 6 TIRE	SINGLE UNIT
%%%A T S S L	0.0 0	0	000000000000000000000000000000000000000	3 4 OR AXLE MORE AXLE	TRUCKS
AXLE CORRECT % SINGLE UNI % SINGLE UNI % TRUCKS AT	0.0 1.9	0	000000000000000000000000000000000000000	4 A	
TON TON PEAK	11.5	6	000000000000000000000000000000000000000	5 AXLE	SINGLE TRAI
	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	TRAILER
0.84 3/ 1/ 2/	0.0	0	000000000000000000000000000000000000000		MULT
52 7 = =	0.0	0	000000000000000000000000000000000000000	AXLE M	I-TRAILER TRUCKS
5.8 % 14.3 % 28.6 %	0.0	0	000000000000000000000000000000000000000		R
	0.0	0*	000000000000000000000000000000000000000	OTHERS V	
		52	0000040040V000000000000000000000000000	TOTAL VEHICLES	

COUNTY: MARSHALL
DISTRICT: 1

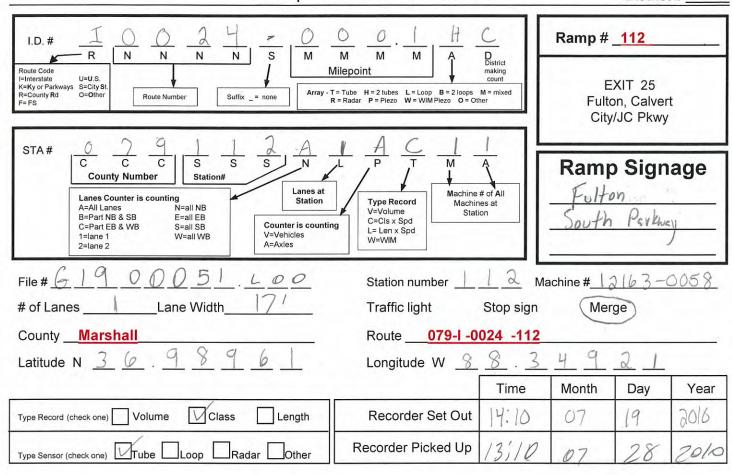
MILEPOST: 25.100

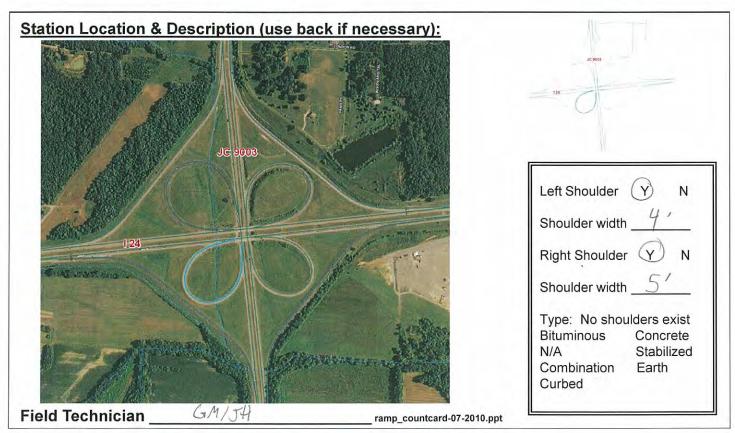
ROAD DESIGNATION: R
LATEST ADT COUNT: 65 IN 2010

LOCATION INFORMATION: RAMP FROM I 24 EB ONTO PURCHASE PARKWAY SB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

7: MARSHALL 2010 STATION: STATION: 111 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:	G	TOTALS:	11000000000000000000000000000000000000			ROUTE: 10024
 100		61	0004MV004/-404VVVA-00-	TUE TUE		
3 BETWEEN	AVERAGE	91	๐๐nn-ผนีอีดดงวีดหหมหมหอ-100c	wED 21	WEEK OF	
57.89	E DAILY	58	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	THU 22	AULY	MA
AM ON SI	Y TRAFFIC:	67	00000044748487478644	FRI 23	20 TO	MARSHALL COL
SUNDAY	57	58	→0→4→→00000000000000000000000000000000	SAT	AJULY	COUNTY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT	7	51	○ N N N N M N M N M O O O O O O O O O O O	SUN 25	26 2010	
OINT D BY OURCE		54	0000000cc4400000cocmco	MON MON		-6.
CEN OFF				10		STATION 112
11		440	11 12 12 12 12 12 12 12 12 12 12 12 12 1	TOTALS		9.51

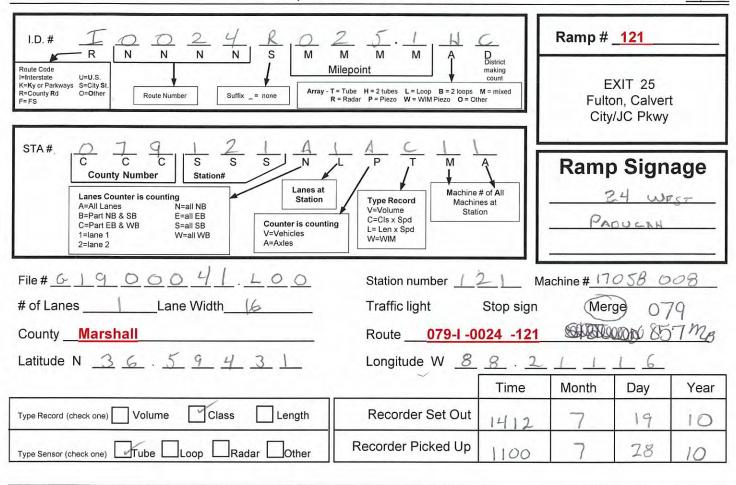
* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

TIME DATE PERIOD DATE 06-07AM 7/19/10 08-09AM 7/19/10 10-11AM 7/19/10 11-12AM 7/19/10 12-03PM 7/19/10 04-05PM 7/19/10 05-06PM 7/19/10 08-09PM 7/19/10 08-09PM 7/19/10 09-10PM 7/19/10 01-01PM 7/19/10 01-01PM 7/19/10 01-02AM 7/19/10 02-03AM 7/19/10 03-04AM 7/19/10 04-05AM 7/19/10 05-06AM 7/19/10	
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CAS COLES	
PASSENGER CARS CARS 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
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T TRUCKS T D PK HOUR	SINGLE TRAILER
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AMORO A PARTICULAR	LER
%%% O	
VEHICLES 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	

COUNTY: MARSHALL
DISTRICT: 1
ROUTE:
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 57 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PKWY SB ONTO I 24 EB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION: 112 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District #



MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: 127 1 1 2 AM 2 3 AM 3 4 AM 7 5 AM 11-12 AM 11-12 AM 12-13 AM 11-12 AM 12-13 AM 13 5 AM 13 5 AM 13 7 AM 13		ROUTE: 10024R	
TOR: 93 : 100 : 168 R: 77 R: 119		913	030 030 040 050 050 050 050 050 050 050 050 05		124R	
BETWEEN	AVERA	894	00000000000000000000000000000000000000	WEEK OF		
11-12 AM 3- 4 PM	AVERAGE DAILY TRAFFIC:	880	TEC 34 4 4 4 7 7 7 8 3 3 4 4 4 7 7 7 8 3 3 3 4 4 4 7 7 7 8 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	F JULY	MAR	
0 N	TRAFFIC	941	12 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	20 10	MARSHALL COUNTY	
SATURDAY	:: 751	697	SAT 4 22 4 22 4 22 4 22 4 22 4 22 4 22 4	ATINE	YTNUC	
MILE PO COUNTED DATA SOI ARRAY LANES CO		484	SUN 57 17 17 17 17 17 17 17 17 17 17 17 17 17	26 2010		
POINT FED BY SOURCE		852	MON 120 120 120 120 120 120 120 120			
CEN CLAS 2 TI					STATION 121	
OFF SS JBES LANES		5661	TOTALS 61 16 116 123 238 273 273 278 286 287 273 278 278 278 278 278 278 278 278 278 278		121	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

TP%%A%	PERCENT	TOTAL VEHICLES	06-07AM 08-09AM 09-10AM 11-12AM 11-12AM 12-01PM 01-02PM 02-03PM 02-03PM 05-06PM 06-07PM 07-08PM 09-10PM 10-11PM 11-12PM 11-12PM 11-12PM 01-02AM 02-03AM 01-05AM	TIME	
% HEAVY TRUCKS = AXLES / TRUCK = % TRAILER TRUCKS = % TRAILERS aPK HR= PEAK HOUR BETWEEN 0 TOTAL HOURS =	PERCENT OF TOTAL	HICLES	777777777777777777777777777777777777777	DATE	
UCKS = UCK = TRUCKS = DPK HR = BETWEEN = S	0.6	ъ	→0000000000→0N0000000	MOTOR- CYCLES	
65/ 243/ 34/ 3-04PM 24	71.9	592	27-27-664121222222244444444444444444444444444	PASSENGER CARS	
823 = 7.0 65 = 3. 823 = 4. 92 = 3. 92 VEH	19.6	161	7N-00-NN-15055500000000000000000000000000	OTHE 2 AX 4 TI VEHIC	
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>° >° > >	1.5	12	00000-0-00-0	3 AXLE	T TRUC
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AUNIECT	1.1	9	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
TON FACTOR T TRUCKS T a PK HR PEAK HOUR	3.0	25	-0000000nnnnnnnnn		TRUCKS
ح ا ا ا ا ا ا	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.94 31/ 3/ 6/	0.0	0	000000000000000000000000000000000000000	5 OR	M C F
823 92 92	0.0	0	000000000000000000000000000000000000000		TRUCKS
633	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		823	254444 24846	TOTAL VEHICLES	

COUNTY: MARSHALL
DISTRICT: 1

MILEPOST: 25.100

ROUTE: I

MILEPOST: 25.100

ROAD DESIGNATION: R

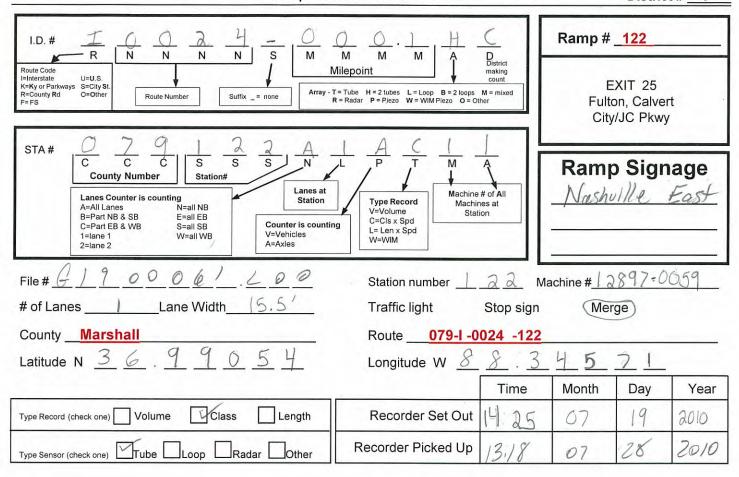
LATEST ADT COUNT: 751 IN 2010

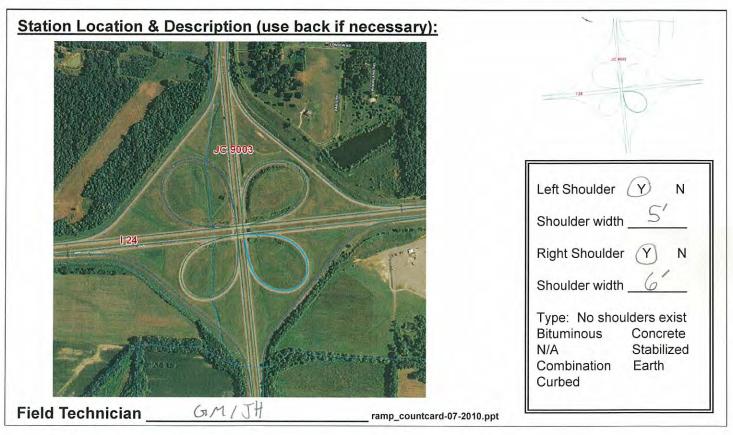
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB ONTO I 24 EB STATION: 121 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION:

District #





MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	12-11 12-11 12-11 12-11 12-11 13-14 14 14 14 14 14 14 14 14 14 14 14 14 1	DATE:	ROUTE: 10024
TOR: 93 : 100 : 168 : 168 : 122 : 76		902	0 17 17 17 17 17 17 17 17 17 17 17 17 17	20	124
0 8 8 2 BETWEEN	AVERA	905	00000000000000000000000000000000000000	WEEK OF	
7- 8 AM 4- 5 PM	SE DAILY	929	1028 1028 1028 1028 1028 1028 1028 1028	F JULY	MAR
20	AVERAGE DAILY TRAFFIC:	923	20244448555544485790291212412	20 10	MARSHALL CO
TUESDAY	: 757	683	2		COUNTY
MILE POI COUNTED DATA SOL ARRAY LANES CO	7	489	NS	26 2010	
BY BY JRCE		874	NN NN NN NN NN NN NN NN NN NN NN NN NN		
CEN OF CLASS CLASS 2 TUBE ALL LA					STATION 122
OFF SS UBES LANES		5705	TOTALS 37 37 29 19 19 20 20 20 20 20 20 20 20 20 20 20 20 20		122

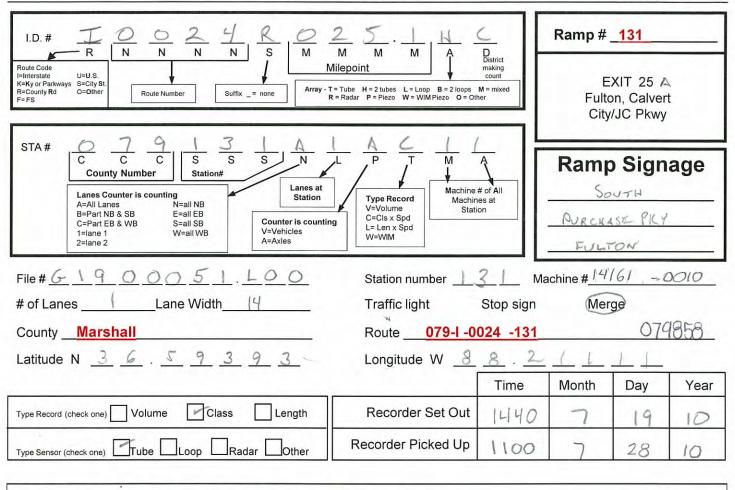
* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

TP%%2%	PERCENT OF	TOTAL VEHICLES	07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 01-02PM 01-02PM 02-03PM 03-04PM 06-07PM 06-07PM 07-08PM 09-10PM 11-12PM 11-12PM 11-12PM 11-12PM 03-04AM 03-04AM 03-04AM	TIME	
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41/ 165/ 28/ 27-08AM 24	77.5	639	100000000000000000000000000000000000000	PASSENGER	
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	1.2	10	00000000000000000000000000000000000000	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
% % % A	0.4	ü	000000000000000000	3 AXLE	TRUC
SING TRUC	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
AXLE CORRECTION % SINGLE UNIT % SINGLE UNIT % TRUCKS AT PI	0.5	4	000000000000	4 OR LESS AXLE	SING
TRUCKS OBAK HOUR	2.9	24	~0000000~0~~~~~nn~nnnnn	5 AXLE	SINGLE TRAILE
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0.95 13/ 3/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
824 89 =	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
3-1-6	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		824	884488884488646 88448688488666 8844868888666	TOTAL VEHICLES	

COUNTY: MARSHALL DISTRICT: 1 ROUTE: MILEPOST: 0.100 ROAD DESIGNATION: R LATEST ADT COUNT: 757 IN 2010 LOCATION INFORMATION: RAMP FROM I 24 EB ONTO PURCHASE PKWY NB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
7: MARSHALL **VEHICLE CLASSIFICATION COUNT** STATION: 122 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District #



Station Location & Description (use back if necessary):



Field Technician ramp_countcard-07-2010.ppt



Left Shoulder N Shoulder width Right Shoulder (Y) Shoulder width Type: No shoulders exist Bituminous) Concrete Stabilized N/A

Combination Earth Curbed

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAY" 12-1 12-1 13-4 AM 1-5-4 AM 5-6-7 AM 11-12-1		ROUTE: 10	
CTOR: 93 R : 100 S : 168 UR: 168 UR: 358		2844	TUE 110 110 110 110 110 110 110 110 110 11		10024R	
B BETWEEN	AVERAGE	2926	43008887777365777777777777777777777777777777	WEEK C		
11-12 AM 5- 6 PM	GE DAILY	3249	12001111111111111111111111111111111111	OF JULY	MAR	A Company
8 S	TRAFFIC:	3447	FRI FRI 1100 1100 1100 1100 1100 1100 1100 11	20 10	MARSHALL CO	100
MONDAY	: 2694	2976	SAT 1158835174 1754 1754 1754 1754	JULY	COUNTY	September 1
MILE P COUNTE DATA S ARRAY LANES		1954	SSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSSS	26 2010		7
POINT FED BY SOURCE COUNTED		2888	MON 2011 11 11 11 11 11 11 11 11 11 11 11 11			
AL CEN					STATION 131	
OFF SS UBES LANES		20284	TOTALS 233 1158 103 89 89 89 89 89 89 89 89 89 89 89 89 89		131	

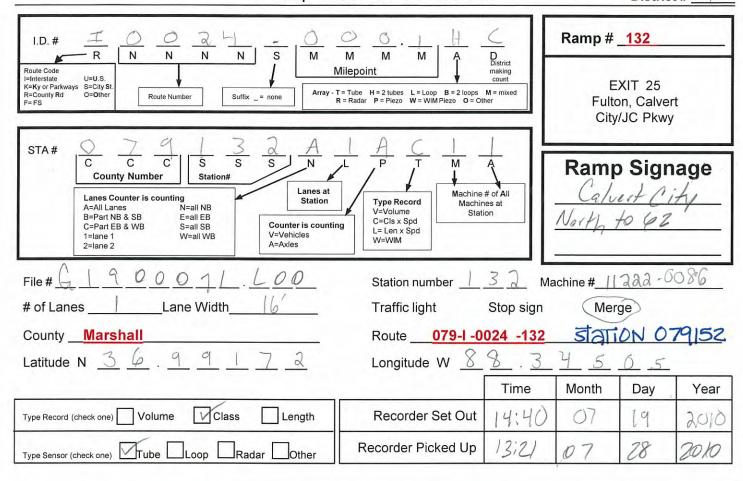
NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 22%

AXL PEATOTOTOT	PERCENT O	TOTAL VEHICLES	07-08AM 08-09AM 08-09AM 11-12AM 11-12AM 11-01PM 01-02PM 02-03PM 03-04PM 04-05PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08AM 01-01AM 01-01AM 01-01AM 01-01AM 01-01AM 01-01AM 01-01AM 01-01AM 01-01AM	TIME	
% HEAVY TRUCKS = AXLES / TRUCKS = % TRAILER TRUCKS = % TRAILERS DPK HR= PEAK HOUR BETWEEN TOTAL HOURS =	OF TOTAL	HICLES	777779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710 7779710	DATE	
UCKS =	0.7	21	000000	MOTOR- CYCLES	
213 887 160 8 05-06P	78.6	2277	33 83 74 78 78 78 78 78 78 78 78 78 78 78 78 78	PASSENGER CARS	
/ 2898 = 7. / 213 = 4. / 2898 = 5. / 283 = 2. M 283 VEH	13.4	387	71-1233323232323233233333333333333333333	OTHE 2 AX 4 TI VEHIC	
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	:	33	NN-00000-0	AXLE	SINGLE UNIT TRUCKS
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AXLE CC % SINGL % SINGL % TRUCK	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
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ווווו	0.4	1	000000000000000000000000000000000000000	NORE NORE	LER
.93 53/ 10/	0.2	6	-0000000000000000000000000000000000	5 OR LESS AXLE	MUL
283 283	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
3.5	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		2898	2108 2111 22222 2111 2112 2112 2113 2113 21	VEHI	

COUNTY: MARSHALL
DISTRICT: 1
ROUTE: I
WILEPOST: 25,100
ROAD DESIGNATION: R
LATEST ADT COUNT: 2694 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB ONTO I 24 EB STATION: 131 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
7: MARSHALL 2010

District #



ramp_countcard-07-2010.ppt



GM/JA

Field Technician

Station Location & Description (use back if necessary):

Left Shoulder Y N
Shoulder width Y N
Shoulder width Y N
Shoulder width W
Type: No shoulders exist Bituminous Concrete N/A Stabilized Combination Earth Curbed

ROUTE: 10024 WEEK OF JULY 20 TO JULY 26 2010 DATE: 20 21 22 23 24 25 26 26 27 21 21 21 21 21 21 21 21 21 21 21 21 21	: 0.1	O BY SOURCE	MILE POINT COUNTED BY DATA SOURCE ARRAY	WEDNESDAY	9	7- 8 AM	BETWEEN	R: 93 : 100 : 168 323	MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR:
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 I UE WED THU FRI SAT SUN MON NON NON NON NON NON NON NON NON NO				2795	TRAFFIC:	DAILY	AVERAG		
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 2010 1 AM 8 8 13 13 10 16 90 18 11 9 10 19 10 10 10 10 10 10 10 10 10 10 10 10 10	21047	3035	2420	2898		3213		3054	TALS:
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 20 21 22 23 24 25 26 TO BEEK OF JULY 20 TO JULY 26 2010 E: 10E WED THU FRI SAT SUN MON TO STATION 137 1 AM 8 8 13 13 10 16 9 17 2 AM 7 10 4 8 16 13 11 2 AM 9 18 13 13 8 7 19 5 AM 9 18 13 13 8 7 19 6 AM 9 18 13 13 8 7 19 6 AM 198 207 186 195 74 55 189 7 AM 282 323 232 211 170 115 229 8 AM 282 323 232 211 170 115 229 10 AM 202 212 196 223 183 137 213 11 AM 202 212 196 223 254 180 198 1 PM 197 180 194 203 193 123 123 2 PM 182 184 219 215 196 219 196 5 PM 189 176 203 216 173 191 186 6 PM 189 176 203 216 173 191 186 7 PM 184 79 167 222 254 199 172 224 9 PM 184 79 67 130 138 99 72 10 AM 202 217 218 190 180 160 140 11 BM 203 218 28 78 78 78 58 38	10	22	21	32	34	26	18	19	12
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 26 27 10 Med Pri July 26 2010 E: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 10024 WEEK OF JULY 20 TO JULY 26 2010 AN	40	38	258	18	77	767	47	7 6	30
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 26 27 27 28 27 28 27 29 28 27 29 28 27 29 28 29 29 29 29 29 29 29 29 29 29 29 29 29	· UT	56	85	117	70	94	82	148	0
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 9 13 10 16 9 12 27 2 AM 7 10 4 8 16 13 11 16 9 13 9 6 12 5 18 9 10 37 10 10 10 10 10 10 10 10 10 10 10 10 10	6	72	99	138	130	67	79	84	8
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 2010 I AM 8 8 13 13 10 16 9 13 9 6 12 5 18 9 10 37	10	140	160	180	190	151	125	124	7
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 I NAM 8 8 13 13 10 16 9 1 AM 8 8 13 13 10 16 9 2 AM 11 9 18 13 13 8 7 10 5 AM 19 18 13 13 8 7 10 5 AM 198 207 186 195 74 55 189 8 AM 222 233 290 268 110 52 307 9 AM 223 233 232 211 170 115 229 10 AM 202 213 230 211 170 115 229 11 AM 202 212 196 225 183 137 213 12 AM 202 212 196 225 238 174 178 1 PM 182 184 219 215 196 219 1 PM 182 184 219 215 196 219 1 PM 189 176 203 203 190 175 188 5 DM 213 103 203 203 196 177 188	12	226	172	100	254	222	187	215	0
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 I NA RED THU FRI SAT SUN MON PRI SAM PRI SAT SUN MON PRI SAM PRI SAT SUN MON PRI SAM PRI	1 1	198	17/	100	202	216	107	212	u t
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 I THE WED THU FRI SAT SUN MON 9 18 13 13 10 16 9 18 7 10 11 9 13 9 6 12 5 18 9 6 12 5 19 10 10 10 10 10 10 10 10 10 10 10 10 10		186	191	373	202	203	727	189	- 6
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 26 27 10 4 8 13 13 10 16 9 18 13 13 18 7 10 10 10 10 10 10 10 10 10 10 10 10 10	14	194	219	196	215	219	184	182	10
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 I TUE WED THU FRI SAT SUN MON B 8 13 13 10 16 9 2 AM 7 10 4 8 16 13 11 3 AM 11 9 13 9 6 12 5 4 AM 9 18 13 13 8 7 10 5 AM 89 81 83 80 44 7 AM 198 207 186 195 74 8 AM 282 323 290 268 110 52 307 10 AM 221 230 211 219 254 180 198 12 AM 202 212 196 222 238 174 178	14	223	223	193	203	194	180	197	_
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 I UE WED THU FRI SAT SUN MON 1 AM 8 8 13 13 10 16 9 2 AM 7 10 4 8 16 13 11 3 AM 11 9 13 9 6 12 5 4 AM 9 18 13 13 9 6 12 5 5 AM 36 39 40 37 19 10 37 6 AM 89 81 83 80 44 35 7 AM 198 207 186 195 74 8 AM 282 323 290 268 110 52 307 10 AM 235 224 238 253 183 137 213 11 AM 221 230 211 219 254 180 198	14	178	174	238	222	196	212	202	12
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 1 1 4 8 16 13 11 9 6 12 5 10 9 9 13 9 6 12 5 10 9 10 37 6 AM 36 39 40 37 19 10 37 6 AM 198 207 186 195 74 55 189 10 AM 222 233 234 253 183 137 213 10 11 12 235 10 AM 222 233 236 253 183 137 213		198	180	254	219	211	230	221	_
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 11 9 13 13 10 16 9 18 13 13 10 16 9 18 13 13 10 16 9 18 13 13 10 16 9 18 13 13 10 16 9 18 13 13 10 16 9 18 13 13 10 16 12 5 10 10 37 10 37 10 3	7.1	213	137	183	253	238	224	235	3
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 9 1 AM 8 8 13 13 10 16 9 2 AM 11 9 13 9 6 12 5 3 AM 9 18 13 13 8 7 10 5 AM 36 39 40 37 19 10 37 6 AM 198 207 186 195 74 55 189	16	307	1152	170	202	042	220	202	00
TE: 10024 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 11 MON 12 AM 11 9 13 13 13 16 13 11 13 11 13 15 AM 11 9 13 9 6 12 5 AM 11 9 18 13 13 8 7 10 5 AM 36 39 40 37 19 10 37 6 AM 89 81 83 80 44 35 105	. 11	189	i Gi	74	195	186	207	198	•
TE: 10024 MARSHALL COUNTY STATION 1 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 11 MO	UT.	105	35	44	80	83	81	89	0
TE: 10024 MARSHALL COUNTY STATION 1 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 11 MON 15 10 16 9 2 AM 7 10 4 8 16 13 11 3 AM 11 9 6 12 5 10	N	37	10	19	37	40	39	36	vi.
TE: 10024 MARSHALL COUNTY STATION 1 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 THU FRI SAT SUN MON 1 AM 8 8 13 13 10 16 9 2 AM 7 10 4 8 16 13 11		30	72	> 0	14	10	18	o -	-
TE: 10024 MARSHALL COUNTY STATION 1 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 TUE WED THU FRI SAT SUN MON 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1	36	16	0 00	44	5	3	N
TE: 10024 MARSHALL COUNTY STATION 1 WEEK OF JULY 20 TO JULY 26 2010 E: 20 21 22 23 24 25 26 THE WED THU FRI SAT SUN MON		9	16	10	13	13	00	100	د ر
MEEK OF JULY 20 TO JULY 26 2010	101	MON 26	SUN 25	SAT	FRI FRI	THU 22	WED 21	TUE 20	ITE:
10024 MARSHALL COUNTY			26 2010	JULY			EX		
	STATION 132			YTN		MARS		24	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

TOTAL HOURS

= 48

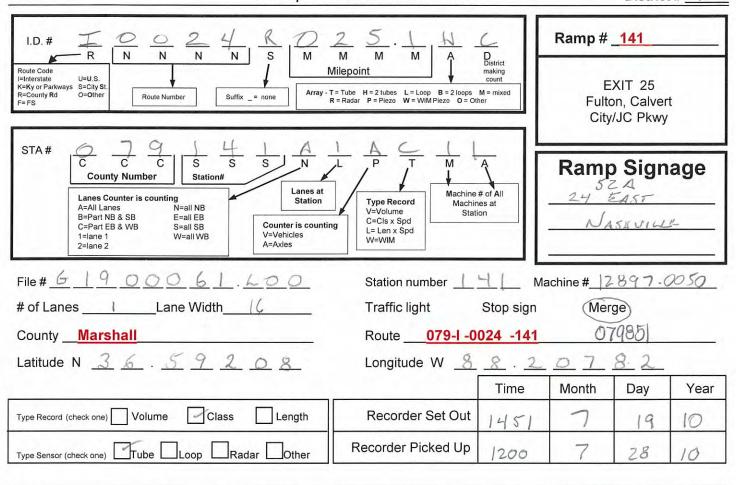
17%% > %	PERCENT	TOTAL VEHICLES	07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 12-01PAM 01-02PAM 02-03PAM 05-06PAM 06-07PAM 07-08PAM 07-10PAM 11-12PAM 11-12PAM 11-12PAM 11-02AAM 03-04AAM 03-04AAM	TIME	
% HEAVY TRUCKS AXLES / TRUCK X TRAILER TRUCKS X TRAILERS DPK HR PEAK HOUR BETWEEN	PERCENT OF TOTAL	HICLES	777777777777777777777777777777777777777	DATE	
TRUCKS = 218/ TRUCK = 908/ R TRUCKS = 170/ RS aPK HR = 4/ R BETWEEN 07-08AM	0.7	22	~000000000~~~~0~0~~~~~	MOTOR- CYCLES	
	76.5	2552	1122336888888888888888888888888888888888	PASSENGER CARS	
3334 = 6. 218 = 4. 3334 = 5. 282 = 1. 282 VEH	16.3	542	282777887777888	0TH 2 A 4 T	
6.5 % 4.165 5.1 % 1.4 % VEHICLES	0.1	Ν	00000000000000000000	BUSSES	
	3	36		2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
%%%P	0.3	10	000000000000000000000000000000000000000	3 AXLE	T TRUC
AXLE CORR % SINGLE % SINGLE % TRUCKS	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
AT	1.4	48	0000000007700707770000007	4 OR LESS AXLE	SINGLE
ION FACTOR T TRUCKS T @ PK HR PEAK HOUR	3.6	120	wua44wawwuur		TRUCKS
пппп	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.93 48/ 2/ 6/	0.1	N	0000000-00000000000000	5 OR LESS AXLE	MUL
3334 282 282	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
= 1.4	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		3334	222 222 223 223 223 223 223 223 223 23	TOTAL VEHICLES	

COUNTY: MARSHALL
DISTRICT: 1
ROUTE:
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 2795 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PKWY NB TO I 24 WB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION: STATION: 132 BOTH E-W ROUTE: 1 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District #



Station Location & Description (use back if necessary):



Field Technician _______ ramp_countcard-07-2010.ppt



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4	
(Y)	N
5	
lders e Concr Stabili Earth	ete
	lders e Concr Stabili

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: 10
CTOR: 93 R : 100 S : 168 UR: 312 UR: 307		3224	TUE 20 20 20 20 20 20 20 20 20 20 20 20 20		10024R
BETWEEN	AVERAGE	3482	TOTAL STATE OF THE PROPERTY OF	WEEK OF	
10-11 4- 5	GE DAILY	3565	1 112222222222222222222222222222222222	F JULY	MAR
AM ON S	TRAFFIC	3941	FRI 33 33 32 244 2570 2778 2778 2778 2778 2778 2778 2778 27	20 T	MARSHALL C
SATURDAY	C: 3142	3197	47774 47	TO JULY	COUNTY
MILE POI COUNTED DATA SOU ARRAY LANES CO	N	2968	SUN SUN 31 31 32 31 31 32 31 31 32 31 31 31 31 31 31 31 31 31 31 31 31 31	26 2010	
POINT ED BY SOURCE		3278	MON 26 MO		10
25.1 CEN OFF CLASS 2 TUBES ALL LANES		23655	TOTALS 200 162 162 318 519 915 917 1180 1273 1180 1273 1586 1614 1596 1618 1618 1618 1618 1618 1618 1618 16		STATION 141

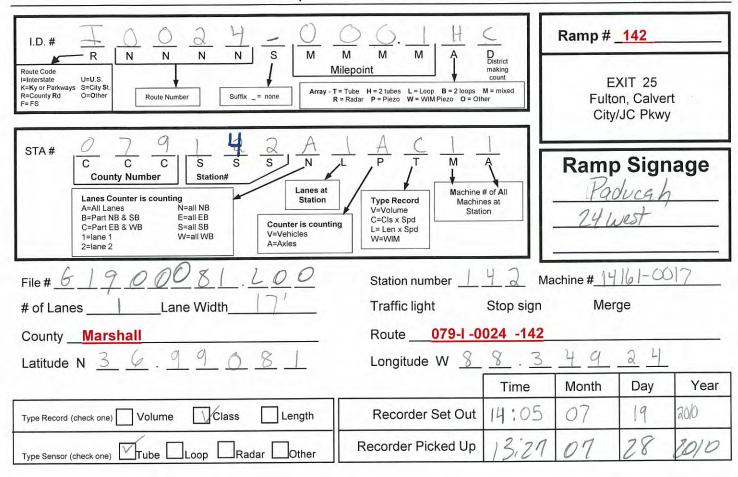
* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

	⊺₽ %% ₽ %	PERCENT	TOTAL VE	07-07AM 08-09AM 09-10AM 11-12AM 12-01PAM 01-02PM 02-03PM 02-03PM 05-06PM 05-06PM 09-10PM 11-12PM 11-12PM 11-12PM 01-02AM 02-03AM 01-02AM 01-05AM	TIME	
200 0000	% HEAVY TRUCKS AXLES / TRUCK % TRAILER TRUCKS % TRAILERS @PK H PEAK HOUR BETWEE TOTAL HOURS	PERCENT OF TOTAL	VEHICLES	777777777777777777777777777777777777777	m	
	27	1.3	42	→→ 00→0→→→0→00000000000000000000000000	ES ?	
	994/3 4080/ 686/3 10-11AM	44.1	1477	088861 0000 0000 0000 0000 0000 0000 000	PASSENGER CARS	
	3353 = 29. 994 = 4. 3353 = 20. 257 = 17. 257 VEH	25.1	840	244595555599559955599555995559955599555	¥20	
	29.6 % 4.105 20.5 % 17.5 % VEHICLES	1.2	39		BUSSES	
		4.7	159	o c c c c c c c c c c c c c c c c c c c	2 AXLE 6 TIRE	SINGLE UNI
	%%% >	3.3	110	W-00	3 AXLE	UNIT TRUCKS
	AXLE CORR % SINGLE % SINGLE % TRUCKS	0.0	0	000000000000000000000000000000000000000		KS
	RRECTION E UNIT THE UNIT OF S AT PEAN	2.4	80	4 D O D O O O O O O O O O O O O O O O O	4 OR LESS AXLE	SINGLE
	ON FACTOR TRUCKS a PK HR EAK HOUR	17.7	593	11 21 22 23 23 24 24 28 27 28 28 28 28 28 28 28 28 28 28 28 28 28	5 AXLE	
	, , , , , , , , , , , , , , , , , , ,	0.1	N	000000000000	6 OR MORE AXLE	TRAILER
	0.76 308/ 29/ 74/	0.3	9	0-000-00-0000	5 OR LESS AXLE	WCF
	3353 257 257	0.1	2	000000000000000000000	6 AXLE	TRUCKS
	= 1.9 28.8	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
	%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
			3353	134 1174 1174 122 122 123 123 123 123 123 123 123 123	TOTAL	

COUNTY: MARSHALL DISTRICT: 1 MILEPOST: 25.100 ROAD DESIGNATION: R LATEST ADT COUNT: 3142 IN 2010 LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB ONTO I 24 WB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
7: MARSHALL **VEHICLE CLASSIFICATION COUNT** STATION: 141 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

District # ___



Station Location & Description (use back if necessary):



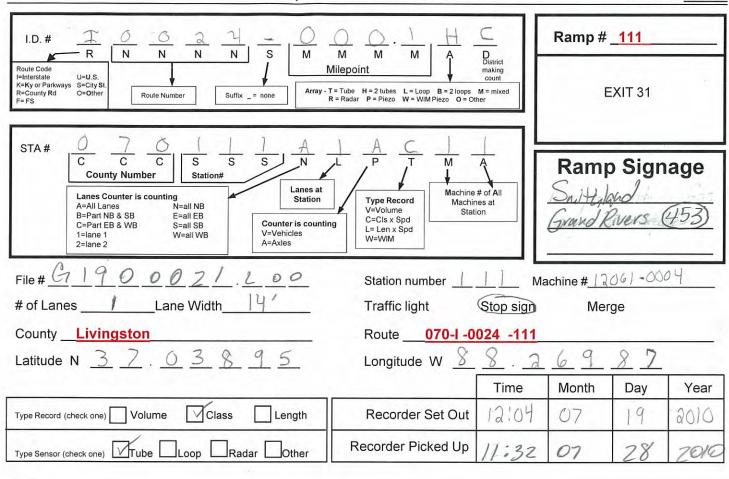
Field Technician GM / TH ______ ramp_countcard-07-2010.ppt

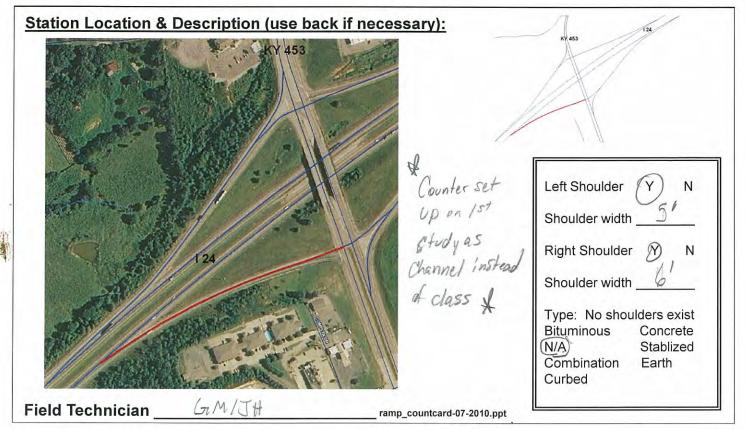


Left Shoulder Y N
Shoulder width Y N
Right Shoulder Y N
Shoulder width 5

Type: No shoulders exist
Bituminous Concrete
N/A Stabilized
Combination Earth
Curbed

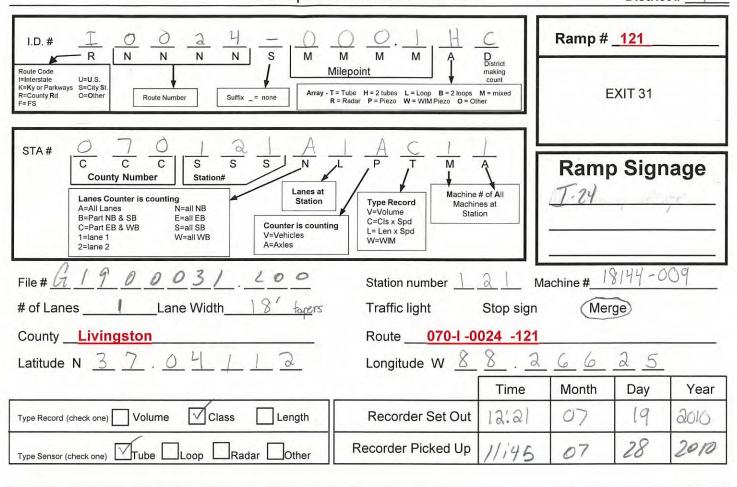
District #

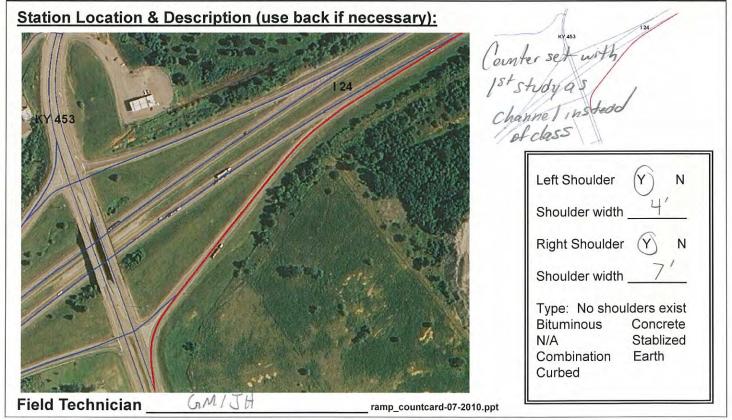




MONTHL AXLE F TOTAL AM HIG		TOTALS	DATE:: DA		ROUTE:	
THLY FACTOR E FACTOR AL HOURS HIGH HOUR		**			10024	
OR: 93		1813	TUE 100 100 100 100 100 100 100 100 100 10		4 RAMP	
BETWE	AVERAGE	1731	12777777777777777777777777777777777777	WEEK		, 70
EN 10-11	AGE DAILY	1842	7727 7727 7727 7727 7727 7727 7727 772	OF JULY	E	PUKTABLE
PM ON	LY TRA	195	SFR 23 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	. ү 20	LIVINGSTON	IKAFFIC
FRIDAY	TRAFFIC:	7 1	7-0064M8-4677-46-748M-9440HM	01		CKECC
AYY	1590	658	4 1 4 2 4 2 4 2 4 2 4 2 4 2 4 2 4 2 4 2	JULY	COUNTY	KECOKDEK K
MILE PO COUNTED DATA SO ARRAY LANES		1372	SUS	26 2010		KETOK I
POINT FED BY SOURCE		1598	225550021100112855556484842668			
CEN OFF CLASS TUBES ALL LANES					STATION	
FF ES ANES		11971	TOTALS 113 64 29 43 29 218 22 218 279 691 749 818 818 819 819 1010 1010 1018 1028 818 819 819 819 819 819 819 819 819 81		111	

District #

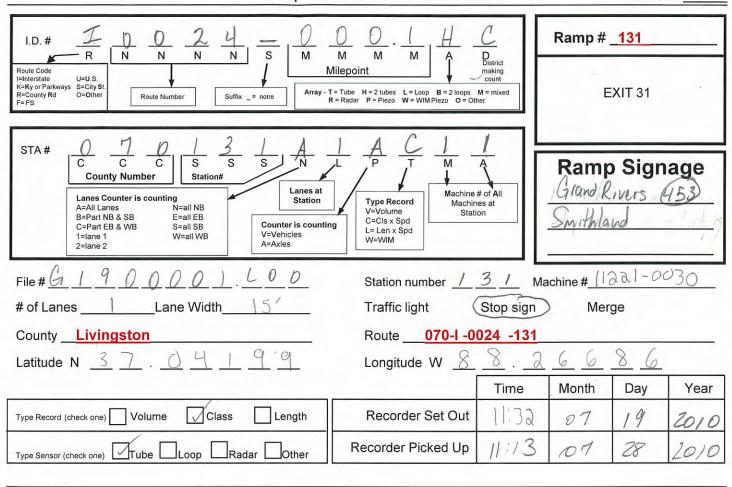


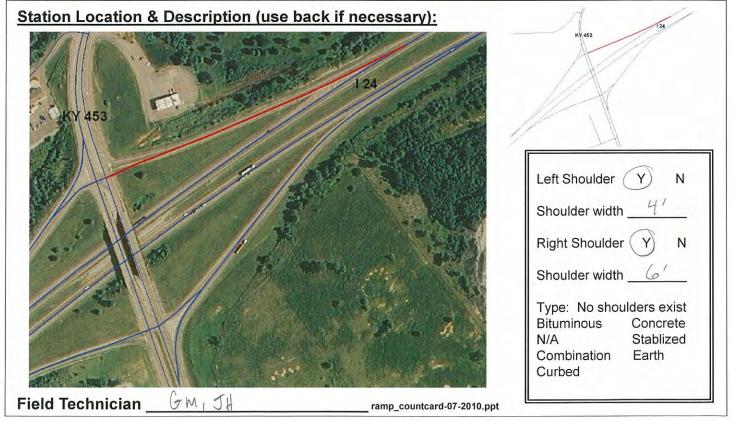


MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: 12-1 AM 12-1 AM 2-3 AM 3-4 AM 6-7 AM 7-8 AM 11-12 AM 11		ROUTE: 10	4
CTOR: 93 : 100 S : 168 JR: 81 JR: 83		599	0 H C C C C C C C C C C C C C C C C C C		10024 RAMP	
BETWEEN	AVERAGE DAILY TRAFFIC:	648	00000000000000000000000000000000000000	WEEK OF		*POR
10-11 AM 4- 5 PM		681	THU	F JULY	LIVIA	*PORTABLE TRA
90		754	22 22 23 24 24 24 24 24 24 24 24 24 24 24 24 24	20 TO	IVINGSTON CO	TRAFFIC RE
SUNDAY	676	870	SAT 4 5 5 5 5 6 6 3 5 6 6 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6	JULY	COUNTY	RECORDER R
MILE POIN COUNTED B' DATA SOUR ARRAY LANES COU		928	SUN	26 2010		REPORT*
OINT D BY OURCE		615	7 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2			
CEN CLAS 2 TL					STATION	
OFF SS JBES LANES		5095	TOTALS 130 130 131 134 193 255 193 318 318 318 318 316 316 316 316 316 316 316 316 316		121	

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACT AXLE FACTOR TOTAL HOURS AM HIGH HOUP PM HIGH HOUP		TOTALS:	DATE: DAY: 12-1 1-2 1-2 2 3-4 3-4 3-4 3-4 3-7 8-9 10-11 11-12 11-1		ROUTE:	
FACTOR: 93 STOR: 100 DURS: 168 HOUR: 115 HOUR: 111		746	TUE 10 20 20 20 20 20 20 20 20 20 20 20 20 20		10024 RAMP	
BETWEEN	AVERA	835	1 0 0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	WEEK OF		
11-12 AM 6- 7 PM	AVERAGE DAILY	857	THU	F JULY	LIVI	
22	TRAFFIC:	1144	FRI 23 FRI 35 66 68 53 66 68 53 66 68 53 66 68 53 68 68 68 68 68 68 68 68 68 68 68 68 68	20 TO	LIVINGSTON COUNTY	CS 10 C SO
SATURDAY	: 850	1200	SAT 115 116 116 117 117 117 117 117 117 117 117	JULY	DUNTY	
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT		899	SUN	26 2010		
₽		721	M8 4 4 4 7 5 5 5 5 6 5 6 7 4 4 4 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6		s	
0.1 CEN OFF CLASS 2 TUBES ALL LANES					STATION 131	
E.		6402	TOTALS 63 42 442 442 442 442 444 444 444 444 444		31	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

% HEAV AXLES % TRAI % TRAI PEAK H	PERCENT OF TOTAL	TOTAL VEHICLES	77-08-20 8-09-20 9-10		
/Y TRU	TOTAL	CLES		1	
CKS = CKS = RUCKS = APK HR	4.2	37	ころでっていることなるできるようでしているのので	MOTOR- CYCLES	
31/ 146/ 28/ 1/ 04-05PM	91.1	794	10000000000000000000000000000000000000	PASSENGER CARS	
872 = 3.6 31 = 4.7 872 = 3.2 72 = 1.4 72 VEHI	1.1	10	00000000000	2 AXLE 4 TIRE VEHICLES	
3.6 % 4.710 3.2 % 1.4 % VEHICLES	0.0	0	000000000000000000000000000000000000000	BUSSES 6	SINGLE
	0.3	W	000000000000000000000000000000000000000	AXLE	
%%%A	0.0	0	000000000000000000000000000000000000000	m	UNIT TRUCKS
AXLE CORF % SINGLE % SINGLE % TRUCKS	0.0	0	000000000000000000000000000000000000000	01/2/2004	KS
AUNEC	0.2	2	0000000000000000	4 OR LESS AXLE	SINGLE
TION FACTOR	2.8	24	-0N0N		TRUCKS
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0.95 3/ 1/ 2/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
872 72	0.2	N	0000000000000000000	6 AXLE	TRUCKS
= 0.3 = 1.4	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	RS	
		872	75566566666666666666666666666666666666	VEHICLES	

COUNTY: LIVINGSTON
DISTRICT: 1
MILEPOST: 0.100
ROAD DESIGNATION: R
LATEST ADT COUNT: 850 IN 2010
LOCATION INFORMATION: RAMP FROM I 24 WB ONTO KY 453

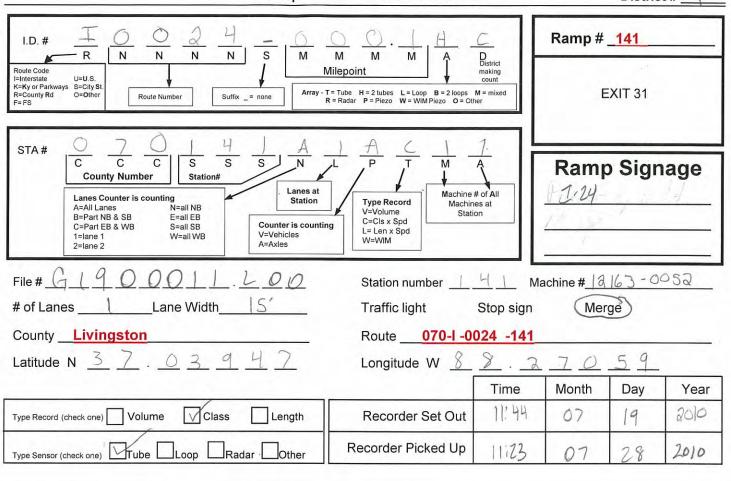
STATION: 131 BOTH E-W ROUTE: I 24R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

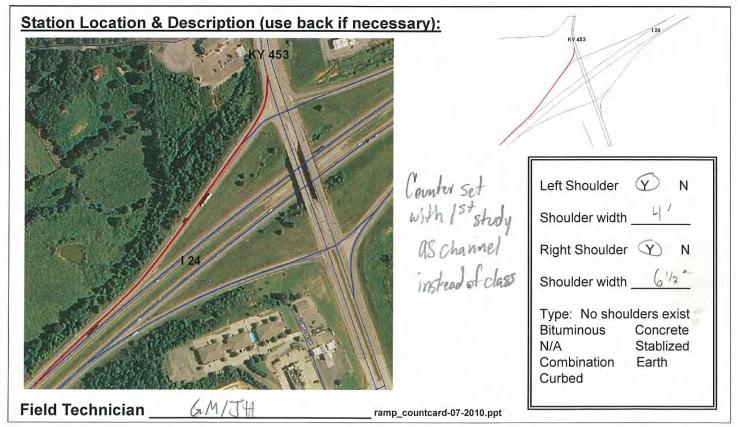
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

Y: LIVINGSTON STATION:

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: 100	
TOR: 93 : 100 : 168 : 168 : 1675		2200	T C C C C C C C C C C C C C C C C C C C		10024 RAMP	
BETWEEN	AVERA	2000	TED 3054 1074 1074 1074 1074 1074 1074 1074 107	WEEK OF		
11-12 AM 4- 5 PM	AVERAGE DAILY TRAFFIC:	2161	T 22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	F JULY	LIVI	
22	TRAFFIC	2309	FRI 300 8 8 1 3 1 3 1 3 1 2 5 6 6 6 8 8 1 3 1 3 1 3 1 2 5 6 6 6 8 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1	20 10	LIVINGSTON COUNTY	
FRIDAY SATURDAY	: 1935	2095	SAT 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	AULY	YTNUO	0000000
MILE POI COUNTED DATA SOL ARRAY LANES CO		1887	2448773481313131313131313131313131313131313131	26 2010		
POINT ED BY SOURCE		1921	00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			
CEN CLA					STATION 141	
OFF SS SS LANES		14573	TOTALS 59 59 59 53 159 390 748 853 985 985 985 985 985 986 1000 1050		141	

Kentucky Transportation Cabinet - Division of Planning

Traffic Count Record District # Delete [Add Combine 0 I.D. # Must be accompanied by reason Ν N N s М М Estimate Milepoint Route Code I=Interstate making V=U.S. K=Ky or Parkways R=County Rd F=FS S=City St. O=Other -T=Tube H=2 tubes R≃Radar P=Piezo L = Loop B = 2 loops W = WIM Piezo Suffix _= none Special Route Numbe Index STA# **ESTIMATE** S **County Number** Station# # of houses _ Lanes Counter is counting
N=att NB Lanes at Machine # of All Station Type Record Machines at A=All Lanes B=Part NB & SB C=Part EB & WB # of Business _ x 25 = V=Volume Station E=all EB C=Cls x Spd L= Len x Spd S=all SB Counter is counting 1≈lane 1 W=all WB V=Vehicles W≍WIM A≃Axles 2≂lane 2 Total File # 6 2 1 0 0 0 4 Machine #_ Station number County name ___ City name Route _ Road/Street name Longitude W Year Time Month Day Class Length Recorder Set Out Type Record (check one) Volume 1 Recorder Picked Up 29 Other 1000 10 _Tube L Loop L JRadar ¹ Type Sensor (check one)

Station Location & Descrip	tion (use baci	<u>k it necessary</u>	<u>):</u>	, 4
			\cdot	W-E
P. Control	\		10	KY 337 \$
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Field Technician	7.17			over (more on back)
TIEIU TECHINOIAN	** ** <u></u>		countcard04-01.ppt	

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS: 70	DATE: 12-11 12-12 13-45 14-15 14-16 14-16 14-16 15-65 16-76	ROUTE: KY9003
97 100 168 458 552		7087	1 22 1 22 1 22 1 22 1 23 1 23 1 23 1 23	
BETWEEN BETWEEN	AVERAGE	7549		VEEK OF
10-11 3-4	E DAILY	5974	S CONNADANANANONOC-	<u>j</u>
PM ON ON	Y TRAFFIC:	5056	S ←_QMUM44MUMQQQ←←	RAVES 22
FRIDAY FRIDAY	IC: 6333	6626		COUNTY
MILE POI COUNTED DATA SOU ARRAY LANES CO	W	6647		28 2010
POINT ED BY SOURCE		6769	2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
12.6 CEN OFF CLASS 2 TUBES ALL LANES		45708	TOTALS 578 429 429 371 915 1646 2313 2267 2687 2687 2927 2927 1883 1589 1491 1243	STATION 310

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 ***

T D % % A %	PERCENT OF	TOTAL VE	05-05AM 07-08AM 08-09AM 109-10AM 11-12AM 11-12AM 01-02PM 02-03PM 05-05PM 09-10PM 11-12PM 11-12PM 01-01AM 02-03AM 03-04AM	28	
% HEAVY TRU AXLES / TRU % TRAILER T. % TRAILERS: PEAK HOUR B	ТО	VEHICLES	77777777777777777777777777777777777777	16	·
CKS =	1.0	64		MOTOR- CYCLES	
2074/ 8598/ 1659/ 109/ 03-04PM	48.2	3174	522222366711116888222223884700688871116888670068871111688867006887111688871168871687168716871687168716	PASSENGER CARS	
6590 = 31.5 2074 = 4.1 6590 = 25.2 481 = 22.7 481 VEHI	19.4	1278	25288448888888888234	OTHER 2 AXLE 4 TIRE VEHICLES	
146% 146%	0.8	54	しことよななななななのでのでしょうとしているとなるなななななななななななななななななななななななななななない。	BUSSES	SINGLE
	3.9	260	などんだんだんだんだんなくなる ひょくしょくり	2 AXLE 6 TIRE	GLE UNIT
**** ****	1.1	74		3 4 AXLE M	TRUCKS
SINGL SINGL SINGL	0.4	27	004-43434 000000000000000000000000		S
LE CORRECTI SINGLE UNIT SINGLE UNIT TRUCKS AT P	10.9	720	00200111 0020001111 0020000000000000000	4 OR LESS AXLE	SINGL
ON FACTOR TRUCKS a PK HR EAK HOUR	12.7	840	00000000000000000000000000000000000000		RUCKS
≅≅	0.6	38			LER
. 75 415/ 26/ 135/	0.7	43	400000000000000000000000000000000000000	5 OR LESS AXLE	MUL1
6590 = 481 = 481 =	0.1	9	00-000-N000000000-0	6 AXLE	TRUCKS
	0.1	9	000000000000000000000000000000000000000	7 OR MORE AXLE	RAILER
36 34 34	0.0	o *	000000000000000000000000000000000000000	OTHERS	
		6590	3341 327 327 327 327 327 327 327 327 327 327	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

ROUTE:
MILEPOST: 12.600

ROAD DESIGNATION: R

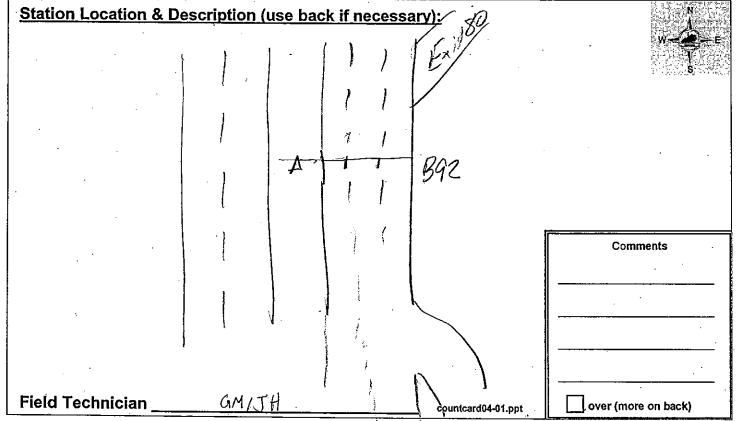
LATEST ADT COUNT: 6333 IN 2010

LOCATION INFORMATION: PURCHASE PARKWAY JUST SOUTH OF KY 339 KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
2010
STATION: STATION: 310 BOTH N-S ROUTE: KY9003
TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District #

	Journ Necolu			District	#
Filidardate U=U.S. K=Ky or Parkways S=Cky St. R=County Rd C=Other Route Number Cutty T Table Array -T = To	A M M A A A A A A A A A A A A A A A A A	D District making count	Add Must be acco	imate	
			[] Ind	ex	
STA# $\begin{bmatrix} O & 4 & 2 & 3 & 4 & 3 \\ C & C & S & S & S & N & 1 \end{bmatrix}$	AVI	1			
C C C S S S N Lanes Counter is counting A=All Lanes N=all NB B=Part NB & SB E=all EB C=Part EB & WB S=all SB 1=fane 1 W=aft WB Z=lane 2 C C S S S N Lanes at Station# Lanes at Station# Counter is counting V=Vehicles A=Axles	# of All es at on	# of Business x 25 =			
File# <u>G 2000171. L00</u>	Station number B	9 <u>2</u> Ma	chine # <u> </u> 8	286 - Ci	22
County name	City name				
Route <u>JC 9003</u>	Road/Street name		-		
Latitude N <u>3 6 . 7 3 4 9 2</u>	Longitude W <u></u>		67	<u>3, 2</u>	· · · · · · · · · · · · · · · · · · ·
		Time	Month	Day	Year
Type Record (check one) Volume Class Length	Recorder Set Out	15:24	07	90	2010
Type Sensor (check one)	Recorder Picked Up	9.17.5	07	29	2010
				•	



PORTABLE TRAFFIC RECORDER REPORT

MONTHLY FACTOR: 97 AXLE FACTOR: 100 TOTAL HOURS: 168 AM HIGH HOUR: 612 PM HIGH HOUR: 701		TOTALS: 8152	DATE: DAY::	ROUTE: KY90US	
BET WEEN	AVERAGE	8513		VEEK OF	
7- 8 AM 4- 5 PM		8255	T	Ĕ) }
22	DAILY TRAFFIC:	5470		GRAVES COU Y 21 TO	
MONDAY Thursday	: 7063	4708		COUNTY	:
MILE PO COUNTED DATA SO ARRAY LANES C		7844	M 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	27 2010	
POINT FED BY SOURCE		8037	11222222222222222222222222222222222222	ro.	
21.7 CEN OFF VOLUME TUBE ALL LANES		50979	TOTALS 467 491 451 666 1282 2352 23880 33189 3211 3211 33183 3187 1542 1132	STATION B92	

Kentucky Transportation Cabinet - Division of Planning

Traffic Count Record District # Add Delete Combine I.D.# Must be accompanied by reason М Estimate Milepoint Route Code t=interstate K=Ky or Parkways Array - T = Tube 'H = 2 tubes L = Loop B = 2 loops R = Radar P = Piezo W = WiM Piezo R=County Rd F= FS Special Index STA# C c **ESTIMATE County Number** Station# # of houses ____ Lanes Counter is counting
N=all NB Machine # of All Station Type Record Machines at # of Business _____ x 25 = A=All Lanes B=Part NB & \$B C=Part EB & WB V=Volume Station E=all EB C=Cls x Spd Counter is counting S≂att SB L= Len x Spd V=Vehicles W=all WB W=WIM File # 6 2000081 L00 Station number A 0 7 Machine #_ City name Route Road/Street name Longitude W $\underline{\mathcal{S}}$ $\underline{\mathcal{S}}$. $\underline{\mathcal{S}}$ $\underline{\mathcal{S}}$ Latitude N Month Day **Year** Time Class Volume Length Recorder Set Out z_{z} 10 Type Record (check one) 0800 Recorder Picked Up ID Type Sensor (check one) __Radar Station Location & Description (use back if necessary): Comments

Field Technician

over (more on back)

countcard04-01.ppt

MONTHLY FACTOR: 99 AXLE FACTOR: 100 TOTAL HOURS: 168 AM HIGH HOUR: 601 PM HIGH HOUR: 560		TOTALS: 6305	DATE: DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: KY9003	
BETWEEN	AVERAGE	6721	1111284 50000 50000 50000 50000 5000 5000 500	WEEK OF		
7- 8 AM 4- 5 PM	DAILY	6828	FRI 38 57 57 57 57 57 57 57 57 57 57 57 57 57	ישורא	GRA	
22	TRAFFIC:	4798	12000000000000000000000000000000000000	21 10	GRAVES COUNTY	
MONDAY	: 5849	4085	SUN SUN SUN SUN SUN SUN SUN SUN	JULY	ATA	
MILE PO COUNTED DATA SO ARRAY LANES C		6359	MON 26 25 25 25 25 25 25 25 25 25 25 25 25 25	27 2010		!
POINT ED BY SOURCE		6268	1100 1100 1100 1100 1100 1100 1100 110			
: 22.8 : CEN OFF : CLASS : 2 TUBES : ALL NB LNS		41364	TOTALS 293 229 317 229 317 2266 2266 2578 2578 2578 2578 2578 2578 2578 2578		STATION AO7 N	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00
OTHER 2 AXLE 4 TIRE VEHICLES 00
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 36 36 36 TOTAL HOURS

п 24

178828 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	PERCENT OF TOTAL	TOTAL VE	07-07AM 07-08AM 09-10AM 11-11AM 11-11AM 11-12AM 12-03PM 03-04PM 03-06PM 07-08PM 07-08PM 07-08PM 01-11PM 11-12PM 11-12PM 01-03AM 03-04AM 03-04AM	TIME PERIOD
RAI EAV	OF TOTAL	VEHICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	DATE
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		5997	250 250 250 250 250 250 250 250 250 250	TOTAL VEHICLES

COUNTY: GRAVES
DISTRICT: 1
MILEPOST: 22.800
ROAD DESIGNATION: U
LATEST ADT COUNT: 11571 IN 2010
LOCATION INFORMATION: BTWN KY 80 AND KY 121

SINGLE UNIT TRUCKS

SINGLE TRAILER TRUCKS

MULTI-TRAILER

STATION: A07 N ROUTE: KY9003 TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
STATION: /

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District # Delete Combine 0 I.D. # Must be accompanied by reason М Ν Ν s Route Code
I=Interstate
K=Ky or Parkways
R=County Rd
F=FS Milepoint Estimate U=U.S. S=City St. O=Other Array - T = Tube : H = 2 tubes L = Loop B = 2 to R = Radar P = Piezo W = WIM Piezo Route Number Suffice _= none Special Index STA# c **ESTIMATE** County Number Station# Lanes Counter is counting
N=all NB # of houses Lanes at Machine # of All Station Type Record Machines at A=All Lanes
B=Part NB & SB
C=Part EB & WB # of Business _ _x 25 ≃ _ V=Volume Station E=all EB C=Cls x Spd S=all SB Counter is counting L= Len x Spd W=WIM V=Vehicles W=all WB A≒Axles Total Machine # 18286 File # 62000071.L00 Station number 📶 City name County name Route Road/Street name Longitude W 🥏 Latitude N Time Month Day Year المجارية وسرا Class Volume Length Recorder Set Out Type Record (check one) Recorder Picked Up 0800 29 10 _Tube l Loop _lRadar Other Type Sensor (check one) Station Location & Description (use back if necessary): Comments 7

Field Technician

over (more on back)

-countcard04-01.ppt

MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR: BM HIGH HOUR: 5		TOTALS: 6126	DATE: DAY: DAY: DAY: 12-1 DAY: 1-2-1 1-2-3 3-4 2-3-3 3-4 3-4 3-4 3-1 1-1 1-1 2-1 1-1 1	ROUTE: KY9003	
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11-12 AM 4- 5 PM	E DAILY	6516	T	ŗ	
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* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00

2626

7 % % A X L T O T P E A X L T O T P E A	PERCENT	TOTAL VE	05-05AM 07-08AM 08-09AM 109-10AM 11-12AM 11-12AM 01-02PM 02-03PM 05-06PM 05-06PM 09-10PM 11-12PM 11-12PM 01-02AM 01-02AM 01-03AM 01-05AM	I ME	
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% % %	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		5876	135 135 135 135 135 135 135 135 135 135	TOTAL VEHICLES	

COUNTY: GRAVES DISTRICT: 1 MILEPOST: 22.800 ROAD DESIGNATION: U LATEST ADT COUNT: 11571 IN 2010 LOCATION INFORMATION: BTWN KY 80 AND KY 121 KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT** Y: GRAVES ZO10 STATION: # STATION: A07 S ROUTE: KY9003 TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 % OTHER 2 AXLE 4 TIRE VEHICLES 00 % 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

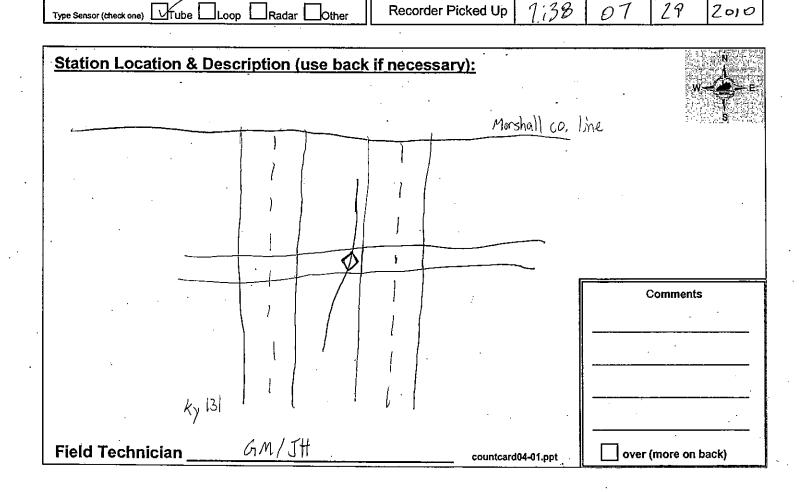
<u> </u>	AXL AXL PEA TOT	PERCENT OF	TOTAL VE	05-07AM 07-08AM 08-09AM 09-10AM 11-11AM 11-11AM 11-01PM 01-02PM 02-03PM 03-04PM 05-06PM 06-07PM 07-08PM 08-07PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 08-07PM 08-07PM 08-07PM 08-07PM 08-07PM 08-08PM 08-08PM 08-08PM 08-08PM	x ⊷ z	
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	39 VE 113	24.4	2894	811624 81624 81624 81624 81624 81624 81624 81624 81624 81624 8162	OTHER 2 AXLE 4 TIRE VEHICLES	
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COUNTY: GRAVES DISTRICT: 1 MILEPOST: 22.800 ROAD DESIGNATION: U LATEST ADT COUNT: 11571 IN 2010 LOCATION INFORMATION: BTWN KY 80 AND KY 121 KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT** 2010 STATION: / STATION: AO7 ROUTE: KY9003 TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District # Add Delete Combine I.D.# Must be accompanied by reason Route Code Milepoint Estimate K≂Ky or Park S=City St O=Other L=Loop B=2 loops W=WIM Piezo Array - T = Tube | H = 2 tubes | R = Radar | P = Piezo R≂County Rd Route Number Suffix _= none Special F=FS Index STA# **ESTIMATE County Number** Station# #.of houses Lanes at Machine # of All Lanes Counter is counting Station Type Record Machines at N≖ali NB A=All Lanes B=Part NB & SB x 25 = # of Business _ E=all EB Station C=Cls x Spd C=Part EB & WB Counter is counting S≔all SB L= Len x Spd V=Vehicles A=Axles 2≃lane 2 Total File# G 200041.L00 5 Machine # 14772-1059 Station number County name / / raves City name 903 Route Road/Street name Latitude N Longitude W X Time Month Day Year Volume Class Type Record (check one) Length Recorder Set Out дO 2010

Recorder Picked Up



MONTHLY FACTOR: 97 AXLE FACTOR: 100 TOTAL HOURS: 168 AM HIGH HOUR: 448 PM HIGH HOUR: 530		TOTALS: 6452	DATE: DAY: DAY: 12-1 AM 12-1 AM 12-1 AM 13-2 AM 2-2 AM 3-4-5 AM 5-6 AM 183 7-8 AM 183 7-8 AM 183 7-8 AM 386 10-11 AM 387 11-12 AM 387 11-2 PM 387 416 5-6 PM 416 5-6 PM 437 8-9 PM 437 8-9 PM 437 8-9 PM 437 11-12 PM 1134 1134		ROUTE: KY9003	-
	AVERAGE	6530	11222445484333322 122244443333322 12234443841333322 12234443841333322 12234434843377778333333333333333333333333	WEEK OF		*PORTABLE
υ <u>¬</u>	E DAILY	7031	L R R R R R R R R R R R R R R R R R R R	AJNF	GR,	
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KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION ***

% HEAVY TRUCKS = AXLES / TRUCK = % TRAILER TRUCKS = % TRAILERS APK HR= PEAK HOUR BETWEEN (TOTAL HOURS =	PERCENT OF TOTAL 2.5	TOTAL VEHICLES 149	05-07AM 7/20/10 6 08-09AM 7/20/10 6 08-09AM 7/20/10 6 11-12AM 7/20/10 7 10-11AM 7/20/10 11 12-01PM 7/20/10 11 12-01PM 7/20/10 9 02-03PM 7/20/10 9 04-05PM 7/20/10 9 06-07PM 7/20/10 5 07-02AM 7/20/10 5 01-02AM 7/20/10 1 03-04AM 7/20/10 2 05-05AM 7/20/10 2	TIME MOTOR- ERIOD DATE CYCLES		
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C 2 % % 9 %	- 08	109	454688097885553333221113	BUSSES	SIN	100 ON: MATI
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% % %	0.0	0 *	000000000000000000000000000000000000000	OTHERS	_	
		6059	275 378 378 378 378 377 445 475 476 661 1776 661 1776 661 1776 661 1776	TOTAL VEHICLES		

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS

888

% % %

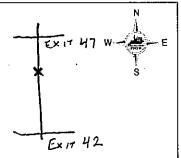
Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District # Add Delete Combine I.D. # Must be accompanied by reason M М Route Code l=Interstate K=Ky or Parkway R=County Rd Milepoint Estimate making ປ=**ບ**.S. S≂City St. O=Other Array - T = Tube H = 2 tubes R = Radar P = Piezo M = mixed Special F= FS Index STA# **ESTIMATE County Number** Station# # of houses _ Lanes Counter is counting
N=all NB Machine # of All Station Type Record V≃Volume Machines at A=All Lanes B=Part NB & SB # of Business _ E=all EB C=Cls x Spd Counter is counting C=Part EB & WB S=all SB L= Len x Spd V=Vehicles W≓WIM A=Axles 2≃lane 2 Total Station number 582 Machine # 18144 002 File # <u>G 2 0 00 0 31 . L 0 0</u> County name _ MAKSWALL City name Road/Street name PURCHASE PAKKWAY Route TR 9003 Latitude N 3 6 . 5 4 1 0 0 Longitude W <u>8</u> <u>8</u> . <u>2</u> <u>0</u> <u>8</u> Time Month Day Year +Class Volume Length Type Record (check one) Recorder Set Out 20 0905 10 Recorder Picked Up 1400 28 Type Sensor (check one) Lube Loop _Radar [10

Station Location	& Description	(use back if	necessary):
		•	

DC

Field Technician



Comments
-
·
· · ·
over (more on back)

countcard04-01.ppt

MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	120 DD A T		ROUTE: KY9003
CTOR: 97 R : 100 S : 168 UR: 1160 UR: 1493		17391	1237 257 257 257 257 257 257 257 257 257 25		003
BETWEEN BETWEEN	AVERAGE	18155	7740 11434 1	HEEK OF	
11-12 AH 4- 5 PH	GE DAILY	18862	100 100 100 100 100 100 100 100 100 100	ר זערג	MAR
22	TRAFFIC:	14765	24 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	21 10	MARSHALL CO
FRIDAY THURSDAY	: 16149	12870	1933 128 128 128 128 128 128 128 128 128 128	AULY	COUNTY
MILE P COUNTE DATA S ARRAY LANES	_	17127	1000 1100 1100 1100 1100 1100 1100 110	27 2010	
POINT FED BY SOURCE C		17373	1000 1000 1000 1000 1000 1000 1000 100		"
45.0 CEN OFF CLASS 2 TUBES ALL LANES		116543	TOTALS 1177 723 572 572 6164 6164 6164 6779 6996 7102 7505 7505 7505 7505 7505 7505 7505 75		STATION 582

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 % % % TOTAL HOURS

11 24

	PERCENT	TOTAL VEHICLE	06-07AM 08-09AM 08-09AM 09-10AM 11-12AM 11-12AM 01-02PM 02-03PM 03-04PM 06-07PM 06-07PM 07-08PM 07-08PM 07-10PM 11-12PM 11-12PM 11-12PM 01-02AM 02-03AM 03-04AM	TIME	
% HEAVY TRU AXLES / TRU % TRAILER T % TRAILERS PEAK HOUR B	OF TOTAL	HICLES	77777777777777777777777777777777777777	DATE	
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20.6 18.6 29.6	0.2	31	0000000	7 OR MORE AXLE	LER
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		16706	973 907 907 908 907 908 908 908 908 908 908 908 908 908 908	TOTAL VEHICLES	

68

COUNTY: MARSHALL
DISTRICT: 1
MILEPOST: 45.000
ROAD DESIGNATION: R
LATEST ADT COUNT: 16149 IN 2010
LOCATION INFORMATION: BTWN KY 348 AND US

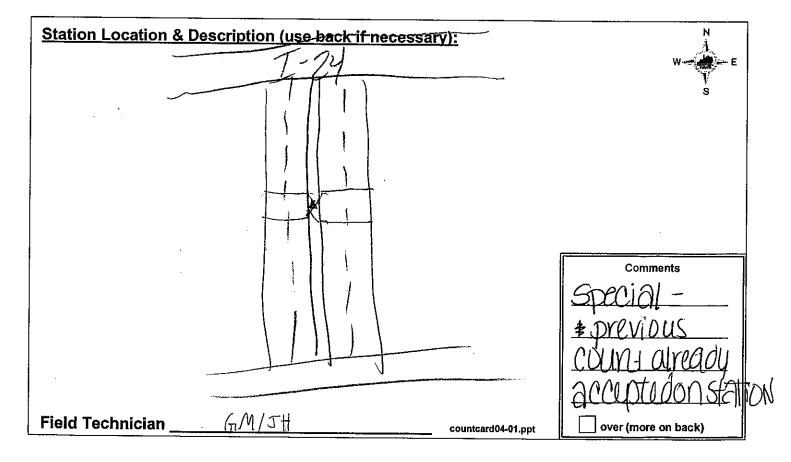
STATION: 582 BOTH N-S ROUTE: KY9003
TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District # Add Delete Combine I.D.# Must be accompanied by reason R Route Code i=interstate K=Ky or Parkw R=County Rd F=FS Milepoint **Estimate** U=U.S. S=City St Array - T = Tube H = 2 tubes L = Loop B = 2 loops M = mixed R = Radar P = Plezo W = WIM Plezo O = Other Route Number Suffix _= none Special Index STA# **ESTIMATE County Number** # of houses Lanes at Machine # of All Lanes Counter is counting Station Type Record V=Volume Machines at A=All Lanes B=Part NB & SB # of Business __ x 25 = E=all EB S=all SB C=Cls x Spd C=Part EB & WB Counter is counting 1≓ane 1 V=Vehicles W=WIM A=Axies Total Station number 7 5 6 Machine # 14772~0012 File # ____ County name Marshall City name PKWY Route Road/Street name Longitude W <u>B</u>B.3 Time Month Year Day Volume Class Type Record (check one) Length Recorder Set Out | 07 2010 Recorder Picked Up 13:51 28 20,00 Type Sensor (check one) _Loop l _Radar L 07



* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 % OTHER 2 AXLE 4 TIRE VEHICLES 00 % 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

-1 - 2 % % > %	PERCENT	TOTAL VEHICLE	06-07AM 08-09AM 08-09AM 09-10AM 11-12AM 11-12AM 12-012AM 02-03PAM 03-04AM 06-07PAM 06-07PAM 07-08AM 07-08AM 07-08AM 08-04AM 08-04AM	TIME PERIOD	
% HEAVY TRUCKS AXLES / TRUCK X TRAILER TRUCKS X TRAILERS APK I PEAK HOUR BETWEE TOTAL HOURS	OF TOTAL	HICLES	77777777777777777777777777777777777777	DATE	
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מל ⁹ כל ⁹ כל	 	172	W	3 AXLE	TRUCKS
SING SING	0.8	131	- - 	4 OR MORE AXLE	XS
AXLE CORRECTION SINGLE UNIT SINGLE UNIT THE TRUCKS AT PE	1.4	226	85070703007082507000000	4 OR LESS AXLE	SINGLE
TRUCKS A PK HR	8.7	1394	43222234445667788888887844445687887878787878787878787878787878787878		LE TRAIL
, n n n n	1.0	163	๑๐๓๑๙๙๐๛๛๛๛๛๛ ๑๐๓๑๙๙๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛	6 OR MORE AXLE	ER
0.84 521/10 31/ 134/	0.2	33	NNNNNNNNNN0	5 OR LESS AXLE	MULT
6113 1236 1236	0.0	œ	-00-000-00-000-000	6 AXLE	TRUCKS
3.2 10.85 8	0.4	68	844440V0V0C00000C	7 OR MORE AXLE	LER
36 36 3 6	0.0	0,	000000000000000000000000000000000000000	OTHERS	
		16113	389 389 31 31 31 31 31 31 31 31 31 31 31 31 31	TOTAL VEHICLES	

COUNTY: MARSHALL 2010
DISTRICT: 1
MILEPOST: 50.200
ROAD DESIGNATION: R
LATEST ADT COUNT: 15584 IN 2010
LOCATION INFORMATION: BTWN US 68 AND I-24

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT

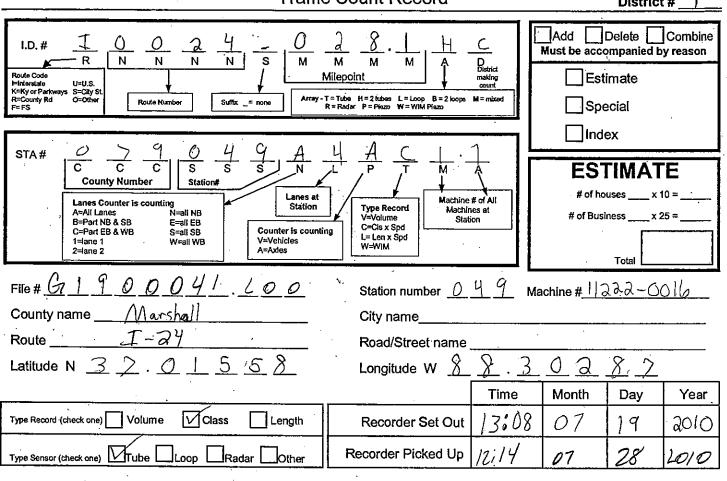
Y: MARSHALL

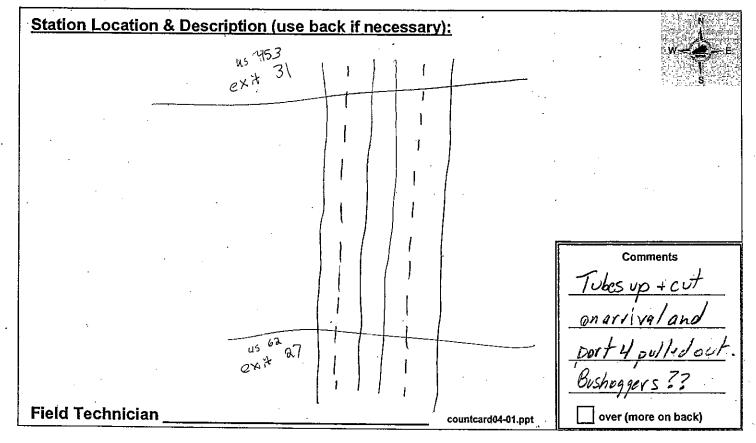
2010

STATION: STATION: 756 BOTH N-S
ROUTE: KY9003
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Traffic Count Record

District #





ROUTE: 10024

WEEK OF JULY

20 TO JULY

26 2010

PORTABLE TRAFFIC RECORDER REPORT

MARSHALL COUNTY

STATION 049

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOU		TOTALS:	DATE: DAY: 12-1 1-2 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4
FACTOR: 93 TOR: 100 URS: 168 HOUR: 2024 HOUR: 2469		25624	75000000000000000000000000000000000000
93 00 24 BETWEEN 59 BETWEEN	AVERAGE	26587	56888888888888888888888888888888888888
10~11 5~ 6		29300	111416 1223 1235 1235 1235 1235 1235 1235 1235
AM ON S	DAILY TRAFFIC:	32343	703829
SATURDAY FRIDAY	IC: 23589	28210	5706 5706 5706 5706 5706 5706 5706 5706
MILE POI COUNTED DATA SOU ARRAY LANES CO	99	19148	400 N S C C C C C C C C C C C C C C C C C C
POINT ED BY SOURCE		16348	MON 312 312 312 312 312 312 313 313 313 313
28.1 CEN OFF CLASS 2 TUBES ALL LANES		177560	TOTAL 2867 2298 1923 1905 2417 3702 5764 77554 77554 11236 11236 11234 11234 11234 11472 10925 1

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

AXL AXL PE T TOT	PERCENT	TOTAL VEHICLE	07-08PM 07-08PM 07-08PM 07-02PM 07-02PM 07-02PM 07-08PM 08-07PM 08-07PM 09-10PM 11-12PM 01-01PM 01-01PM 01-02PM 01-02PM 01-03PM 01-02PM	1 20 14	<u> </u> .
202200	OF TOTAL	HICLES	777777777777777777777777777777777777777	DATE	
TVEEN	6.5	1491	5322238464386778666673588888888	MOTOR- CYCLES	
= 5676/22821 = 27651/ 5676 = 4980/22821 = 289/ 1592 = 03-04PM 159	59.8	13650	247 2020 2020 2020 2020 2020 2020 2020 2	9	
= 24.9 = 4.8 = 21.8 = 18.2 >2 VEHI	8.5	1940	£2525255555555555555555555555555555555	OTHER 2 AXLE 4 TIRE VEHICLES	
CE ** ** 22 **	0.7	161	0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	BUSSES	NIS
	0.9	206	๛๛ผผผพ๛ธธธ _ร าวไม่สี่มีสี่มีสี่มีว่าใช้	2 AXLE 6 TIRE	SINGLE UNIT
%%% >	1.0	233		3 AXLE	TRUCKS
AXLE CORRECT NO. SINGLE UN. SINGLE UN. RINGLE UN. RINGLE UN. RINGLE AT.	0.4	96	000000พผพพลงพูลาปปดบพระ	4 OR MORE AXLE	KS
DRRECTI E UNIT	2.0	460	12992222222222222222222222222222222222	4 OR LESS AXLE	SINGLE
ON FACTO TRUCKS D PK HR EAK HOUR	14.5	3320	198890000000000000000000000000000000000	5 AXLE	TRUCKS
X	2.9	653	00404040404040404040404040404040404040	6 OR MORE AXLE	LER
0.74 696/2: 48/ 337/	0.6	145	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	5 OR LESS	ME F
22821 1592 1592	0.3	63	บยถ44の48ยยอดดดดอดดอดดอด	6 AXLE	TRUCKS
	1.5	339	72623333333335555888888888	7 OR MORE AXLE	ILER
	0.3	64*	-\N\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	OTHERS	
		22821	1063 1063 1063 1063 1063 1063 1063 1063	TOTAL	

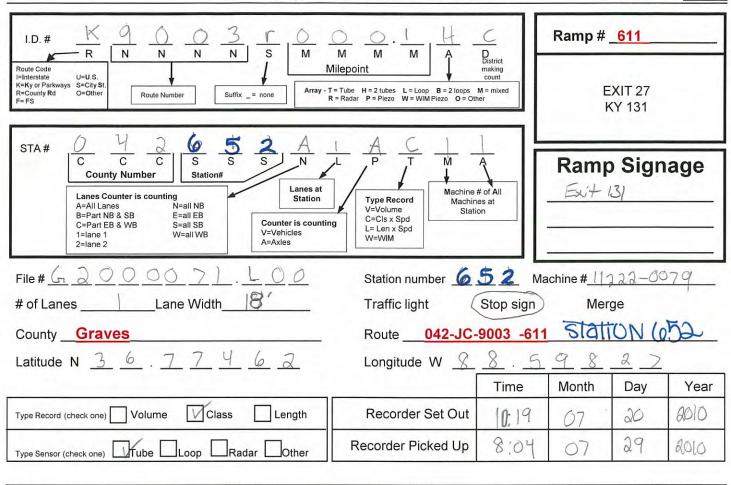
COUNTY: MARSHALL STATION DISTRICT: 1 ROUTE: MILEPOST: 28.100 ROAD DESIGNATION: R LATEST ADT COUNT: 23589 IN 2010 LOCATION INFORMATION: BTWN US 62 AND LIVINGSTON COUNTY LINE

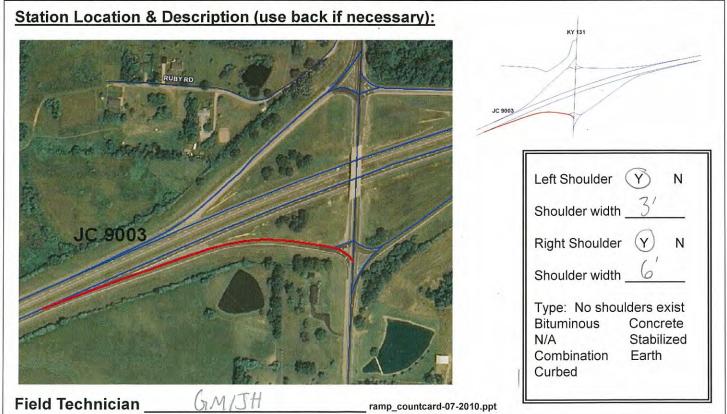
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

Y: MARSHALL 2010 STATION: STATION: 049 BOTH E-W ROUTE: I 24
TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 1

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTOR AXLE FACTOR HOURS AM HIGH HOURD PM HIGH HOU		TOTALS:	6- 7 PM 7- 8 PM 8- 9 PM 10-11 PM	DATE: 12-1-12-13-43-43-43-43-43-43-43-43-43-43-43-43-43	ROUTE: KY9
FACTOR: 97 TOR: 100 JURS: 168 HOUR: 76		1144	244 244 264 264	VED 11.5.1.1.2.1.2.1.2.1.2.1.2.1.2.1.2.1.2.1	KY9003R
8 8 H H	AVERAGE	1088	156493 100 100 100 100 100 100 100 100 100 10		
0.0	0	1155	3475 440 400 400 400	FRI 23 56 65 56 65 65 65 65 65 65 65 65 65 65	, P
20	-1	783	235447	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0
UNDAY URSDA	••	809	187 187 187	8 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	NTY
MILE P COUNTE DATA S ARRAY LANES		991	253 253 253	MON	27 2010
POINT TED BY SOURCE Y COUNTED	j	1112	225045	TUE 7 11 5 5 6 8 6 8 6 8 6 8 6 8 6 8 6 8 6 8 6 8	
: 0.1 : CEN OFF : CLASS : 2 TUBES : ALL LANES		7082	507 347 252 153	TOTALS 67 43 285 137 285 381 381 383 383 383 383 775	STATION 652

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 28%

*

2	PERCENT OF TOTAL	TOTAL VEHICLES	07-0AM 7/20 08-09AM 7/20 09-10AM 7/20 09-10AM 7/20 10-11AM 7/20 11-12AM 7/20 01-02PM 7/20 01-02PM 7/20 03-04PM 7/20 05-06PM 7/20 06-07PM 7/20 06-07PM 7/20 07-08PM 7/20 07-08PM 7/20 07-08PM 7/20 01-11PM 7/20 01-12PM 7/20 01-20AM 7/20 01-02AM 7/20 03-04AM 7/20 05-06AM 7/20 05-06AM 7/20	Ö.	
HEAVY TRUCKS LES / TRUCK TRAILER TRUC TRAILERS DPK	TOTAL	LES	777777777200110	m	
EE S	0.8	œ	000000000000000000000000000000000000000	MOTOR- CYCLES	
= 119/ 10 = 351/ 1 = 40/ 10 = 3/ 1	49.7	504	0-10740100000101010000010101000001010000001010	PASSENGER CARS	
1015 = 11.7 % 119 = 2.950 1015 = 3.9 % 114 = 2.6 % 114 VEHICLES	37.8	384		ESEE	
CLES	0.1	_	000000000000000000000000000000000000000	BUSSES 6	SING
	6.9	70	04DDBD444D080B00CCCOCC	AXLE	SINGLE UNIT TRUCKS
%%%A	0.8	œ	000000000000000000000000000000000000000	3 AXLE M	TRUCK
AXLE CORREC % SINGLE UN % SINGLE UN % TRUCKS AT	0.0	0	000000000000000000000000000000000000000		S
	1.5	15	0000000000	4 OR LESS AXLE	SINGLE
ION FACTOR T TRUCKS T a PK HR PEAK HOUR	2.5	25	-000000N-NNNNNN		RUCKS
R	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
.95 79/ 1 12/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULT
015 114 =	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
7.8 7.9 10.5	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		1015	13248669123335555555555555555555555555555555555	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

ROUTE: KYS

MILEPOST: 0.100

ROAD DESIGNATION: R

LATEST ADT COUNT: 980 IN 2010

LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB ONTO KY 131

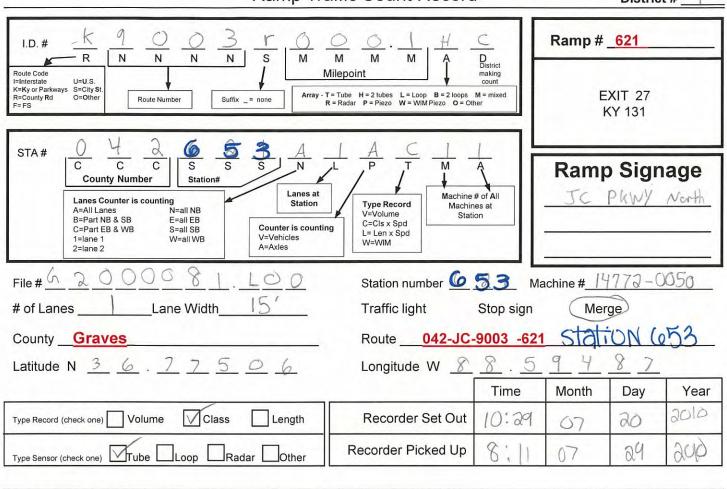
STATION: 652 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

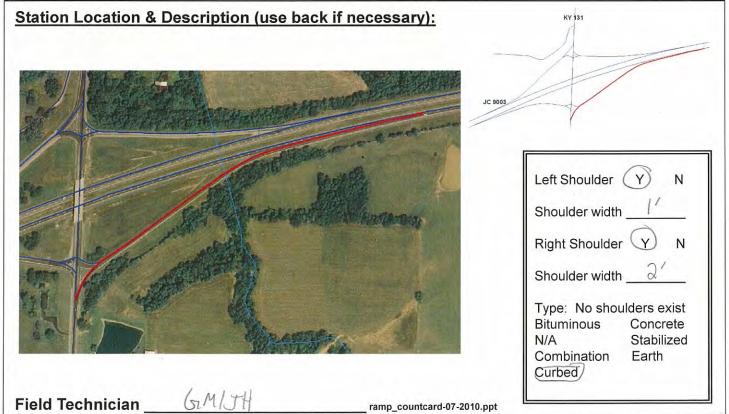
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

7: GRAVES 2010 STATION:

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAY: 12-1 1AM 12-1 2 AM 3-4 AM 5-6 AM 6-7-8 AM 7-8 AM 11-12		ROUTE: KY9003R	
10R: 97 : 100 : 168 : 168 : 18: 50		474	L C C C C C C C C C C C C C C C C C C C)03R	
77 70 79 8ETWEEN 79 BETWEEN	AVERAGE	497	THU THU 100 200 200 200 200 200 200 200 200 200	WEEK OF		*POR
5- 7 A		562	FRI 3000 22 4 4 20 0 0 2 2 2 2 2 2 2 2 2 2 2	F JULY	G.	*PORTABLE IN
AM ON T	DAILY TRAFFIC	421	AT 4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	21 10	GRAVES CO	TRAFFIC R
THURSDAY	C: 452	332	SEN	O JULY	COUNTY	RECORDER
MILE P COUNTE DATA S ARRAY LANES	10	465	000 000 000 000 000 000 000 000 000 00	27 2010		REPORT
OINT D BY OURCE		518	L L C C C C C C C C C C C C C C C C C C			
: 0.1 : CEN OFF : CLASS : 2 TUBES		14	7		STATION 653	
		3269	TOTALS 125 125 254 2554 2574 1298 1298 1298 1298 1298 1298 1298 1298		~	

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 28%

*

TOTAL HOURS

= 24

% HEAVY TRUCKS = 34/ AXLES / TRUCK = 125/ % TRAILER TRUCKS = 20/ % TRAILERS @PK HR = 2/ PEAK HOUR BETWEEN 06-07AM	PERCENT OF TOTAL	TOTAL VEHICLES	07-08AM 08-09AM 09-10AM 10-11AM 11-12AM 11-12AM 01-02PM 03-04PM 05-06PM 06-07PM 06-07PM 07-08PM 09-10PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-13AM 11-	RIV	
	OF TOTAL	HICLES	77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10 77/200/10	DATE	
	0.4	2	000000000000000000000000000000000000000	MOTOR- CYCLES	
	71.3	326	27-1-125622772222772222	PASSENGER CARS	
457 = 7.4 34 = 3.6 457 = 4.4 38 = 5.3 38 VEHI	20.8	95	- - - - - - - - - - - - - - - - - - -	OTHER 2 AXLE 4 TIRE VEHICLES	
7.4 % 3.676 4.4 % 5.3 % VEHICLES	0.0	0	000000000000000000000000000000000000000	BUSSES 6	SING
	1.5	7	-0-0000000000000-0	AXLE TIRE	LE UNI
AXLE CORRECTION % SINGLE UNIT TR % SINGLE UNIT a % TRUCKS AT PEAK	1.5	7	00000000000000	3 AXLE	SINGLE UNIT TRUCKS
	0.0	0		4 OR MORE AXLE	
RRECTI E UNIT E UNIT S AT P	2.2	10	000000000000	4 OR LESS AXLE	SINGLE
ON FACTOR TRUCKS a PK HR EAK HOUR	2.2	10	0-00000000000		LE TRAILER
יה או היה היה היה היה היה היה היה היה היה הי	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
2/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
457 38 38	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
= 3.1 5.3	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		457	71023322222332223322233223323323332333233	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 0.100

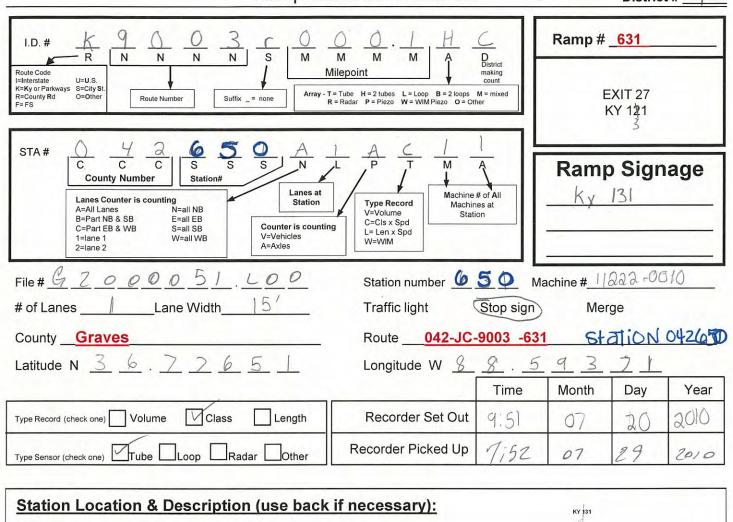
ROAD DESIGNATION: R
LATEST ADT COUNT: 452 IN 2010

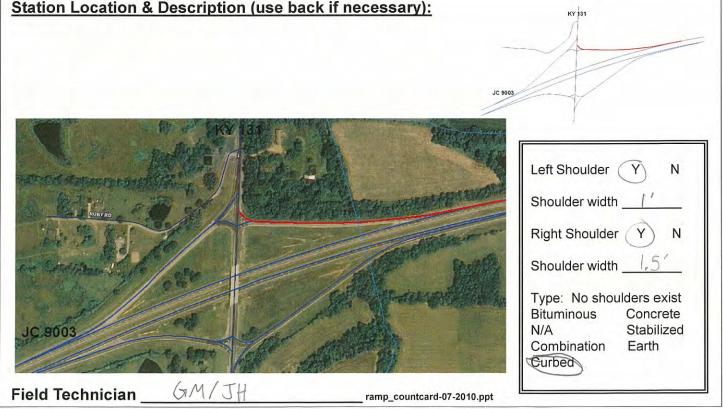
LOCATION INFORMATION: RAMP FROM KY 131 ONTO PURCHASE PARKWAY NB STATION: 653 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
2010 STATION:

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACT AXLE FACTOR TOTAL HOURS AM HIGH HOUF PM HIGH HOUF		TOTALS:	TOTALS:	DATE: DAYE: 12-1-1 AM 3-4 AM 4-4 AM 4					
		460	802522222222222222222222222222222222222		03R				
0 BETWEEN	AVERA	AVERA	AVERA	AVERA	478	T 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	WEEK OF		* POK
58- 69- 9A	SE DAILY	550	FRI 33 17 17 17 17 17 17 17 17 17 17 17 17 17	F JULY	GR	PORTABLE IN			
22	TRAFFI	415	22222222222222222222222222222222222222	21 1		IKAFFIC RECORDER			
UESDAY HURSDAY		363	SUN	0 JULY	YTNU				
MILE P COUNTE DATA S ARRAY LANES		451	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	27 2010		KET CK I			
OINT D BY OURCE		471	TUE 1000 1000 1000 1000 1000 1000 1000 10						
CEN OFF CLASS 2 TUBES		318	T07		STATION 650				
	THLY FACTOR: 97 E FACTOR : 100 COUNTED BY AL HOURS : 168 HIGH HOUR: 40 BETWEEN 8- 9 AM ON TUESDAY ARRAY HIGH HOUR: 61 BETWEEN 5- 6 PM ON THURSDAY LANES COUNTED	AVERAGE DAILY TRAFFIC: 441 THLY FACTOR: 97 E FACTOR: 100 AL HOURS: 168 HIGH HOUR: 40 BETWEEN 8- 9 AM ON TUESDAY ARRAY HIGH HOUR: 61 BETWEEN 5- 6 PM ON THURSDAY LANES COUNTED	ALS: 460 478 550 415 363 451 471 AVERAGE DAILY TRAFFIC: 441 THLY FACTOR: 97 E FACTOR: 100 AL HOURS: 168 HIGH HOUR: 40 BETWEEN 8- 9 AM ON THURSDAY ARRAY HIGH HOUR: 61 BETWEEN 5- 6 PM ON THURSDAY LANES COUNTED	E: 21 22 23 24 25 26 27 10 MON TUE 1 AM ED THU FRI SAT SUN MON TUE 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	E: 21 22 23 24 25 26 27 10 IUE AND THE AND	TE: KY9003R WEEK OF JULY 27 2010 E: 21 22 23 24 25 26 27 I AM 5 7 10 31 50			

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

1P%% 2%	PERCENT	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 01-02PM 01-02PM 02-03PM 03-04PM 06-07PM 07-08PM 08-09PM 09-10PM 11-12P	RIV	
% HEAVY TRU AXLES / TRU X TRAILER T X TRAILERS PEAK HOUR B TOTAL HOURS	PERCENT OF TOTAL	HICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	DATE	
AVY TRUCKS = S / TRUCK = AILER TRUCKS = AILERS aPK HR= HOUR BETWEEN 05-1 L HOURS = 24	0.0	0	000000000000000000000000000000000000000	MOTOR- CYCLES	
34/ 128/ 20/ 1/ 06PM	71.1	312	7 6 6 5 5 6 5 6 6 6 6 6 6 6 6 6 6 6 6 6	PASSENGER	
439 = 7.7 34 = 3.7 439 = 4.6 47 = 2.7 47 VEHI	21.2	93	りょりょうしょうとしょうしょうしょうしょうしょうしょうしょうしょうしょうしょうしょうしょうしょうし	OTHER 2 AXLE 4 TIRE VEHICLES	
7.7 % 3.765 4.6 % 2.1 % VEHICLES	0.0	0	000000000000000000000000000000000000000	BUS	
	2.3	10	000000000000000000000000000000000000000	AXLE	SINGLE UNIT TRUCKS
%%%A *	0.2	1	00000000000000000	т	T TRUCK
SINGLE SINGLE SINGLE TRUCKS	0.7	3	000000000000000000000000000000000000000	4 OR 4 MORE L	S
AXLE CORRECTION % SINGLE UNIT TR % SINGLE UNIT @ % TRUCKS AT PEAK	1.6	7	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
TRUCKS a PK HR BEAK HOUR	3.0	13	00000000000000000000000000000000000000		RUCKS
RR 98	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
2/1/4	0.0	0	000000000000000000000000000000000000000		MULT
439 = 47 =	0.0	0	000000000000000000000000000000000000000		I-TRAILER TRUCKS
423	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		439	5-1-1-466222246822212221222	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

ROUTE: KY9

MILEPOST: 0.100

ROAD DESIGNATION: R

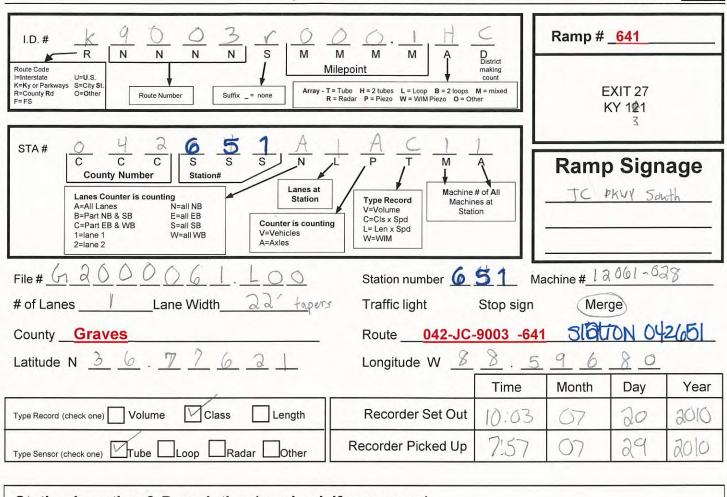
LATEST ADT COUNT: 441 IN 2010

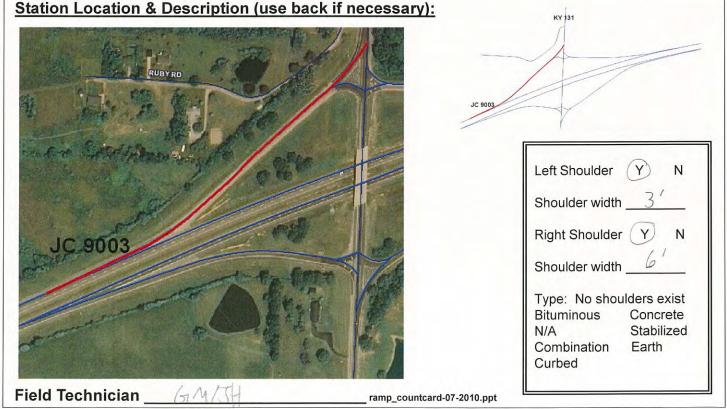
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB ONTO KY 131

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**
2010

STATION: 6 STATION: 650 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

District #





MONTH AXLE TOTAL AM HI		TOTALS	DATE: DATE: 12-7-88-9-10-11-12-11-11	ROUTE	
HLY FACT FACTOR L HOURS IGH HOUR		S	A C C C C C C C C C C C C C C C C C C C	E: KY9003R	
~~ S		1170	WED 21 115 2 6 8 6 8 6 7 7 6 8 8 6 9 7 7 8 8 6 9 7 7 8 8 6 9 7 7 8 6 9 8 6 9 7 8 6 9	03R	
97 100 168 115 BETWEEN 88 BETWEEN	AVERAGE	1103	T 22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	WEEK	, 70
V 7- 8 AM	AGE DAILY	1182	FRI 1050 1050 1050 1050 1050 1050 1050 105	GR. OF JULY	*PORTABLE IR
22	TRAFFIC:	839	S A T T A T T A T T A T T A T T A T A T	GRAVES COUNTY Y 21 TO J	RAFFIC REC
JEDNESDAY TUESDAY	1013	807	12224 12224	חרג ווא	RECORDER R
MILE PO COUNTED DATA SO ARRAY LANES		1071	MON MON 113 13 553 13 5	27 2010	REPORT
OURCE		1149	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
0.1 CEN OFF CLASS 2 TUBES ALL LANE			4	STATION 65	
Ø		7321	TOTALS 43 38 82 53 38 82 299 409 409 402 298 417 412 440 458 410 496 468 410 496 468 410 496 410 410 410 410 410 410 410 410 410 410	_	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 % OTHER 2 AXLE 4 TIRE VEHICLES 00 % 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

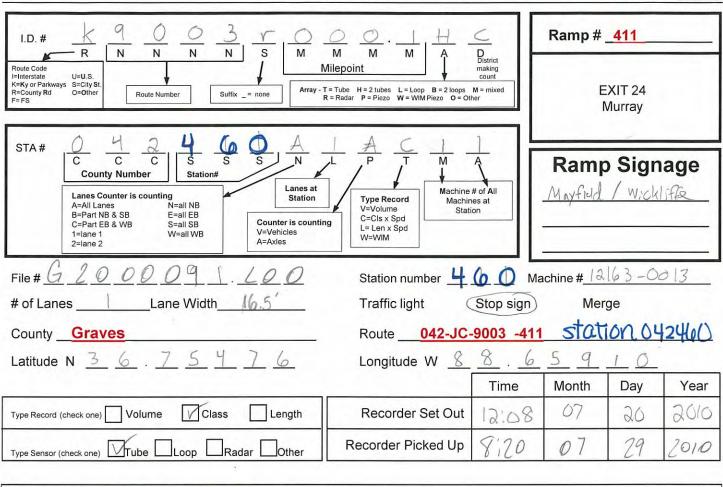
TP%%A%	PERCENT	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-11AM 11-12AM 01-02PM 02-03PM 03-04PM 04-05PM 08-06PM 08-06PM 08-06PM 08-06PM 08-07PM 08-07PM 08-07PM 08-07PM 08-07PM 08-07PM 08-07PM 08-08P	TIME	
% HEAVY TRUCKS = AXLES / TRUCK = TRAILER TRUCKS = TRAILERS APK HR= PEAK HOUR BETWEEN 07- TOTAL HOURS = 24	PERCENT OF TOTAL	EHICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	DATE	
TRUCKS = TRUCKS = R TRUCKS = RS DPK HR= R BETWEEN URS =	0.2	2	000000000000000000000000000000000000000	MOTOR- CYCLES	
205/ 537/ 45/ 08AM	27.4	285	たってはいいないでしないのところとしなるない	PASSENGER CARS	
1039 = 19.1 205 = 2.0 1039 = 4.3 86 = 2.3 86 VEHI	52.6	547	₩₹₩₩₩₩₩₩₩₩₩ ₩₩₩₩₩₩₩₩₩₩₩ ₩₩₩₩₩₩₩₩₩₩₩₩₩₩	OTHER 2 AXLE 4 TIRE VEHICLES	
19.7 % 2.620 4.3 % 2.3 % VEHICLES	0.3	w	00000000000000-00000	BUSSES	SIN
	14.2	148	00000000000000000000000000000000000000	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
%%% >	0.9	9	000000000000000000000000000000000000000	3 AXLE	T TRUC
AXLE CO % SINGL % TRUCK	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
XLE CORRECTION SINGLE UNIT TR SINGLE UNIT @ TRUCKS AT PEAK	2.0	21	00000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
ION FACTOR T TRUCKS T @ PK HR PEAK HOUR	1.9	20	00000000		TRUCKS
	0.4	4	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.94 160/ 18/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MCF
1039 86 86	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
= 15.4 = 18.6 = 20.9	0.0	0	000000000000000000000000000000000000000	7 O MOR AXL	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		1039	123435655556565656565656565656565656565656	TOTAL VEHICLES	

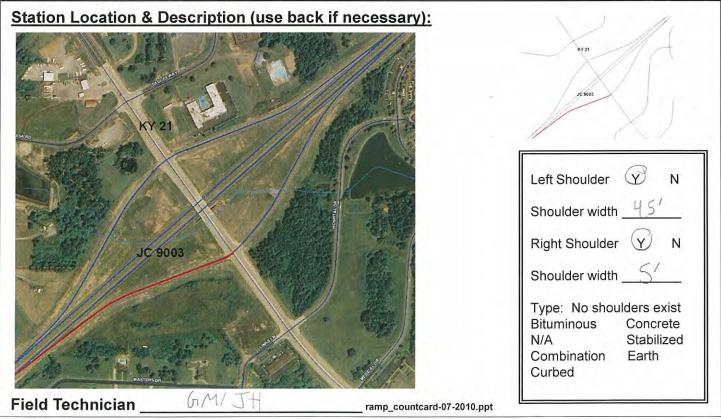
COUNTY: GRAVES DISTRICT: 1 MILEPOST: 0.100 ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2 LATEST ADT COUNT: 1013 IN 2010 LOCATION INFORMATION: RAMP FROM KY 131 ONTO PURCHASE PARKWAY SB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

TY: GRAVES 2010 STATION:

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY	ROUTE: KY9003R	
CTOR: 97 R : 100 S : 168 UR: 225 UR: 149		1596	WED 21 12 5 5 1 1 7 7 1 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2	003R	
BETWEEN	AVERAGE	1773	12224441111111111111111111111111111111	WEEK OF	
7- 8 AM 4- 5 PM		1704	FRI 3 12 6 3 12 6 3 12 6 3 12 6 12 6 12 6 1	JUL	TABLE TWO
0 Q	DAILY TRAFFIC:	994	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	GRAVES COUNTY Y 21 TO J	110 700
MONDAY	1479	928	11224447567166776777777777777777777777777777	חרג זורא	Chorn N
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT		1997	MON C C C C C C C C C C C C C C C C C C C	27 2010	07.1
OINT D BY OURCE COUNTED:		1685	TOTAL	s	
0.1 CEN OFF CLASS 2 TUBES ALL LANES		10677	TOTALS 59 48 64 68 86 275 628 1004 717 695 683 683 764 655 763 763 678 717 717 717 717 717 717 717	STATION 460	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 28%

% HEA AXLES % TRA PEAK TOTAL	PERCENT	TOTAL VEHICLES	07-08AM 08-09AM 08-09AM 09-10AM 11-12AM 12-012AM 01-02PM 02-03PM 03-04PM 06-07PM 06-07PM 07-08PM 09-10PM 10-11PM 11-12PM 11-12PM 01-02AM 03-04AM 03-04AM		
ILER ILER ILER ILER	OF TOTAL	HICLES	7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10	DATE	
RUCKS	0.6	9	~00000000~~~~0~0~000~~0	MOTOR- CYCLES	
142/ 548/ 90/ 6/ 07-08AM	67.8	1040	2736461719 2736461719 2736461719	PASSENGER	
1533 = 9.3 142 = 3.8 1533 = 5.9 152 = 3.9 152 = 3.9	22.3	342	141113333333333333333333333333333333333	<	
9.3 3.859 5.9 % 3.9 % VEHICLES	0.6	9	000000000000	BUSSES	NIS
	1.9	29	- MUN4MUMMH-0000000000000000000000000000000000	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
	0.9	14	0000000	3 AXLE	T TRU
AXLE CORF % SINGLE % SINGLE % TRUCKS	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	CKS
AUNEC	1.3	20	-00000000-0NN-WN	4 OR LESS AXLE	SINGLE
TION FACTOR	4.6	70	4404DD044D4BUNNNC	5 AXLE	
	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	TRAILER
0.92 52/ 5/ 11/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	WUL
1533 152 152	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
7.34	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	SILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		1533	134 8 8 8 12 12 24 4 4 7 8 9 8 9 8 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	TOTAL	

COUNTY: GRAVES

DISTRICT: 1

ROUTE: KY
MILEPOST: 0.100

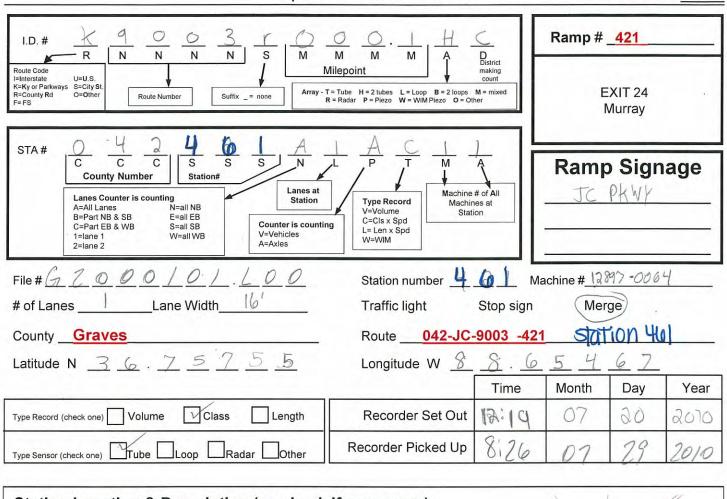
ROAD DESIGNATION: U

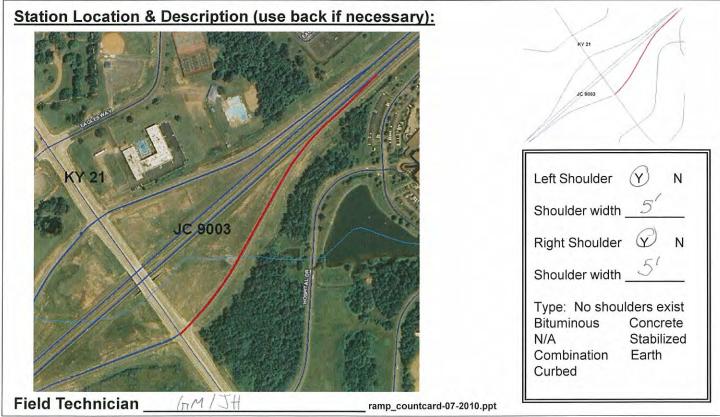
LATEST ADT COUNT: 1479 IN 2010

LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB ONTO KY 21 KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
2010

**ATTION: ' STATION: 460 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: KY9003R	
		981	00000000000000000000000000000000000000		3R	
97 100 168 81 BETWEEN 98 BETWEEN	AVERAC	976	T T T T T T T T T T T T T T T T T T T	WEEK OF		
4.7 58	E DAIL	1097	722442888873377446246132358887337746464667676777777777777777777777	YJULY	6	
PM ON	AVERAGE DAILY TRAFFIC:	671	718288888889 7188888888889 718888888888889 718888888888	21	GRAVES COUNTY	. 201 (000
TUESDAY	IC: 872	513	1221233333432711 122123333334377417232158	TO JULY	YTNUC	
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT	Ν	1021	MOZ 2017 2017 2017 2017 2017 2017 2017 2017	27 2010		
E .		1038	TUE 7 10 5 10 10 10 10 10 10 10 10 10 10 10 10 10			
CEN OFF		6297	TOTALS 24 25 27 27 27 27 27 27 27 27 27 27 27 27 27		STATION 461	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

⊣₽%% ₽%	PERCENT	TOTAL VEHICLES	06-07AM 08-09AM 09-10AM 11-12AM 11-12AM 12-01PAM 01-02PM 02-03PM 04-05PM 06-07PM 06-07PM 07-08PM 07-08PM 10-11PM 11-12PM 11-12PM 11-12PM 07-03AM 07-05AM 07-05AM	RIN	
% HEAVY TRU AXLES / TRU X TRAILER T X TRAILERS PEAK HOUR B TOTAL HOURS	PERCENT OF TOTAL	HICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10	DATE	
RUCKS BPK HR ETWEEN	0.8	7	000000000000000000000000000000000000000	MOTOR- CYCLES	
66/ = 260/ = 44/ = 2/ 04-05PM = 24	71.8	653	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	PASSENGER	
909 = 7 66 = 3 909 = 4 80 = 2 80 VEH	20.1	183	o E C E E E E E E E E E E E E E E E E E	OTH 2 A 4 T	
7.3 % 3.939 4.8 % 2.5 % VEHICLES	0.1	<u>ن</u>	000000000000000000000000000000000000000	ES	SIN
	1.2	11	00000000000	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
	1.1	10	00000000000	3 AXLE	T TRU
AXLE C % SING % TRUC	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	ICKS
XLE CORRECTION SINGLE UNIT TR SINGLE UNIT @ TRUCKS AT PEAK	1.5	14	00000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
TION FACTOR IT TRUCKS IT @ PK HR PEAK HOUR	3.3	30	N-00000NNNNW-NN-NN	5 AXLE	
S S S	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	TRAILER
0.93 22/ 2/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MCF
909 80 80	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCK
522	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	TRAILER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		909	47477000700000000000000000000000000000	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 0.100

ROAD DESIGNATION: U

LATEST ADT COUNT: 872 IN 2010

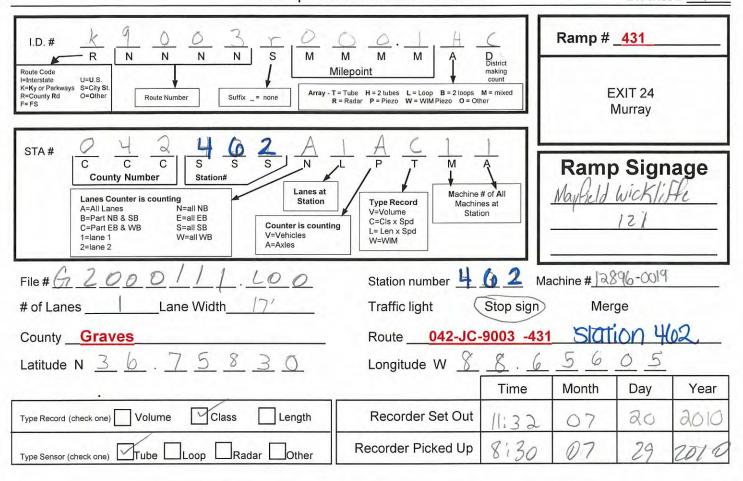
LOCATION INFORMATION: RAMP FROM KY 21 ONTO PURCHASE PARKWAY NB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

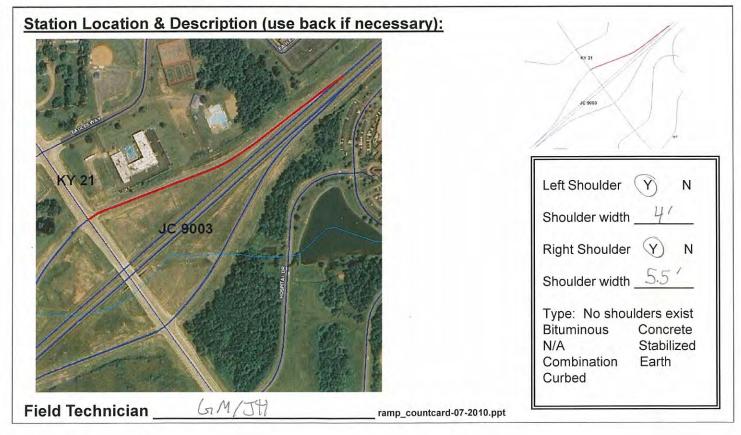
Y: GRAVES

2010

TATION: 4 STATION: 461 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

District #





THLY E FAC AL HO HIGH		TOTALS:	DATE:: DAY::: DAY::: 12:-1-2 1 3-4 AM 3-4 AM 7-5 AM 7-6 AM 7-7 AM		ROUTE: KY90	
~~		1001	TED 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		103R	
97 00 58 BETWEEN	AVERA	1016	T 22 T 22 7 27 7 27 7 27 7 27 7 27 7 27	WEEK O		707
7-8A		1080	1742210188458426524265211823	F JULY	GR	LOW I WOLE IN
22	TRAFF	656	SAT 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	21		INAFFIC
MONDAY		617	322 1380 1380 1380 1380 1380 1380 1380 1380	LO JULY	YTNUC	KECOKDEK
Omo	3	1155	000 000 000 000 000 000 000 000 000 00	27 2010		KEPOK!
OINT D BY OURCE		1069	TUE 77 1024 1024 1024 1024 1024 1024 1024 1024	i		
CEN CLAS		659	TOTA 5-1 5-1 6-1 1-6 1-6 1-7 1-7 1-7 1-7 1-7 1-7 1-7 1-7 1-7 1-7		STATION 462	
	THLY FACTOR: 97 E FACTOR : 100 COUNTED BY : CEN OFF AL HOURS : 168 HIGH HOUR: 122 BETWEEN 7- 8 AM ON MONDAY ARRAY : 2 TUBES HIGH HOUR: 96 BETWEEN 4- 5 PM ON TUESDAY LANES COUNTED: ALL LANE	AVERAGE DAILY TRAFFIC: 913 LY FACTOR: 97 FACTOR: 100 FACTOR: 100 HOURS: 168 GH HOUR: 122 BETWEEN 7- 8 AM ON MONDAY ARRAY GH HOUR: 96 BETWEEN 4- 5 PM ON TUESDAY LANES COUNTED: ALL LANE	ALS: 1001 1016 1080 656 617 1155 1069 AVERAGE DAILY TRAFFIC: 913 THLY FACTOR: 97 E FACTOR: 100 COUNTED BY : CEN OFF AL HOURS: 168 HIGH HOUR: 122 BETWEEN 7- 8 AM ON MONDAY ARRAY HIGH HOUR: 96 BETWEEN 4- 5 PM ON TUESDAY LANES COUNTED: ALL LANE	E: 21 22 23 24 25 26 27 10 1 AM	E: 21 22 23 24 25 26 27 27 10 ANN 9 7 11 6 4 4 4 5 2 6 2 7 1 1	TE: KY9003R WEEK OF JULY 21 TO JULY 27 2010 E: 21 22 23 24 25 26 27 I AM 9 7 11 7 12 3 1 6 4 4 5 2 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

⊤₽%% Α%	PERCENT OF	TOTAL VEHICLES	07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 11-12PM 01-02PM 02-03PM 03-04PM 06-07PM 07-08PM 08-06PM 09-10PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 03-04AM 03-04AM	TIME	
% HEAVY TRUCKS = 62/ AXLES / TRUCK = 268/ % TRAILER TRUCKS = 48/ % TRAILERS @PK HR = 3/ PEAK HOUR BETWEEN 07-08AM TOTAL HOURS = 24	OF TOTAL	HICLES	777200110 77200110 77200110 77200110 77200110 77200110 77200110 77200110 77200110	4	
UCKS = UCK = TRUCKS = DPK HR= BETWEEN =	0.6	6	-00000000-0000000	ES.	
	78.4	744	184444488886 18648864886488648648	PASSENGER	
949 = 6.5 62 = 4.3 949 = 5.1 80 = 3.7 80 VEHI	14.4	137	บบ_บผมระตอบีบัง บ _ั บัตอง จอง จ	OTHER 2 AXLE 4 TIRE VEHICLES	11
.5 % .323 .1 % .7 % HICLES	0.0	0	000000000000000000000000000000000000000	BUSSES 6	SING
	0.9	9	00000000000	5 TIRE	SINGLE UNIT TRUCKS
W W W P	0.0	0	000000000000000000000000000000000000000	3 AXLE	T TRU
AXLE CORR % SINGLE % SINGLE % TRUCKS	0.5	vi	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
CORRECTION OF THE CORRECTION OF THE CONTROL OF THE CORRECTION OF T	1.1	10	-000000000000	4 OR LESS AXLE	SINGLE
ION FACTOR TRUCKS DEAK HOUR	4.0	38	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		TRUCKS
	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.93 14/ 0/ 3/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
949 80 80	0.0	0	000000000000000000000000000000000000000	6 AXLE	TI-TRAILER
3.7	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		949	200275255555555555555555555555555555555	TOTAL	

COUNTY: GRAVES
DISTRICT: 1
ROUTE: KY
MILEPOST: 0.100
ROAD DESIGNATION: U
LATEST ADT COUNT: 913 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB ONTO KY 21 KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

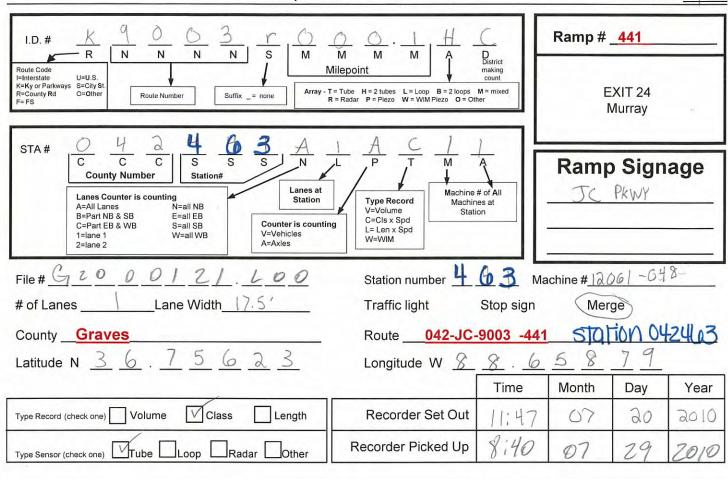
VEHICLE CLASSIFICATION COUNT

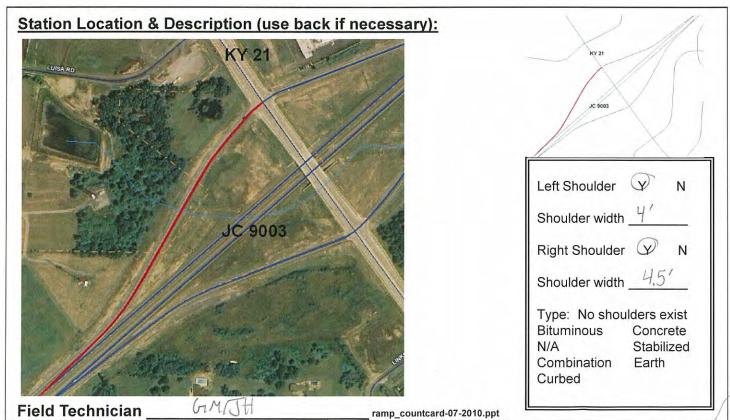
Y: GRAVES

2010

TATION: 4 STATION: 462 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

District #





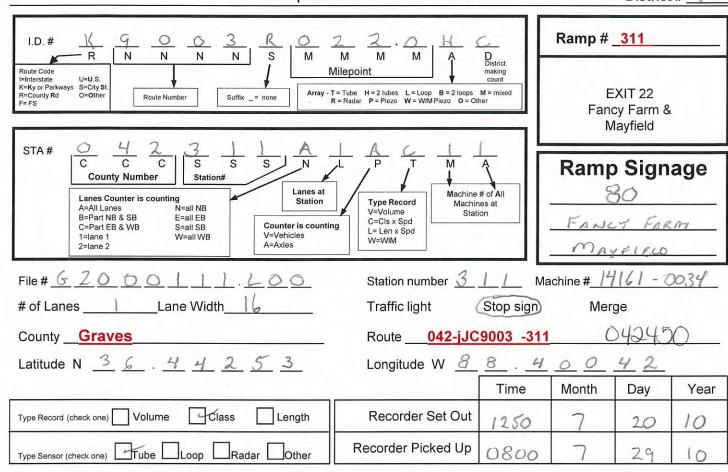
MONTHLY AXLE FA TOTAL H AM HIGH PM HIGH		TOTALS:	DATE: 12-7-6-5-4-3-12-11-11-11-12-7-6-5-4-3-12-11-11-11-11-11-11-11-11-11-11-11-11-		ROUTE:
NTHLY FACTOR: LE FACTOR: TAL HOURS: HIGH HOUR: HIGH HOUR:		: 1441	PMM 1366 PMM 1370 PMM		ROUTE: KY9003R
97 100 168 123 163		_	00800010101000000100000000000000000000		
BETWEEN	AVERAGE	1552	THU 22 100 100 100 100 100 100 100 100 100	WEEK OF	*POR
10-11 4- 5	GE DAILY	1493	FRI 1002 425 110 5 7 8 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	F JULY	*PORTABLE TF
AM ON THE	TRAFFIC:	917	7274 7274 744 757 767 767 767 767 767 767 767 767 767	21 10	GRAVES COUNTY
MONDAY	1306	809	12784529954959959595959595959595959595959595	JULY	DER
MILE PO COUNTED DATA SO ARRAY LANES		1712	MON	27 2010	REPORT*
POINT TED BY SOURCE Y S COUNTED		1508	TUE 10 1 1 1 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		
: 0.1 : CEN OFF : CLASS : 2 TUBES : ALL LANES		9432	TOTAL: 61 35 34 40 60 160 292 292 455 625 625 626 627 627 747 176 338 338 338		STATION 463
		32	277875766657200066453468 2778775666657500666453468 277875766657600066453468		

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 28%

TP%%A%	PERCENT	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-11AM 11-11AM 11-01PM 01-02PM 03-04PM 03-04PM 06-07PM 07-08PM 08-09PM 08-09PM 09-10PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 08-09AM 08-04AM 01-02AM 01-02AM 01-03AM 01-03AM 01-03AM	× -3	
% HEAVY TRU AXLES / TRU X TRAILER T % TRAILERS PEAK HOUR E TOTAL HOURS	PERCENT OF TOTAL	HICLES	7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 7/220/10 10 10 10 10 10 10 10 10 10 10 10 10 1	DATE	
DCKS DCKS DPK HR	0.6	œ	000000000	ES P	
112/ 464/ 78/ 6/ 4-05PM	71.5	970	24744898989898989898989898989898989898989	PASSENGER CARS	
1357 = 8.3 112 = 4.1 1357 = 5.7 129 = 4.7 129 VEHI	19.7	267	0EEEEEEEEEEEE	VEHI VEHI	1
8.3 % 4.143 5.7 % 4.7 % VEHICLES	0.1	2	000000000000000000000000000000000000000	BUSSES 6	
	1.1	15	00000000000000000000000000000000000000	∃&	SINGLE UNIT TRUCKS
%%% >	1.2	16	0000000	3 AXLE	T TRUC
AXLE CORE % SINGLE % TRUCKS	0.1	_	00000000000000000000	4 OR MORE AXLE	KS
AUNC	1.0	14	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
TON FACTOR T TRUCKS T a PK HR PEAK HOUR	4.6	62	N44554456666666666666666666666666666666	5 AXLE	TRUCKS
2 11 11 11 11	0.1	2	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.92 34/ 1/ 7/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
1357 129 129	0.0	0	000000000000000000000000000000000000000	6 AXLE	TI-TRAILER TRUCKS
5.4 5.4	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
%%%	0.0	0*	000000000000000000000000000000000000000	RS	
		1357	28882400442 1121 200442 2004 2004 2004 2004	TOTAL	

COUNTY: GRAVES
DISTRICT: 1
ROUTE: KY
MILEPOST: 0.100
ROAD DESIGNATION: U
LATEST ADT COUNT: 1306 IN 2010
LOCATION INFORMATION: RAMP FROM KY 21 ONTO PURCHASE PARKWAY SB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
2010 STATION: 4 STATION: 463 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

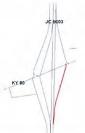
District #



Station Location & Description (use back if necessary):



Field Technician DS DC ramp_countcard-07-2010.ppt



Left Shoulder	(Y)	N
Shoulder width	4	_
Right Shoulder	\odot	N
Shoulder width	8	
Type: No shou	lders e	xist
Bituminous	Concr	
N/A	Stabil	ized

Earth

Combination

Curbed

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: 1227 1317 1317 1317 1317 1317 1317 1317		ROUTE: KY9003R
CTOR: 97 100 168 168 182 182		1128	WED 117778886774200777777777777777777777777777777777		003R
BETWE	AVER	1123	THU HU 108 108 108 108 108 108 108 108 108 108	WEEK	2
EN 10-11	AGE DAIL	1159	7224856789687776666273 722485667896877766666273 722485667896877766666777666677766776776776776776776	OF JULY	FUK ABLE
AM ON T	AVERAGE DAILY TRAFFIC:	852	2024472472472472474724747474747474747474	21	GRAVES CO
TUESDAY	C: 996	696	782 700 700 700 700 700 700 700 700 700 70	דס שערא	COUNTY
MILE COUNT DATA ARRAY LANES	8	1160	MON TO THE SECOND TO THE SECON	27 2010	X
POINT NTED BY SOURCE		1072	100 50 50 50 50 50 50 50 50 50 50 50 50 5	0	
: 22.0 : CEN OFF : CLASS : 2 TUBES : ALL LANES		7190	TOTALS 56 38 41 27 110 187 415 405 446 517 646 547 426 181 131		STATION 311

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

PERCENT	TOTAL VE	000000111000000000111000	2 - 3	
OF TOTAL	HICLES	77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10	DATE	
0.5	υī	-0000000000000-00-0	MOTOR- CYCLES	
76.4	802	1447747747747747747747747747747747747747	PASSENGER CARS	
17.9	188	oureevery 12212122299999999999999999999999999999		
0.0	0	000000000000000000000000000000000000000	BUSSES	SINGLE
1.3	14	00000000000000000000000000000000000000	2 AXLE 6 TIRE	
0.8	00	000000000000	3 AXLE	UNIT TRUCKS
0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	CKS
1.0	10	0000000000	4 OR LESS AXLE	SINGLE
2.2	23	-00000-0-NN-NNNN0	5 AXLE	TRUCKS
0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUL
0.0	0	000000000000000000000000000000000000000	6 AXLE	TI-TRAILER TRUCKS
0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
0.0	0*	000000000000000000000000000000000000000	OTHERS	
	1050	132246887274757688876747576776776776776776776776777677777777	TOTAL VEHICLES	
	0.5 76.4 17.9 0.0 1.3 0.8 0.0 1.0 2.2 0.0 0.0 0.0 0.0	CLES 5 802 188 0 14 8 0 10 23 0 0 0 0 0* TOTAL 0.5 76.4 17.9 0.0 1.3 0.8 0.0 1.0 2.2 0.0 0.0 0.0 0.0 0.0	7/20/10 1 48 12 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	DATE MOTOR- PASSENGER 2 THER 3

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 22.000

ROAD DESIGNATION: U

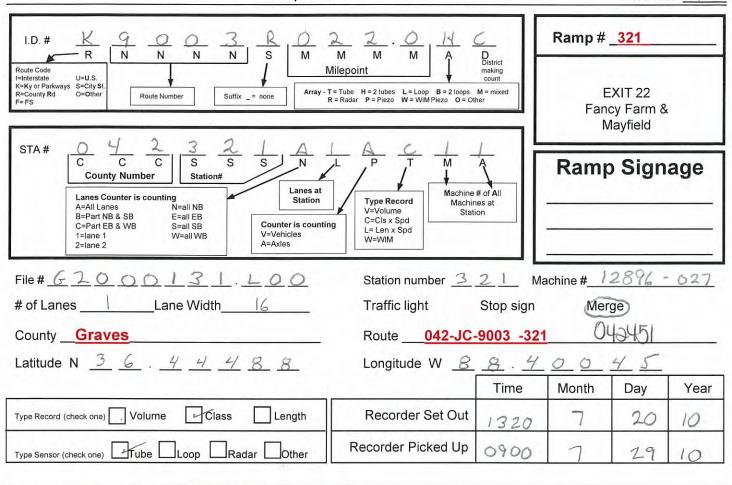
LATEST ADT COUNT: 996 IN 2010

LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB TO KY 80

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
2010

7: GRAVES STATION: 450 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 12

District #



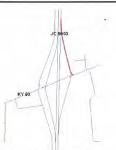
Station Location & Description (use back if necessary):



Field Technician

25 DC

ramp_countcard-07-2010.ppt



Left Shoulder (Y)	N
Shoulder width	
Right Shoulder (Y)	N
Shoulder width	
Type: No shoulders e	

Type: No shoulders exist
Bituminous Concrete
N/A Stabilized
Combination Earth
Curbed

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAYE:	ROUTE: KY9003R
		1460	WED 1131 124 125 126 127 127 127 127 127 127 127 127	003R
97 100 168 178 BETWEEN 124 BETWEEN	AVERA	1533	77 103 103 103 103 103 103 103 103 103 103	WEEK OF
7- 8 AM	AVERAGE DAILY TRAFFIC:	1560	FRI 1105 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	ב יחרא
90	TRAFFI	1026	SAT 1 8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	GRAVES COU
MONDAY THURSDAY	C: 1329	979	SUN 10 10 10 10 10 10 10 10 10 10 10 10 10	COUNTY TO JULY
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT	9	1577	MON MON MON MON MON MON MON MON MON MON	27 2010
OURCE		1463	TUE 707 103 103 103 103 103 103 103 103	
: Z2.0 CEN OFF CLASS : Z TUBES : ALL LANES		9598	TOTALS 27 27 27 27 27 28 42 89 59 59 59 59 59 59 59 59 59 59 59 59 59	STATION 321

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 888

TOTAL HOURS

11 24

7 P % % X L	PERCENT	TOTAL VEHICLES	06-07AM 08-09AM 09-10AM 11-12AM 11-12AM 12-01PAM 01-02PM 02-03PM 02-03PM 04-07PM 06-07PM 06-07PM 07-08PM 09-10PM 11-12PM 11-12PM 11-12PM 07-03AM 02-03AM 04-05AM	TIME	
EAVY TR ES / TR RAILER RAILERS K HOUR	PERCENT OF TOTAL	HICLES	77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10	DATE	
UCKS = 89/ UCK = 288/ TRUCKS = 32/ BPK HR = 3/ BETWEEN 07-08AM	0.6	9	0000000000	MOTOR- CYCLES	
89/ 288/ 32/ 7-08AM	70.3	984	3923258555555555555555555555555555555555	PASSENGER CARS	
1400 = 6. 89 = 3. 1400 = 2. 139 = 2. 139 VEH	22.7	318	1611113359762222222336 2601036	OTH 2 A 4 T VEHI	
6.4 % 3.236 2.3 % 2.2 % VEHICLES	0.1	3	000000000000000000000000000000000000000	BUSSES 6	SINIS
	2.7	38	ND0000000077N4MMMMMMMMNMM	2 AXLE 6 TIRE	SINGLE UNIT TRUCKS
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AXLE CORR % SINGLE % SINGLE % TRUCKS	0.5	7	그리아 1일 레이지 그리아이다니 그리스 등 등을 제 속에 되었다.	4 OR MORE AXLE	KS
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ON FAC TRUCK a) PK EAK HC	1.4	19			TRUCKS
UR = 0	0.1	_	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
57/ 6/ 9/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULT
139 = 139 =	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
644	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	THERS	
		1400	41000000000000000000000000000000000000	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

ROUTE: KY
MILEPOST: 22.000

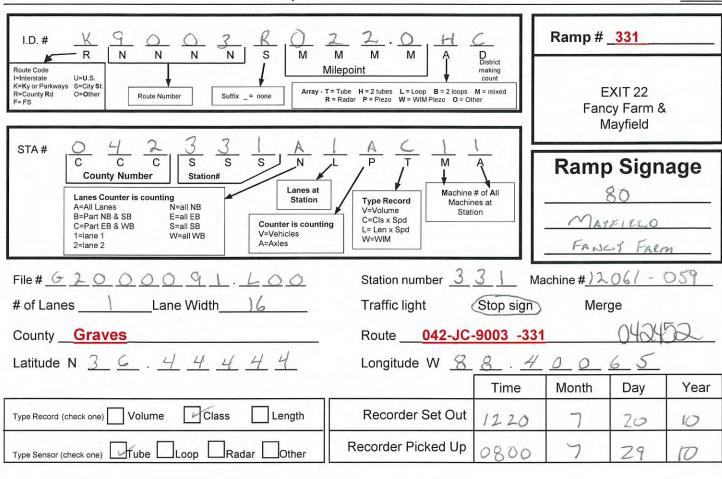
ROAD DESIGNATION: U

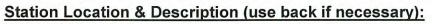
LATEST ADT COUNT: 1329 IN 2010

LOCATION INFORMATION: RAMP FROM KY 80 ONTO PURCHASE PARKWAY NB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **YEHICLE CLASSIFICATION COUNT**
2010 STATION: STATION: 451 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

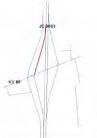
District #







Field Technician ______ ps_ _____ ramp_countcard-07-2010.ppt



4
Ý N
8
llders exist Concrete Stabilized Earth

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: DAYE: DAYE: 12-1 1-2 1-2 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4 3-4		ROUTE: KY9003R	
CTOR: 97 R : 100 S : 168 UR: 99 UR: 169		1334	WED 1088 1		003R	
7 0 9 BETWEEN	AVERAGE	1391	14 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	WEEK C		
11-12 AM 4- 5 PM	GE DAILY	1454	FRI 3 5 5 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	OF JULY	GR.	
0 Q	DAILY TRAFFIC:	1032	SAT 22 212 7 8 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	21 10	GRAVES COUNTY	
FRIDAY		950	1300 8 4 7 7 6 5 5 7 7 6 5 5 7 8 8 4 3 7 7 1 1 1 2 5 5 5 7 8 8 4 3 7 1 1 1 2 5 5 5 7 8 8 4 3 7 1 1 1 2 5 5 5 6 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6	JULY	NTY	
MILE P COUNTE DATA S ARRAY LANES		1384	MON	27 2010		
POINT ED BY SOURCE	-0	1416	TUE 7 27 3 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3 3 4 7 2 3		**	
22.0 CEN OFF CLASS 2 TUBES 2 TUBES	19	8961	TOTALS 80 46 30 22 36 71 167 349 406 418 558 558 558 588 588 588 588 588 588 732 735 735		STATION 331	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 %%%

TP%%A%	PERCENT	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 12-01AM 01-02PM 02-03PM 03-04PM 06-07PM 06-07PM 07-08PM 08-09PM 09-10PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 11-12PM 03-04AM 03-04AM 03-04AM	20 -	
% HEAVY TRU AXLES / TRU X TRAILER T X TRAILERS PEAK HOUR B TOTAL HOURS	PERCENT OF TOTAL	EHICLES	77220010 77220010 77220010 77220010 77220010 77220010 77220010 77220010 77220010 77220010 77220010 77220010	DATE	
TRUCKS = TRUCK = R TRUCKS = RS aPK HR= R BETWEEN C	0.6	œ	000000000000000000000000000000000000000	MOTOR- CYCLES	
95/ 276/ 28/ 28/ 2/ 14-05PM	59.1	770	1000444408014 400444408014 400446048	PASSENGER CARS	
1302 = 7.3 95 = 2.9 1302 = 2.2 133 = 1.5 133 VEHI	32.9	429	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	OTHER 2 AXLE 4 TIRE VEHICLES	
7.3 % 2.905 2.2 % 1.5 % VEHICLES	0.2	N	0000000000000000	BUSSES 6	SINGLE
	3.8	49	NNNN44NNN144NN←N←000000←	2 AXLE 5 TIRE	
%%%AXLE	1.1 0	14	000000000000000000000000000000000000000	3 4 0 AXLE MOR AXL	UNIT TRUCKS
XLE CORRECTIC SINGLE UNIT SINGLE UNIT TRUCKS AT PE	0.2 1.	2	000000000000000000000000000000000000000	4 OR 4 OR MORE LESS	
ABAZ	2 0.9	6 1	00000000000000000000000000000000000000		SINGLE TRA
N FACTOR = TRUCKS = D PK HR = AK HOUR =	9 0.0	2 0	000000000000000000000000000000000000000	6 OR E MORE AXLE	TRAILER
0.97 67/ 6/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MUC
1302 = 133 = 133 =	0.0	0	000000000000000000000000000000000000000	AXLE I	TRUCKS
4.5.1 %%	0.0	0	000000000000000000000000000000000000000	7 OR MORE O	ER
0.000	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		1302	124 U4 6 1 1 2 4 1	TOTAL	

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 22.000

ROAD DESIGNATION: U

LATEST ADT COUNT: 1241 IN 2010

LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB TO KY 80

STATION: 452 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

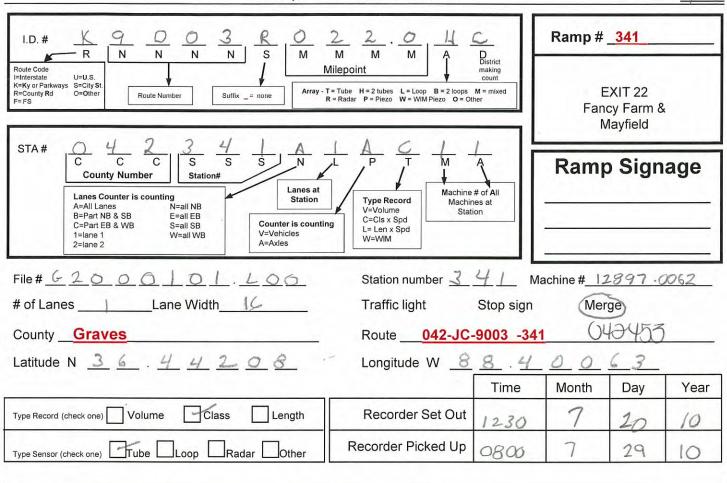
VEHICLE CLASSIFICATION COUNT

7: GRAVES

2010

TATION:

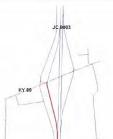
District # /



Station Location & Description (use back if necessary):



Field Technician DS DC ramp_countcard-07-2010.ppt



1	L	
Left Shoulder	(Y)	N
Shoulder width	4	
Right Shoulder	(Y)	N
Shoulder width	6	
Type: No shou Bituminous N/A Combination	lders e Conc Stabil Earth	rete lized

MONTHLY FAC AXLE FACTOR TOTAL HOURS AM HIGH HOU PM HIGH HOU		TOTALS:	DATE: 12-1 12-1 13-2 2-3 AM 2-3 AM 4-5-6 AM 7-8 AM 10-11 AM 11-12		ROUTE: KY90	
R		1143	CO 4 5 C C C C C C C C C C C C C C C C C C		03R	
BETWEEN	AVERA	1184	T T T T T T T T T T T T T T T T T T T	WEEK O		. 0
11-12 A	GE DAILY	1170	FRI 30 60 70 70 885 885 885 885 885 885 885 885 885 88	F JULY	GR	12000
22	TRAFFIC	848	S 24 37 37 37 37 37 37 37 37 37 37 37 37 37	21 10	AVES COU	20,100
ESDAY		711	805947477 80594749957477 8059499574777 8059777777777777777777777777777777777777	JULY	NTY	CONDEN
MILE P COUNTE DATA S ARRAY LANES		1184	NOW 2017 20 20 20 20 20 20 20 20 20 20 20 20 20	27 2010		
NT BY RCE		1146	T T T T T T T T T T T T T T T T T T T			
CEN CLAS					STATION	
NES T		7386	TOTALS 47 34 166 167 177 405 446 446 446 446 446 446 446 446 446 44		341	
	THLY FACTOR: 97 E FACTOR : 100 COUNTED BY : CEN OFF AL HOURS : 168 HIGH HOUR: 81 BETWEEN 11-12 AM ON TUESDAY ARRAY : 2 TUBES HIGH HOUR: 108 BETWEEN 4-5 PM ON MONDAY LANES COUNTED: ALL LANE	AVERAGE DAILY TRAFFIC: 1023 THLY FACTOR: 97 E FACTOR: 100 COUNTED BY: CEN OFF AL HOURS: 168 HIGH HOUR: 81 BETWEEN 11-12 AM ON TUESDAY ARRAY HIGH HOUR: 108 BETWEEN 4-5 PM ON MONDAY LANES COUNTED: ALL LANE	ALS: 1143 1184 1170 848 711 1184 1146 AVERAGE DAILY TRAFFIC: 1023 THLY FACTOR: 97 E FACTOR: 100 E FACTOR: 100 AL HOURS: 168 HIGH HOUR: 81 BETWEEN 11-12 AM ON TUESDAY ARRAY: 2 TUBES HIGH HOUR: 108 BETWEEN 4-5 PM ON MONDAY LANES COUNTED: ALL LANE	E: 21 22 23 24 25 26 27 I AM 4 7 6 7 12 MON TUE AM 5 5 7 7 12 MON TUE AM 9 4 5 7 12 MON TUE AM 9 4 5 7 12 MON TUE AM 9 4 6 5 7 12 MON TUE AM 9 4 6 5 7 12 MON TUE AM 9 4 6 5 7 12 MON TUE AM 9 4 6 5 7 12 MON TUE ALS: 116 BETWEEN 1-12 AM ON TUESDAY LANES COUNTED: ALL LAN	HEEK OF JULY 21 TO JULY 27 2010 E: 21 22 23 24 25 26 27 I AM 5 7 6 7 12 6 5 AM 6 7 7 12 6 5 AM 8 13 13 13 6 6 11 9 AM 6 8 73 70 32 13 74 60 AM 72 69 67 56 40 66 76 AM 72 69 67 56 40 66 76 AM 83 82 83 70 47 74 60 B PM 83 82 83 70 47 74 60 AL ROURS 100 AVERAGE DAILY TRAFFIC: 1023 WILE POINT E CLASS B PH HOUR: 108 BETWEEN 11-12 AM ON TUESDAY ARRAY LANES COUNTED: ALL LAN	TE: KY9003R WEEK OF JULY 21 TO JULY 27 2010 E: 21 22 23 24 25 26 27 2010 E: 4 PED 1140 FRI SAT SUN MON TUESDAY STATION 3 WEED 12 27 2010 FRI SAT SUN MON TUE STATION 3 ANA 5 5 6 6 1 7 8 3 5 5 2 4 6 6 6 6 6 6 6 6 6 7 6 6 6 7 6 6 7 6 6 7 6 6 7 6 6 7 6 7 6 7 6 7 6 7 6 7 7 8 8 7 7 7 8 7 7 7 8 8 7 7 7 8 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 7 8 8 7 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 8 7 7 7 8 7 7 7 8 7 7 7 8 7 7 7 8 7 7 7 8 7 7 7 8 7 7 7 8 7 7 7 7 8 8 7 7 7 7 8 7 7 7 8 7 7 7 7 8 7 7 7 7 8 7 7 7 7 8 7 7 7 7 8 7

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 28%

*

PERCENT	TOTAL VE	06-07AM 08-08AM 08-09AM 09-10AM 11-12AM 11-12AM 12-015PM 03-05PM 03-05PM 03-06PM 06-07PM 07-08PM 08-09PM 09-10PM 11-12PM 11-12PM 11-201AM 01-03AM 01-03AM 01-03AM 01-03AM	TIME PERIOD	
OF TOTAL	HICLES	77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10 77/220/10	DATE	
0.7	7	-0000000000000000	MOTOR- CYCLES	
72.3	766	0.55744455555555555555555555555555555555	PASSENGER CARS	
20.0	212	0001222222000	OTHE 2 AX 4 TI VEHIC	
0.3	3	000000000000000	BUSSES 6	SING
1.8	19	00000000000000000000000000000000000	AXLE TIRE	SINGLE UNIT TRUCKS
2.0	21	00000NNN0	3 AXLE	T TRUC
0.2	N	000000000000000000000000000000000000000	4 OR MORE AXLE	KS
0.8	00	000000000000	4 OR LESS AXLE	SINGLE
2.1	22	0-0-0-00-0	5 AXLE	TRUCKS
0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	M.C.
0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ILER
0.0	0*	000000000000000000000000000000000000000	OTHERS	
	1060	210 u u u u u u u u u u u u u u u u u u u	TOTAL VEHICLES	
	.7 72.3 20.0 0.3 1.8 2.0 0.2 0.8 2.1 0.0 0.0 0.0 0.0	7 766 212 3 19 21 2 8 22 0 0 0 0 0* 0.7 72.3 20.0 0.3 1.8 2.0 0.2 0.8 2.1 0.0 0.0 0.0 0.0	7720/10 0 45 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	DATE CYCLES PASSENGER 2 THER 2 AXLE AXLE MORE LESS

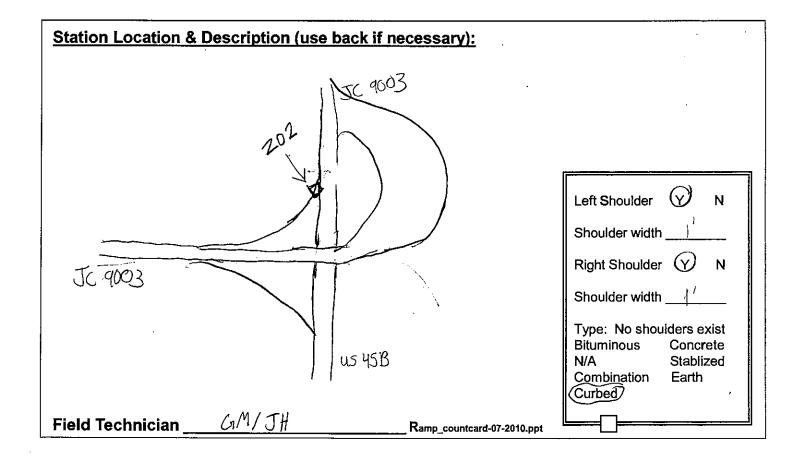
COUNTY: GRAVES
DISTRICT: 1
ROUTE: KY
MILEPOST: 22.000
ROAD DESIGNATION: U
LATEST ADT COUNT: 1023 IN 2010
LOCATION INFORMATION: RAMP FROM KY 80 ONTO PURCHASE PARKWAY SB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

/: GRAVES 2010 STATION: 4 STATION: 453 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

District #

Ramp# 202 K I.D.# R Ν М Route Code Milepoint i=Interstate
K=Ky or Parkways
R=County Rd Ų=U.\$. S=City St O=Other -T=Tube H=2 tubes L=Loop B=2 loops M=mixed R=Redar P=Plezo W=WIM Plezo O=Other Route Number F=FS STA# Ramp Signage **County Number** Station# PKWY, Memphis Machine # of All Lanes Counter is counting Station Type Record Machines at A=All Lanes N=all NB V=Volume B=Part NB & SB E=all EB C=Cls x Spd Counter is counting C=Part EB & WB L≃ Len x Spd 1=lane 1 W=WIM A=Axles 2=lane 2 Lane Width # of Lanes Traffic light Stop sign (Merge) County Giraves Phwy <u>station 042900</u> Route 6 7 Longitude W Latitude N Time Month Day Year 2010 Type Record (check one) Volume Class Recorder Set Out 07 20 Length 14:13 29 Recorder Picked Up 07 2010 Type Sensor (check one) Radar



MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	110 98 76 54 44 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ROUTE: KY9003R
TOR: 97 : 100 : 168 : 168 JR: 209 JR: 283		3326	VE 22 23 2 24 25 25 25 25 25 25 25 25 25 25 25 25 25)03R
BETHEEN	AVERAGE	3331	TREE TO SEE THE SEE TH	WEEK O	
10-11 A	GE DAILY	3561	726623333333333333333333333333333333333	ATOF 30	ç.
AM ON F	TRAFFIC	2882	SAT 47 47 47 47 47 47 47 47 47 47 47 47 47	21 10	GRAVES CO
RIDAY RIDAY	C: 3058	2517	SUN	ס אחרג	COUNTY
MILE P COUNTE DATA S ARRAY LANES	œ	3247	MORE SERVICE S	27 2010	
POINT TED BY SOURCE Y S COUNTED		3207	108 34 34 34 34 34 34 34 34 34 34 34 34 34		
CEN OFF CLASS TUBES		22071	TOTALS 278 278 278 279 175 207 251 437 437 1086 1086 1167 1167 1168 11595 11496 888 888 8995 434		STATION 900

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00
OTHER 2 AXLE 4 TIRE VEHICLES 00
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 % % %

% HEAVY TRUCKS = 1188/ 3164 = 37.5 % AXLES / TRUCK = 4960/ 1188 = 4.175 % TRAILER TRUCKS = 843/ 3164 = 26.6 % TRAILERS apk HR= 51/ 238 = 21.4 % PEAK HOUR BETWEEN 03-04PM 238 VEHICLES TOTAL HOURS = 24	PERCENT	TOTAL VE	06-07AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 12-03PM 03-05PM 03-05PM 06-07PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM 07-08PM		
	OF TOTAL	VEHICLES	7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 7/20/10 10/20/10 10/20/10 10/20/10 10/20/10	DATE	
	1.6	50	しししのしとししころころままかままままとし	MOTOR- P	
	31.7	1002	244422 24442	ASSE	
	29.2	924	250054456994568644696446964469644496944696444969449694496944969496949694969496949	S m m	
	0.9	27		BUSSES	SINGLE
	7.1	224		2 AXLE 6 TIRE	SLE UNIT
****A T S S L	1.4	45	しらざろろろ かろくろろう クリー・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・	3 4 AXLE	TRUCKS
Z C C C C C C C C C C C C C C C C C C C	1.5	49	00000000000000000000000000000000000000		S
ONIT UNIT AT P	3.0	96	484977797789848800~0~~~B	4 OR LESS AXLE	SING
ON FACTOR = 0.71 TRUCKS = 345/3164 = 10.9 % Tapk HR = 26/238 = 10.9 % PEAK HOUR = 77/238 = 32.4 %	22.8	721	20000000000000000000000000000000000000		GLE TRAIL
	0.2	7	000000000000000000000000000000000000000	6 OR MORE AXLE	ER
	0.6	18	NO-0000-N-00000-N-1-1-1-N	5 OR LESS AXLE	MCL1
	0.0	_	000000-000000000000000	6 AXLE	TRUCKS
	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	E E
	0.0	0 *	000000000000000000000000000000000000000	OTHERS	
		3164	00000000000000000000000000000000000000	TOTAL	

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 0.100

ROAD DESIGNATION: U

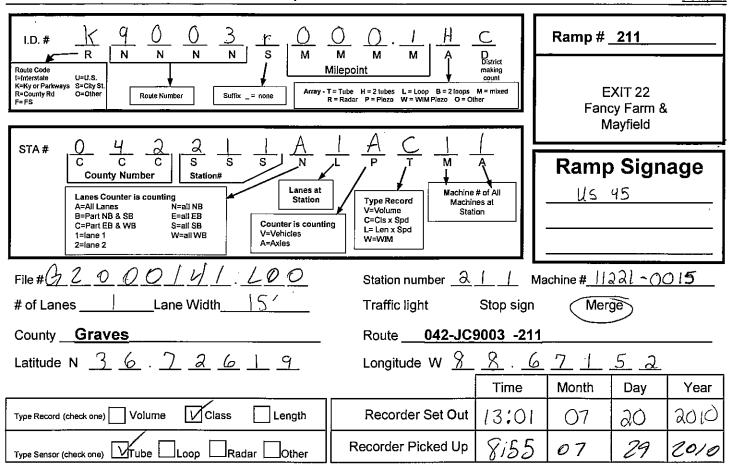
LATEST ADT COUNT: 3058 IN 2010

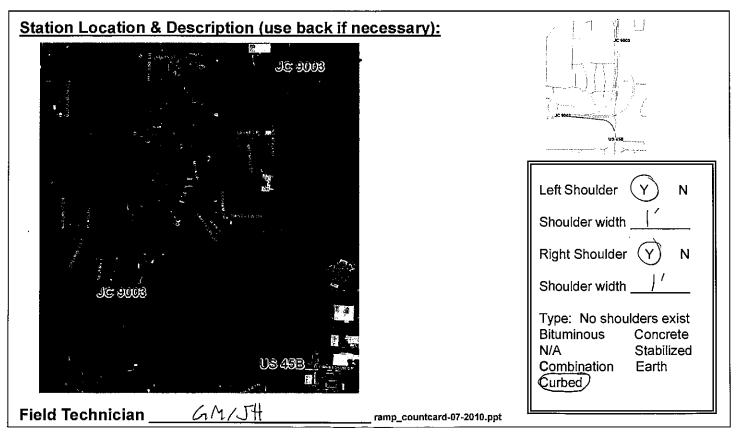
LOCATION INFORMATION: RAMP FROM KY PURCHASE PKWY/US 45B ONTO PURCHASE PKWY SB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT
7: GRAVES
2010
STATION: 9

District #





DIVISION OF TRANSPORTATION PLANNING *PORTABLE TRAFFIC RECORDER REPORT*

	TOTALS:	DATE DAYE 127 1-1-1-1-5-4 2-1-2-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1		ROUTE: KY9003R
,	324	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		03 _R
AVERAG	300	THU 22 85 22 22 22 22 22 22 22 22 22 22 22 22 22	WEEK OF	
	319	22 L 2 L 2 L 2 L 2 L 2 L 2 L 2 L 2 L 2	י יחרג	GR.A
TRAFFIC:	238	2 S S S S S S S S S S S S S S S S S S S	21 TO	GRAVES COUNTY
: 271	186	52 NA 2 L 2 M 2 L 2 C 2 C 2 C 2 C 2 C 2 C 2 C 2 C 2 C	JULY	YTY
	298	20 M M M M M M M M M M M M M M M M M M M	27 2010	
	300	L L L L L L L L L L L L L L L L L L L		10
	1965	TOTALS 118 12 12 12 12 12 12 12 12 12 12 12 12 12		STATION 211
	AVERAGE DAILY TRAFFIC: 271	: 324 300 319 238 186 298 300 AVERAGE DAILY TRAFFIC: 271	E: 21 22 23 24 25 26 27 IHU FRI SAT SUN MON TUE AM 1 1 2 4 4 4 3 3 4 5 6 4 4 4 3 4 6 4 4 4 4 4 4 4 4 4 6 4 6 4	HEEK OF JULY 21 TO JULY 27 2010 E: 21 22 23 24 25 26 27 I AM 1 1 1 2 3 4 4 4 3 3 3 2 3 3 4 4 4 4 3 3 3 2 3 3 4 4 4 4

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 36 36 36

% HEAVY TRUCKS = AXLES / TRUCK = % TRAILER TRUCKS = % TRAILERS apk HR= PEAK HOUR BETWEEN O TOTAL HOURS =	PERCENT	TOTAL VEHICLES	06-07AM 07-08AM 09-10AM 11-12AM 11-12AM 11-12AM 01-02PM 02-03PM 05-06PM 06-07PM 09-10PM 11-12PM 11-12PM 01-01AM 01-02AM 01-05AM 01-05AM	TIME PERIOD	
	OF TOTAL	HICLES	77777777777777777777777777777777777777	DATE	
	0.0	0	000000000000000000000000000000000000000	MOTOR- CYCLES	
28/ 132/ 26/ 2/ 3-04PM	68.2	189		PASSENGER CARS	
277 = 10.1 3 28 = 4.714 277 = 9.4 3 27 = 7.4 3 27 VEHICL	21.7	60	-0-00-200-2044555566666666666666666666666666666666	OTHER 2 AXLE 4 TIRE VEHICLES	
S C C K K K K K K K K K K K K K K K K K	0.0	0	000000000000000000000000000000000000000	BUSSES	SINGLE
AXLE CO % SINGL % TRUCK	0.7	2	000000000000000000000000000000000000000	2 AXLE 6 TIRE	GLE UNIT
	0.0	0	000000000000000000000000000000000000000	3 AXLE	TRUCKS
	0.0	0		4 OR MORE AXLE	်
RRECTI	0.7	N	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
ON FAC TRUCK a) PK EAK HO	8.7	24	000000000-NNNNN-NN-NN-	5 AXLE	TRAI
TOR III	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
2/0/2/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MOL.
277 = 0.7 % 27 = 0.0 % 27 = 7.4 %	0.0	0	000000000000000000000000000000000000000	6 AXLE	I-TRA
	0.0	0	000000000000000000000000000000000000000	7 OR MORE	LER
	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		277	4-M-NM6/860000/0001/14/2000/00/00/00/00/00/00/00/00/00/00/00/0	TOTAL	

COUNTY: GRAVES

2010

STATION: 211 BOT
DISTRICT: 1

ROUTE: KY9003R
MILEPOST: 0.100

ROAD DESIGNATION: U

LATEST ADT COUNT: 271 IN 2010

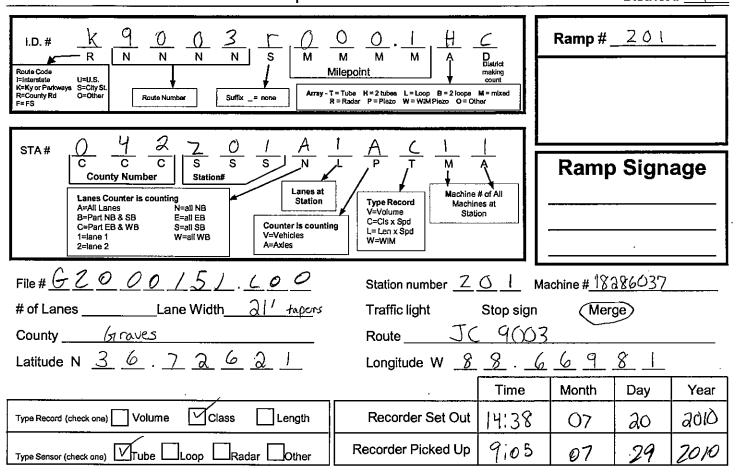
LOCATION INFORMATION: RAMP US 45B TO CONTINUE ON PURCHASE PARKWAY NB

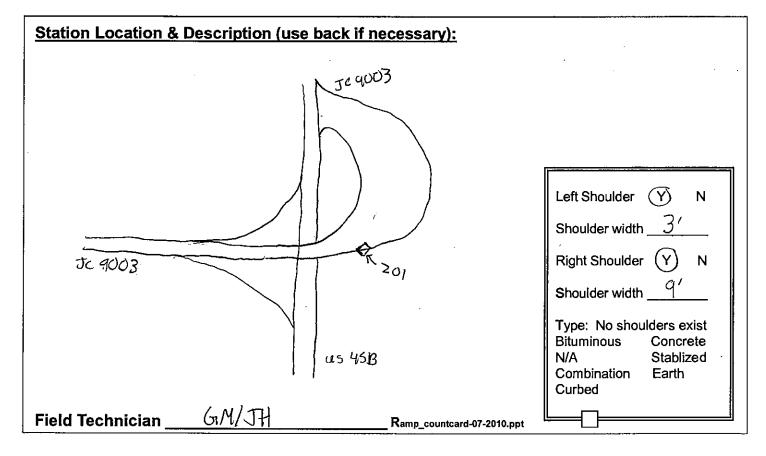
STATION: 211 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 12

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT
7: GRAVES
2010
STATION:

District #





MONTHLY FACTOR: 97 AXLE FACTOR: 100 TOTAL HOURS: 168 AM HIGH HOUR: 260 PM HIGH HOUR: 291		TOTALS: 3348	DATE: DAY: 12-11 DAY: 1-2-1 1-2-1 2-1-2 3-4-4 3-4-4 3-4-4 3-4-4 3-10 3-10 3-10 3-10 3-10 3-10 3-10 3-10		ROUTE: KY9003R
BETWEEN	AVERAG	3521	THU 222222222222222222222222222222222222	WEEK OF	
9-10 AM 4- 5 PM	AVERAGE DAILY	3667	174000000000000000000000000000000000000	JULY	GR/
ON N	TRAFFIC:	2871	SAT 1 103 22 23 27 24 26 66 67 67 67 67 67 67 67 67 67 67 67 67	21 10	GRAVES COUNTY
SATURDAY THURSDAY	: 3060	2410	SUN	YJULY	NTY
MILE P COUNTE DATA S ARRAY LANES		3105	M P P P P P P P P P P P P P P P P P P P	27 2010	
E POINT NTED BY NOURCE		3165	11112222222222222222222222222222222222		40
CEN OFF CLASS 2 TUBES ALL LANES		22087	TOTAL 107 AL 1092 4092 4093 4094 4094 4094 4096 4096 4096 4096 4096		STATION ZO1

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 %
OTHER 2 AXLE 4 TIRE VEHICLES 00 %
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

THE PRIOD DATE CYCLES CARS 2 ANLE 3 4 OR 4 OR 5 OR 6 7 OR 700						
MOTOR- PASSENGER 2 MALE MOTOR- PASSENGER P	⊒ 5%%≯%	PERCENT				
NOTOR- PASSENGER CARLE OFFICE CARLE OFFICE CARLE OFFICE CARLE OFFICE OF	≖ 0	ŏ	HICLES		× —	
SINGLE UNIT TRUCKS	RUCKS PPK HR ETWEEN	<u>.</u>	34	00000000000000000000000000000000000000	MOTOR- CYCLES	
SINGLE UNIT TRUCKS REAL BUSSES OF TIRE AXLE MORE LESS AXLE MORE L	1053 4574 835 87-04-05P	44.	1395	2287772888446888888888888888888888888888	ASSENGER CARS	
SINGLE UNIT TRUCKS BUSSES 6 TIRE AXLE MORE LESS AX	33. 26. 25.	21.8	690	250 250 250 250 250 250 250 250 250 250	AXLE TIRER	
SINGLE TRAILER TRUCKS TRUCKS TRUCKS OR 4 OR BRE LESS AXLE MORE CTICLE AXLE MORE LESS AXLE MORE CTICLE O	*4**1	0.7	22	0000-0-000	USSES	NIS
SINGLE TRAILER TRUCKS TRUCKS TRUCKS OR 4 OR ERS LESS AXLE HORE LESS AXLE HORE LESS AXLE HORE AXLE		4.0	126	๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛	AXL TIR	GLE UNI
SINGLE TRAILER TRUCKS TRUCKS TRUCKS OR 4 OR BRE LESS AXLE MORE CTICLE AXLE MORE LESS AXLE MORE CTICLE O	æ 3€ 3€ 3€	2.2	70	→→○○○○○○→○→○ ₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	3 AXLE	TRUC
SINGLE TRAILER TRUCKS OR 5 6 6 7 7 8 8 8 8 8 8 8 9 42 1 1 1 1 1 1 1 1 1 1 1 1 1	국22 5	0.0	0	000000000000000000000000000000000000000	MORE AXLE	KS
TRAILER MULTI-TRAILER TRUCKS TRUC	AUSE	3.7	116	440087707868848888488888474468	4 OR LESS AXLE	SING
LER MULTI-TRAILER 5 OR 5 OR 6 7 OR TRUCKS MORE LESS AXLE MORE OTHERS VEHICLE AXLE AXLE MORE AXLE O 0 0 175 1 1 0 0 0 0 175 1 1 0 0 0 0 175 1 1 0 0 0 0 218 1 1 0 0 0 0 228 1 1 0 0 0 0 228 1 1 0 0 0 0 228 1 1 0 0 0 0 228 1 1 0 0 0 0 228 1 1 0 0 0 0 228 1 1 0 0 0 0 228 2 1 0 0 0 0 228 2 1 0 0 0 0 228 2 1 0 0 0 0 228 2 1 0 0 0 0 228 2 2 1 0 0 0 0 228 3 2 2 0 0 0 0 0 228 3 2 3 1 0 0.0 0.0 5 7 0.0 0.0 0.0 8 2 3 172 = 6.9 % 8 = 7.1 % 7 3/ 226 = 32.3 %	E P Z P	21.7	687	11111111111111111111111111111111111111	AXLE	ı⊂
MULTI-TRAILER TRUCKS 5 OR 6 7 OR OTHERS VEHICLE AXLE MORE OTHERS VEHICLE AXLE AXLE MORE OTHERS VEHICLE AXLE O	22 2	•	œ	00000000000	AXLE	ILER
I-TRAILER TRUCKS TRUCKS AXLE MORE OTHERS VEHICLE AXLE MORE OTHERS VEHICLE CLE CLE CLE CLE CLE CLE CLE CLE CLE	N•	0.7	23	-N00N00000N-N0N	5 OR LESS	
TOTAL TO	17 22 22	0.0	_	0000000-000000000000	6 AXLE	TRUCK
THERS VEHICLE 0 175 0 201 0 20	32.76	0.0	0	000000000000000000000000000000000000000	7 OR AXLE	ILER
TOTAL VEHICLES 1714 1795 1795 2218 2218 2218 2218 2218 2218 222 822 8	36 36 3 6	0.0	Q		THERS	
			3172	1090 1090 1090 1090 1090 1090 1090 1090	TOTAL	

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 0.100

ROUTE: KY9003R

TYPE COUNT: AUTOMATIC

ROAD DESIGNATION: U

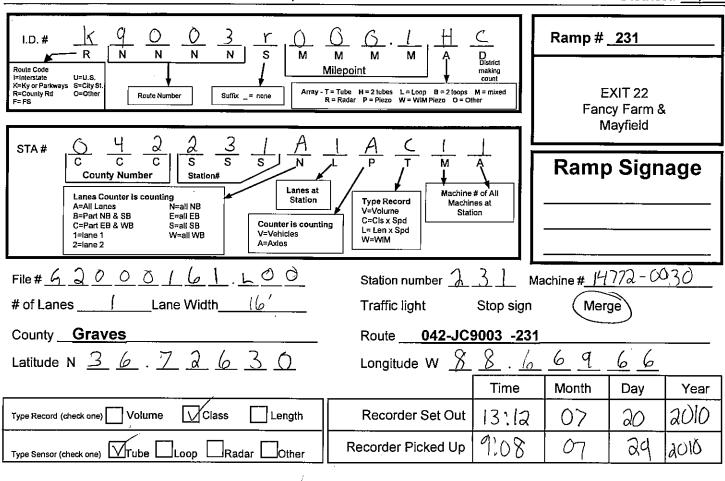
LATEST ADT COUNT: 3060 IN 2010

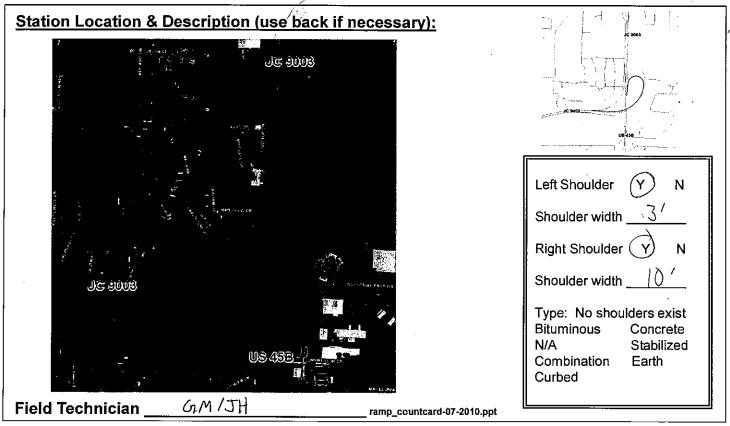
LATEST ADT COUNT: AMP FROM PURCHASE PKWY NB TO CONTINUE ON PURCHASE PKWY/US 45 N

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
2010
STATION: 21

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS: 3	DATE: 12-12 AM 23-4 AM 34-4 AM 35-4 AM 37-8 AM 37-8 AM 37-12 AM 37-8 AM 37-8 AM 37-8 PM 37-8 P		ROUTE: KY9003R
: 100 : 168 : 27		317	# 2000000000000000000000000000000000000		
BETWEEN BETWEEN	AVERAG	315	T 2200480866666666666666666666666666666666	WEEK OF	
7- 8 AM 4- 5 PM	AVERAGE DAILY TRAFFIC:	328	FRI 33 12 12 12 12 12 12 12 12 12 12 12 12 12	JULY	GRA
0 <u>0</u>	TRAFFI	237	72 A1 L L C C C C C C C C C C C C C C C C C	21 1	GRAVES COUNTY
THURSDAY THURSDAY	C: 273	188	รฐ ผู้ผู้ผู้ผู้ เก็ก ผู้ คือ	TO JULY	UNTY
MILE POI COUNTED DATA SOU ARRAY LANES CO		310	9240-00090000000000000000000000000000000	27 2010	
POINT TED BY SOURCE Y COUNTED		279	TUE 27		
CEN OFF		15	10		STATION 231
		1974	TOTALS 115 123 123 123 123 124 124 127 127 127 127 127 127 127 127 127 127		

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 % 3K 3K TOTAL HOURS

II 24

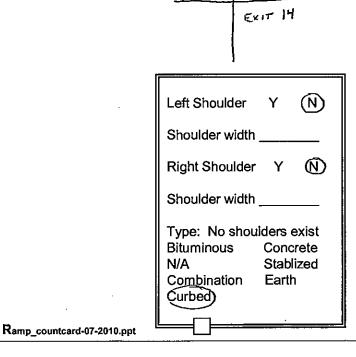
. % % % G I	PERCENT	TOTAL VEHICLES	06-07AM 08-09AM 09-10AM 11-11AM 11-11AM 12-01PAM 01-02PAM 02-03PAM 02-05PAM 07-08PAM 07-08PAM 07-08PAM 07-01PAM 01-11PAM 01-01AM 01-01AAM 05-05AAM	TIME PERIOD	
% HEAVY TRU AXLES / TRU % TRAILER T % TRAILERS PEAK HOUR B	PERCENT OF TOTAL	HICLES	77777777777777777777777777777777777777	DATE	
TRUCKS = 2 R TRUCK = 2 R TRUCKS = 2 RS apk HR= RS apk HR=	0.0	0	000000000000000000000000000000000000000	MOTOR- CYCLES	
49M 33/ 34/	35.9	103	~&WAMMM&&&& \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	PASSENGER CARS	
287 = 23.7 68 = 3.4 287 = 11.8 21 = 14.3 21 VEHI	40.4	116	N80101001801044W4W	OTHER 2 AXLE 4 TIRE VEHICLES	
3.7% 3.426 1.88% 4.3%	0.3	_	000000000000000000000000000000000000000	BUSSES	NIS
	11.5	33		2 AXLE 6 TIRE	SINGLE UNIT
%%% <u>\$</u>	0.0	0	000000000000000000000000000000000000000	AXLE M	TRUCKS
AXLE CORREC % SINGLE UN % SINGLE UN % TRUCKS AT	0.0	0	000000000000000000000000000000000000000	4 OR 4	S
RRECTION E UNIT THE E UNIT OF S AT PEAN	1.7	υī	0000000000000000	4 OR LESS AXLE	SINGLE
C P C F A C	10.1	29	~~000000~0~~~~NNNNNWWW\		TRUCKS
	0.0	0			LER
34/ 5/	0.0	0	000000000000000000000000000000000000000		MULT
287 = 21 = 21 =	0.0	0	000000000000000000000000000000000000000	AXLE	I - TRAI
11.8 9.5 23.8	0.0	0	000000000000000000000000000000000000000	7 OR MORE	ER
36 36 36	0.0	o _‡	000000000000000000000000000000000000000	OTHERS	
		287	2-1-100500000000000000000000000000000000	TOTAL VEHICLES	

COUNTY: GRAVES DISTRICT: 1 ROUTE: KYS MILEPOST: 0.100 ROAD DESIGNATION: U LATEST ADT COUNT: 273 IN 2010 LOCATION INFORMATION: RAMP FROM PURCHASE PKWY NB ONTO US 458 SB KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT** 2010 STATION: STATION: 231 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 12

Kentucky Transportation Cabinet - Division of Planning

Ramp Ira	affic Count Record			District	#
=Interstate U=U.S. K=Ky or Parkways S=City St. B=County Rd O=City St.	M M M M M M M M M M M M M M M M M M M		Ramp#	111	
STA# C C C S S S N County Number Lanes Counter is counting A=All Lanes N=all NB B=Part NB & SB E=all EB C=Part EB & WB S=all SB 1=lane 1 W=all WB Z=lane 2 AALIES Counter is counting V=Vehicles A=Axles	Type Record V=Volume C=Cis x Spd L= Len x Spd W=WiM	es at	3	Sign 39 1702 160	age
File # <u>6 2 1 0 0 0 3 1 . L 0 0</u> # of Lanes <u>1 Lane Width 16</u> County <u>GRAVES</u> Latitude N <u>3 6 . 3 8 7 7 7 4</u>	Station number Traffic light (Route <u>0 ー 2 -</u> Longitude W <u>8</u>	Stop sign		ge)	30 2142 Year
Type Record (check one) Volume Class Length	Recorder Set Out	0915	7	21	10
Type Sensor (check one) Lifube Loop Radar Other Station Location & Description (use back		1000 K	¥ 339	29 56 PKY	10 12

Field Technician __



DIVISION OF TRANSPORTATION PLANNING *PORTABLE TRAFFIC RECORDER REPORT*

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: 12-1 1-2 3-4 5-6 7-8 8-9 8-9 11-12 11		ROUTE: KY9003R
TOR: 97 : 100 : 168 R: 17 R: 32		242	10000000000000000000000000000000000000		03R
BETWEEN	AVERAGE	268	FRI 3116 1 5 2 1 1 1 1 1 6 6 6 6 7 1 1 1 1 1 1 1 1 1 1	WEEK OF	٠
11-12 AM 5- 6 PM	E DAILY	206	38777795077770035734-1-2-6277	: JULY	GR/
22	DAILY TRAFFIC	139	22 NA *** CO **** CO ***********************	22 10	GRAVES COUNTY
MONDAY Tuesday	: 215	227	MOR 28 28 28 28 28 28 28 28 28 28 28 28 28	ATOF	NTY
MILE PO COUNTED DATA SO ARRAY LANES (236	75 THE STATE	28 2010	
E POINT NTED BY A SOURCE :		242	20000000000000000000000000000000000000		s
14.0 CEN OFF CLASS 2 TUBES ALL LANES					STATION 111
ίνi		1560	TOTALS 18 17 17 12 18 88 88 88 88 88 88 88 88 88 88 88 88		

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00
OTHER 2 AXLE 4 TIRE VEHICLES 00
4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 % % %

AXL PEA TOT	PERCENT OF	TOTAL VEHICLES	06-07AM 07-08AM 08-09AM 09-10AM 11-12AM 11-12AM 01-02PM 02-03PM 03-04PM 03-04PM 04-07PM 08-09PM 08-09PM 08-09PM 01-12PM 11-12PM 11-12PM 11-12PM 11-03AM 03-04AM	TIME PERIOD	
A A A B B	OF TOTAL	HICLES	7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10 7721/10	DATE	
VY TRUCKS = / TRUCK = ILER TRUCKS = ILER TRUCKS = ILERS aPK HR= HOUR BETWEEN 0	1.4	S	0000000000	MOTOR-	
20/ 67/ 8/ 0/ 5-06PM	63.8	139	このよろのなんでいっているようとともものできるともものできるのできるというというというというといいましょう	PASSENGER CARS	
218 = 9.2 20 = 3.3 218 = 3.7 23 = 0.0 23 VEH1	25.7	56		OTHER 2 AXLE 4 TIRE VEHICLES	
350 350 37% 10%	0.0	0	000000000000000000000000000000000000000	BUSSES 6	SINGLE
	4.1	9	000000000000000000000000000000000000000	AXLE 5 TIRE	HE UNIT
⊄ 3≮ 3≮ 26	1.4	ы	000000000000	3 AXLE	TRUCKS
AXLE CORF % SINGLE % SINGLE % TRUCKS	0.0	0	0000000000000000000000	4 OR MORE AXLE	Κ̈́ς
AT P	0.0	0	000000000000000000000	4 OR LESS AXLE	SING
ON FACTOR TRUCKS D PK HR PEAK HOUR	3.7	œ	000000000000000000000000000000000000000	5 AXLE	SINGLE TRAILER
0 11 11 0	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.94 12/ 1/ 1/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULI
218 23 ==	0.0	0	000000000000000000000000000000000000000	6 AXLE	I - TRAILER
5.44 7.44	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ER
96 96	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		218	0000	TOTAL VEHICLES	

COUNTY: GRAVES DISTRICT: 1 ROUTE: KY9 MILEPOST: 14.000 ROAD DESIGNATION: R LATEST ADT COUNT: 215 IN 2010 LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB ONTO KY 339

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

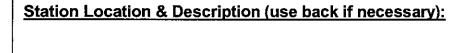
VEHICLE CLASSIFICATION COUNT

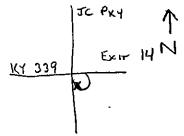
STATION: STATION: 142 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #

I.D. # K 9 0 0 3 R M Route Code R N N N N N S M M M M M M M M M M M M M M	District making count M = mixed	Ramp# 121 Exi7 14				
STA# C C C C S S S N County Number Lanes Counter is counting A=All Lanes N=all NB B=Part NB & SB E=all EB C=Part EB & WB S=all SB 1=lane 1 W=all WB 2=lane 2 C C C S S S N Lanes at Station Counter is counting V=Vehicles A=Axles	Type Record V=Volume C=Cls x Spd L= Len x Spd W=WIM	es at .	Ramp	Sign	age	
File# <u>G 2 1 0 0 0 2 1 . L 0 0</u>	Station number	2 <u>1</u> Ma	chine # 2	036 - 0	0026	
# of LaneslLane Width16	Traffic light	Stop sign	Mer			
County GRAVES	Route 042 - 3	16-9003	-121	<u> </u>	<u>43</u>	
Latitude N <u>3 6 . 3 8 7 3 3</u>	Longitude W 8	<u>8</u> .4	52	14		
		Time	Month	Day	Year	
Type Record (check one) Volume Lass Length	Recorder Set Out	0900	7	21	10	
Type Sensor (check one)	Recorder Picked Up	1000	7	29	10	





	<u> </u>
	Left Shoulder Y (Ñ)
	Shoulder width
	Right Shoulder Y (N)
	Shoulder width
	Type: No shoulders exist Bituminous Concrete N/A Stablized Combination Earth Curbed
_	

Ramp_countcard-07-2010.ppt

MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: DAY: DAY: DAY: DAY: DAY: DAY: DAY: DAY		ROUTE: KY9003R
OR: 97 : 100 : 168 :: 47 :: 36		375	T T T T T T T T T T T T T T T T T T T)3R
BETWEEN	AVERAC	355	FRI 31122227421011212238274210121212121212121212121212121212121212	WEEK OF	
7- 8 AM 5- 6 PM	E DAILY	304	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	: JULY	GR/
22	AVERAGE DAILY TRAFFIC:	235	์ อูกอาวายุ อูกอุกอุกอุกอุกอุกอุกอุกอุกอุกอุกอุก รัฐ	22 TO	GRAVES COUNTY
WEDNESDAY Tuesday	333	368	NOV 20 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	JULY	łΤΥ
MILE PO COUNTED DATA SO ARRAY LANES C		376	TURE 7 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	28 2010	
OURCE :		395			s
14.0 CEN OFF CLASS 2 TUBES ALL LANES		2408	TOTALS 118 124 124 152 152 169 169 169 169 169 169 169 169 169 169		STATION 121

NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 **36 36 36**

T P E A X L I	PERCENT	TOTAL VE	06-07AM 08-09AM 09-10AM 11-12AM 12-01PM 01-02PM 02-03PM 05-06PM 05-06PM 09-10PM 11-12PM 11-12PM 11-12PM 01-03AM 02-03AM 03-04AM		
AL HOLL SAILE SAILE SAILE	OF TOTAL	VEHICLES	77777777777777777777777777777777777777	DATE	
RUCKS = BPK HR=ETWEEN	0.0	0	000000000000000000000000000000000000000	MOTOR- CYCLES	
250/ 524/ 7/ 07-08AM	26.7	91	C B C I F O D A D D D D D D O C C C C C C C C C C C C C	PASSENGER CARS	
341 = 73.3 % 250 = 2.096 341 = 2.1 % 35 = 0.0 % 35 VEHICLI	0.0	0		m m m	
s E % % % %	45.2	154	๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛๛	BUSSES 6	SINGLE
	25.8	88	り8404555005442432~2~0~2~	2 AXLE 5 TIRE	SLE UNIT
# AXLE # SII # TRI	0.3 0			3 4 AXLE MC	TRUCKS
_ ~ ~		0	000000000000000000000000000000000000000		,,
AUNI	0.3	_	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGL
ION FACTOR T TRUCKS T @ PK HR PEAK HOUR	1.2	4	000000000000000000000000000000000000000	5 AXLE	SINGLE TRAILER
יה ווווווי	0.3	_	000000000000000000000000000000000000000	6 OR MORE AXLE	ER
0.97 243/ 23/ 23/ 23/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULT
341 35 	0.0	0	000000000000000000000000000000000000000	6 AXLE	I-TRAILE
71.3 65.7 65.7	0.3		000000000000000000000000000000000000000	7 OR MORE AXLE	Ę
34 34 34	0.0	0 *	000000000000000000000000000000000000000	χ ω	
		341	,	TOTAL VEHICLES	

COUNTY: GRAVES
DISTRICT: 1
ROUTE:
MILEPOST: 14.000
ROAD DESIGNATION: R
LATEST ADT COUNT: 333 IN 2010
LOCATION INFORMATION: RAMP FROM KY 339 ONTO PURCHASE PKWY NB STATION: 143 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT
STATION:

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District#

= Interstate U=U.S. X=Ky or Parkways S=City St. Array - T	M M M A Aillepoint T=Tube H=2 tubes L=Lcop B=2 loops R=Radar P=Piszo W=WIM Piszo O=Oth	District making count M = mixed er	Ramp#_131 Exit 14			
STA# C C C C S S S N County Number Lanes Counter is counting A=All Lanes B=Part NB & SB E=all EB C=Part EB & WB S=all SB 1=lane 1 W=all WB 2=lane 2 Counter is counting V=Vehicles A=Axles	Type Record V=Volume C=Cis x Spd L=Len x Spd W=WIM Machine Machine Stati	es at .	Ramp	Sign	age	
File# <u>62100011.L00</u>	Station number	3 <u>]</u> Ma	chine # <u>18</u>	<u> 286 · c</u>	001	
# of Lanes Lane Width. 16	Traffic light	Stop sign	Mer	ge		
County GRAVES	Route 042 ·	JC-900	3-13	046	2144	
Latitude N <u>3 6</u> . <u>3 8 7 7 1</u>	<u>8</u> .4	51.	9 &	•		
		Time	Month	Day	Year	
Type Record (check one) Volume 4 Class Length	Recorder Set Out	0900	7	21	10	
Type Sensor (check one)	Recorder Picked Up	1000	1 7	29	10	



TCPKY A ExITIA N

Left Shoulder	Y	N
Shoulder width		
Right Shoulder	Y	N
Shoulder width		
Type: No shoul Bituminous N/A Combination Curbed	Con	crete dized

Field Technician DS DC Ramp_countcard-07-2010.ppt

DIVISION OF TRANSPORTATION PLANNING *PORTABLE TRAFFIC RECORDER REPORT*

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE		ROUTE: KY9003R
TOR: 97 : 100 : 168 R: 36 R: 27		284	THO 22 33 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		03R
BETWEEN BETWEEN	AVERAG	318	5110895555672221553354681103E	WEEK OF	
6: 7 AM	AVERAGE DAILY TRAFFIC	242	222 222 222 223 223 224 224 225 225 225 225 225 225 225 225	. JULY	GRA
22	TRAFFIC:	162	กระการและสมอออกกราบการเพลง เรา เกาะเกาะเกาะเกาะเกาะเกาะเกาะเกาะเกาะเกาะ	22 TO	GRAVES COUNTY
TUESDAY FRIDAY	253	283	42 000 000 000 000 000 000 000 000 000 0	JULY	łγ
MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNT		256	7272 8688866986767867786778677867786778677867	28 2010	
OURCE :		283	8 d		εņ
14.0 CEN OFF CLASS 2 TUBES ALL LANES		1828	TOTALS 111 12 12 148 148 149 114 131 131 141 141 156 422 28		STATION 131

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 % OTHER 2 AXLE 4 TIRE VEHICLES 00 % 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %

7 % % A X L P & X T P E A A	PERCENT OF	TOTAL VE	06-07AM 08-09AM 09-10AM 11-11AM 11-11AM 02-03PM 02-03PM 05-06PM 06-07PM 09-109PM 11-12PM 11-12PM 01-03AM 02-03AM 04-05AM	P.E.	
L A A I L	금	VEHICLES	77721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100 27721100	DATE	
RUCKS II	0.8	2	000000-0000-00000000	MOTOR- CYCLES	
71/ 215/ 23/ 23/ 2/ 07AM	36.5	95	_ ○8444508555455555566666666666666666666666	PASSENGER CARS	
260 = 27.3 71 = 3.0 260 = 8.8 26 = 7.7 26 = 7.7 26 VEHI	35.4	92	899445555555555555555555555555555555555	OTHER 2 AXLE 4 TIRE VEHICLES	
C % % & %	2.7	7	000000000000000000000000000000000000000	BUSSES	SINGLE
	13.8	36	→→○○○○○→→○ ₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	2 AXLE 6 TIRE	GLE UNIT
₹ %%%	0.0	0	000000000000000000000000000000000000000	3 4 AXLE M	TRUCKS
AXLE CORREC % SINGLE UN % SINGLE UN % TRUCKS AT	1.9	ر.			S
222	2.7	7	00000000000000000000000000000000000000	4 OR LESS AXLE	SING
ION FACTOR T TRUCKS T @ PK HR PEAK HOUR	5.8	15		5 AXLE	SINGLE TRAILER
0 11 11 11	0.4	_	000000000000000000000000000000000000000	6 OR MORE AXLE	ER
0.88 48/ 6/ 8/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	¥C.
260 = 26 = 26 =	0.0	0	000000000000000000000000000000000000000	6 AXLE	I - TRAILER
18.5 23.1 30.8	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	ER .
ንሩ ን ሩ ንሩ	0.0	0,	000000000000000000000000000000000000000	OTHERS	
		260	00777777777777777777777777777777777777	TOTAL VEHICLES	

COUNTY: GRAVES

DISTRICT: 1

MILEPOST: 14.000

ROAD DESIGNATION: R

LATEST ADT COUNT: 253 IN 2010

LOCATION INFORMATION: RAMP FROM KY 339 ONTO PURCHASE PKWY SB

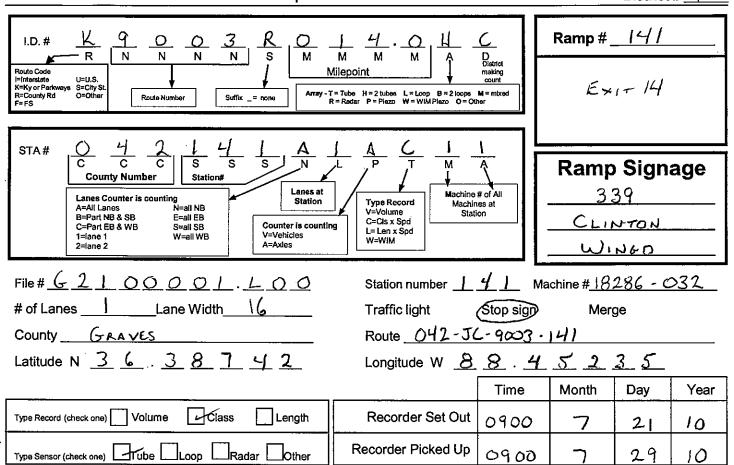
STATION: 144 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

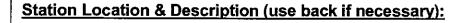
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

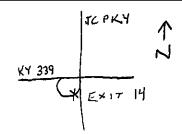
VEHICLE CLASSIFICATION COUNT
7: GRAVES
2010
STATION:

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #







Left Shoulder	Υ	(2)
Shoulder width		
Right Shoulder	Υ	Ñ
Shoulder width		<u>.</u>
Type: No shoul Bituminous N/A Combination Curbed	ders Cond Stab Eartl	crete lized
	· ••••	

Field Technician ___ D\(\) ___ Ramp_countcard-07-2010.ppt

ROUTE: KY9003R

WEEK OF JULY 22 TO JULY 28 2010

STATION 141

PORTABLE TRAFFIC RECORDER REPORT GRAVES COUNTY

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE 1227 A AMM A AMM A PRIMARY PRIMAR
~~ <u>F</u>		381	1 0 0 0 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2
97 00 68 21 BETWEEN 49 BETWEEN	AVERAGE	374	F R 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
9-10 <i>j</i> 5- 6 F	E DAILY	277	1188 4188 4188 4188 4188 4188
AM ON .	Y TRAFFIC:	229	1322711232000000000000000000000000000000
THURSDAY	IC: 321	359	081222244222316131317-1-0244222316131317-1-0244222316131317-1-0244222316131317-1-024422316131817-1-024422316131817-1-024422316131817-1-024422316131817-1-024422316131817-1-024422316131817-1-024422316181817-1-0244223161818181818181818181818181818181818181
MILE COUNTI		344	111 12448822111211
POINT : SOURCE : COUNTED:		357	WE BE BE SECURED TO A SECURED T
14.0 CEN OFF CLASS 2 TUBES ALL LANES			
Ö		2321	TOTALS 30 25 25 25 25 25 25 25 25 25 25 25 25 25

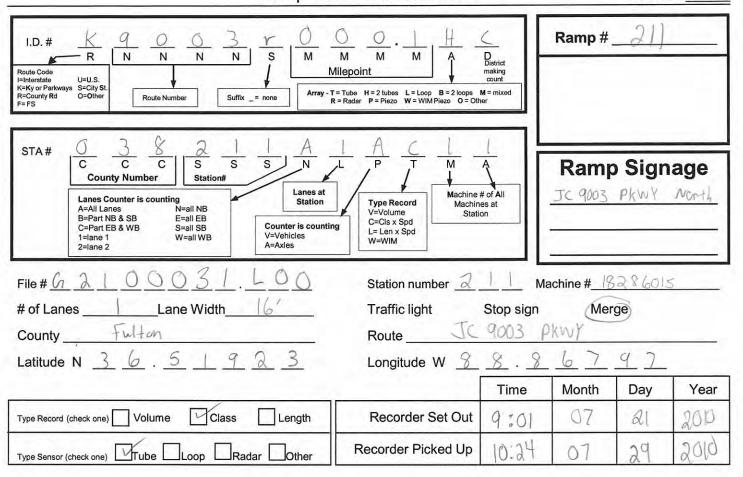
NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS
OTHER 2 AXLE 4 TIRE VEHICLES
4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 36 36 36

-1 ⊅%%≯ %	PERCENT	TOTAL VE	05-05AM 07-08AM 09-10AM 11-12AM 11-12AM 01-02PM 02-03PM 05-05PM 05-05PM 09-10PM 11-12PM 11-12PM 11-12PM 03-05AM 05-05AM	TIME PERIOD	
% HEAVY TRU AXLES / TRU X TRAILER T % TRAILERS PEAK HOUR B TOTAL HOURS	OF TOTAL	VEHICLES	77777777777777777777777777777777777777	DATE	
CKS	0.6	2	000000000000000000000000000000000000000	MOTOR- CYCLES	
18/ 60/ 9/ 1/ 05-06PM	73.2	243	48 <u></u>	PASSENGER CARS	
332 = 5.4 18 = 3.33 332 = 2.7 37 = 2.7 37 VEHIC	20.8	69		OTHER 2 AXLE 4 TIRE VEHICLES	
3333 7 % 7 % 1 CLES	0.0	0	000000000000000000000000000000000000000	BUSSES	SINGLE
	2.7	9	000000000000000000000000000000000000000	2 AXLE 6 TIRE	GLE UNIT
<****	0.0	0	000000000000000000000000000000000000000	3 4 AXLE M	TRUCKS
	0.0	0	000000000000000000000000000000000000000		S
LE CORRECTION SINGLE UNIT SINGLE UNIT TRUCKS AT PI	0.9	W	000000000000000000000000000000000000000	4 OR LESS AXLE	SING
ION FACTO TRUCKS OPK HR	1.8	Φ.	000000000000000000000000000000000000000		SINGLE TRAILER
~~ <u>9</u>	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	ER
0.97 9/ 1/ 2/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULT
332 37 ==	0.0	0	000000000000000000000000000000000000000	AXLE	I-TRAILER
52.7	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
36 36	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		332	1112847474747474747474747474747474747474747	TOTAL VEHICLES	

COUNTY: GRAVES
DISTRICT: 1
ROUTE:
MILEPOST: 14.000
ROAD DESIGNATION: R
LATEST ADT COUNT: 321 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PKWY SB ONTO KY 339 KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION
VEHICLE CLASSIFICATION COUNT
7: GRAVES
2010
STATION: STATION: 141 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





ht 3co

Jc 9003

Station Location & Description (use back if necessary):

Field Technician GM/J

Ramp_countcard-07-2010.ppt

Left Shoulder Y N

Shoulder width _____

Right Shoulder Y N

Shoulder width _____

Type: No shoulders exist
Bituminous Concrete
N/A Stablized
Combination Earth

Curbed

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DATE: 127-121-121-121-121-121-121-121-121-121-		ROUTE: KY9003R	
TOR: 97 : 100 : 168 : 168 JR: 35 JR: 43		338	T EC		3R	
BETWEEN	AVERA	383	00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	WEEK OF		*POR
7- 8 AM 4- 5 PM	AVERAGE DAILY TRAFFIC:	270	SAT 4 20 20 3 3 3 5 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	FJULY	FU	*PORTABLE IN
8 Q	TRAFFIC	219	S 20 20 20 20 20 20 20 20 20 20 20 20 20	22 10	FULTON COUNTY	TRAFFIC RECORDER
FRIDAY THURSDAY	: 300	326	MON SECURE 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	JULY	YTN	CORDER K
MILE PO COUNTED DATA SO ARRAY LANES O		306	TUE 7 10 10 10 10 10 10 10 10 10 10 10 10 10	28 2010		KEPOK! *
POINT TED BY SOURCE		330	7 6 3 7 6 3 7 6 3 7 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		10	
0.1 CEN OFF CLASS 2 TUBES ALL LANES		217	1 0 1		STATION 211	
NES F		2172	TOTALS 10 10 11 10 10		211	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS OTHER 2 AXLE 4 TIRE VEHICLES 4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 28% TOTAL HOURS

24

IP%%A%	PERCENT OF TOTAL	TOTAL VEHICLES	07-08AM 09-10AM 11-12AM 12-01AM 12-01AM 12-01AM 01-02PM 01-02PM 02-03PM 03-04PM 06-07PM 07-08PM 09-10PM 10-11PM 11-12PM 11-12PM 11-02AM 03-04AM 05-06AM	TIME	
% HEAVY TRU AXLES / TRU X TRAILER T X TRAILERS PEAK HOUR B	OF TOTAL	HICLES	77221110 77221110 77221110 77221110 77221110 77221110 77221110 77221110 77221110 77221110	DATE	
TRUCKS = 29/ TRUCK = 104/ R TRUCKS = 16/ RS @PK HR= 1/ RS @PK HR= 1/	0.0	0	000000000000000000000000000000000000000	MOTOR- CYCLES	
	71.8	221	นักรีนั้นก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็ก็	PASSENGER CARS	
308 = 9.4 29 = 3.5 308 = 5.2 28 = 3.6 28 VEHI	18.8	58	W D W D W D A D D W C C C C C C C C C C C C C C C C C	OTHER ER 2 AXLE 4 TIRE BU VEHICLES	
9.4 % 3.586 5.2 % 3.6 % VEHICLES	0.0	0	000000000000000000000000000000000000000	BUSSES 6	SING
	3.9	12	-0-00000000-0	2 AXLE 5 TIRE	SINGLE UNIT TRUCKS
%%%A	0.3	ح	000000000000000000000000000000000000000	3 AXLE	T TRUC
AXLE CORR % SINGLE % SINGLE % TRUCKS	0.0	0		4 OR MORE AXLE	KS
AT	1.0	3	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
ION FACTOR T TRUCKS T D PK HR PEAK HOUR	4.2	13	000000000000000000000000000000000000000		TRUCKS
RR OR	0.0	0	000000000000000000000000000000000000000		LER
13/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULT
308 28 28	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
= 4.2 7.1	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		308	400-105677875352977729847	TOTAL	

COUNTY: FULTON

STATION: 21

DISTRICT: 1

ROUTE: KYS

MILEPOST: 0.100

ROAD DESIGNATION: R

LATEST ADT COUNT: 300 IN 2010

LOCATION INFORMATION: RAMP FROM KY 307 ONTO PURCHASE PARKWAY SB

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

VEHICLE CLASSIFICATION COUNT

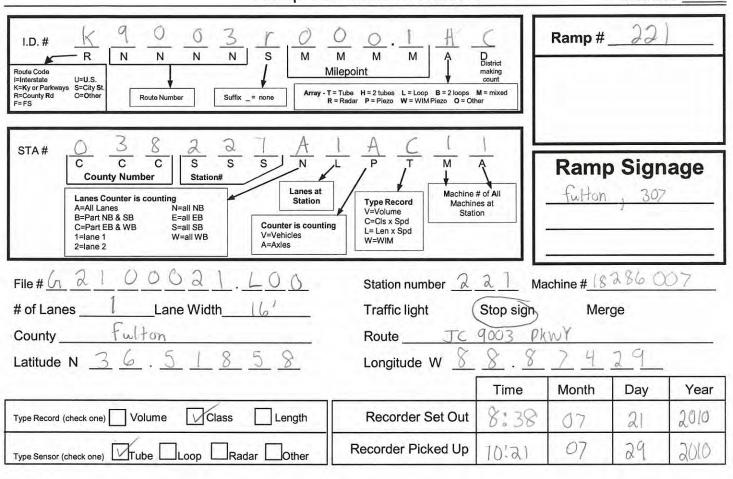
7: FULTON

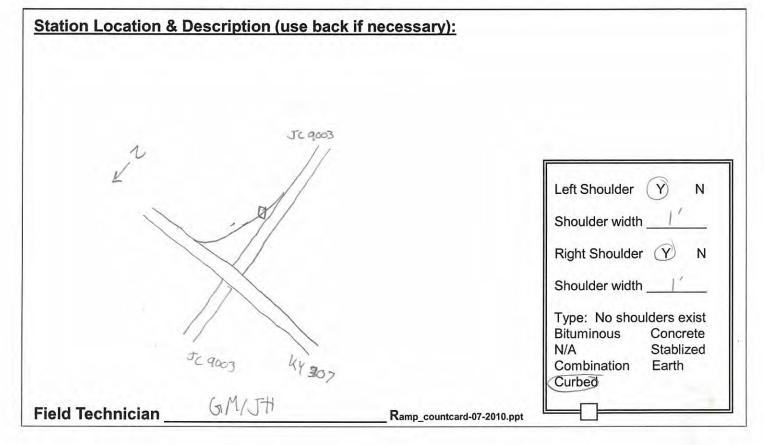
2010

STATION: 2 STATION: 211 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	DAY: 12-1 12-1 13-4 AM 2-3 AM 4-5 AM 7-8 AM 11-1 11-1 AM 11-1	DATE:	ROUTE: KY9003R	
TOR: 97 100 168 168 18: 30 JR: 48		461	11782223311252222113622222211362222222222222	22	103R	
8 BETWEEN	AVERA	461	FRI 1100000000000000000000000000000000000	WEEK OF		*PORTABLE
11-12 AM	AVERAGE DAILY TRAFFIC:	384	SAI 111111111111111111111111111111111111		Ę	
99	TRAFFI	281	\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	22 TO 25	FULTON COUNTY	TRAFFIC RECORDER
FRIDAY THURSDAY	396	434	00 - 6 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		YTN	
MILE P COUNTE DATA S ARRAY LANES	6	427	T C C C C C C C C C C C C C C C C C C C	28 2010 27		REPORT
POINT TED BY SOURCE		416	THE TOTAL PROPERTY OF	28		
CEN CLAS 2 TU					STATION	
OFF S BES LANES		2864	TOTALS 31 17 17 17 17 18 36 183 183 184 185 185 185 185 185 185 185 185 185 185		221	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%% TOTAL HOURS

= 24

IP%%A%	PERCENT	TOTAL VEHICLES	07-08AM 08-09AM 09-10AM 11-12AM 12-01PAM 12-01PAM 01-02PM 01-05PM 03-04PM 05-06PM 06-07PM 07-08PM 08-06PM 09-10PM 11-12PM 11-12PM 11-02AM 03-04AM 05-06AM	TIME PERIOD	
% HEAVY TRUCKS = 14/ AXLES / TRUCK = 50/ % TRAILER TRUCKS = 8/ % TRAILERS aPK HR= 2/ PEAK HOUR BETWEEN 03-04PM	PERCENT OF TOTAL	HICLES	777221110 777221110 777221110 777221110 777221110 777221110 777221110 777221110 777221110 777221110	DATE	
TRUCKS = TRUCK = R TRUCKS = R TRUCKS = R DPK HR= R BETWEEN	0.3	1	000000000000000000000000000000000000000	MOTOR- CYCLES	
	73.7	294	00000000000000000000000000000000000000	PASSENGER CARS	
399 = 3.5 14 = 3.5 399 = 2.0 36 = 5.6 36 VEHI	22.6	90	488857787787766000	OTHER 2 AXLE 4 TIRE VEHICLES	
3.5 % 3.571 2.0 % 5.6 % VEHICLES	0.0	0	000000000000000000000000000000000000000	BUSSES	
	1.5	6	000000000000000000000000000000000000000	2 AXLE 6 TIRE	
	0.0	0	000000000000000000000000000000000000000	3 AXLE	
AXLE CORF % SINGLE % SINGLE % TRUCKS	0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	
ONIT ONIT AT P	0.5	2	000000000000000000000000000000000000000	4 OR LESS AXLE	
ON FAC TRUCK a PK EAK HC	1.5	6	000000000000000000000000000000000000000	5 AXLE	TRUCKS
ווווו	0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	
0.97 6/ 1/ 3/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	
399 36 36	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
82.5 385	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	S
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		399	C1111222222222222222222222222222222222	TOTAL VEHICLES	

SINGLE UNIT TRUCKS

SINGLE TRAILER

MULTI-TRAILER

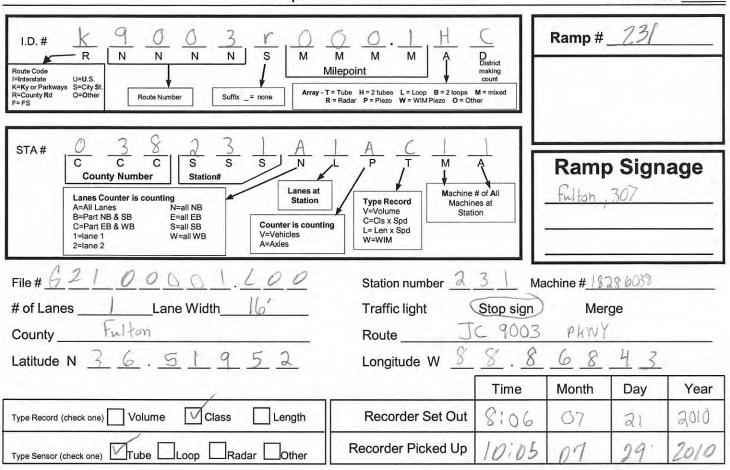
COUNTY: FULTON STATION: 22
DISTRICT: 1
ROUTE: KY9
MILEPOST: 0.100
ROUTE: KY9
TYPE COUNT:
ROAD DESIGNATION: R
LATEST ADT COUNT: 396 IN 2010
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY SB ONTO KY 307 STATION: 221 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

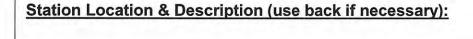
KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS
DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION

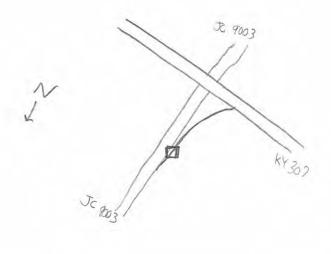
VEHICLE CLASSIFICATION COUNT
7: FULTON 2010 STATION: 2

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #







Field Technician GM/JH

Ramp_countcard-07-2010.ppt

Left Shoulder	Y N
Shoulder width	1/
Right Shoulder	Y N
Shoulder width	1
Type: No shou	ulders exist Concrete

Type: No shoulders exist
Bituminous Concrete
N/A Stablized
Combination Earth
Curbed

MONTHLY FACTOR AXLE FACTOR TOTAL HOURS AM HIGH HOUR PM HIGH HOUR		TOTALS:	12 A A A A A A A A A A A A A A A A A A A		ROUTE: KY9003G	
R		439	T T T T T T T T T T T T T T T T T T T		030	
97 100 168 26 BETWEEN 43 BETWEEN	AVERAGE	425	5824528333388225325333553355555555555555	WEEK OF		*PORTABLE
11-12 AM		347	SAT 122 321 22 22 22 22 22 22 22 22 22 22 22 22 2	AJULY	2	
90	DAILY TRAFFIC	274	S 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	22 10	FULTON COUNTY	RAFFICK
TUESDAY THURSDAY	C: 365	400	MON	D JULY	UNTY	KECORDER
MILE PO COUNTED DATA SO ARRAY LANES	G	370	T 27 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	28 2010		KETOK I *
POINT FED BY SOURCE		389	800 800 800 800 800 800 800 800 800 800		**	
CEN OFF CLASS 2 TUBES ALL LANES		2644	TOTALS 126 127 128 128 128 128 128 128 128 128 128 128		STATION 231	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS OTHER 2 AXLE 4 TIRE VEHICLES 4 OR LESS AXLE SINGLE TRAILER TRUCKS 888 ***

TOTAL HOURS

II 24

PERCENT	TOTAL VE	06-07AM 07-08AM 08-09AM 09-10AM 11-11AM 11-012AM 01-02PM 02-03PM 03-04PM 04-06PM 05-06PM 07-08PM 08-07PM 07-08PM 07-08AM 07-01AM 07-01AM 07-01AM 07-01AM 07-01AM	TIME	
OF TOTAL	HICLES	7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10 7/21/10	DATE	
0.3	_	000000000000000000000000000000000000000	MOTOR- CYCLES	
73.1	275	451111111111111111111111111111111111111	PASSENGER	
18.6	70	とのしししとし とり とり とり とり こう	V 420	
0.0	0	000000000000000000000000000000000000000	BUSSES	
4.0	15	0-0000000000NN-	2 AXLE 6 TIRE	SINGLE UNI
0.0	0	000000000000000000000000000000000000000	3 AXLE	UNIT TRUCKS
0.0	0	000000000000000000000000000000000000000	4 OR MORE AXLE	Ks
0.8	3	00000000000	4 OR LESS AXLE	SINGLE
3.2	12	00000000		TRUCKS TRUCKS
0.0	0	000000000000000000000000000000000000000	6 OR MORE AXLE	LER
0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULI
0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
0.0	0*	000000000000000000000000000000000000000	OTHERS	
	376	24242687965757576575765757575757575757575757575	TOTAL VEHICLES	
	73.1 18.6 0.0 4.0 0.0 0.0 0.8 3.2 0.0 0.0 0.0 0.0	1 275 70 0 15 0 0 3 12 0 0 0 0 0* 0.3 73.1 18.6 0.0 4.0 0.0 0.0 0.8 3.2 0.0 0.0 0.0 0.0	7/21/10 0 12 3 0 1 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0	DATE MOTOR PASSENGER 2 ANLE 3 4 OR 4 OR 5 6 OR 5 OR 6 7 OR 7

COUNTY: FULTON

STATION: 23
DISTRICT: 1

ROUTE: KYS
MILEPOST: 0.100

ROAD DESIGNATION: R

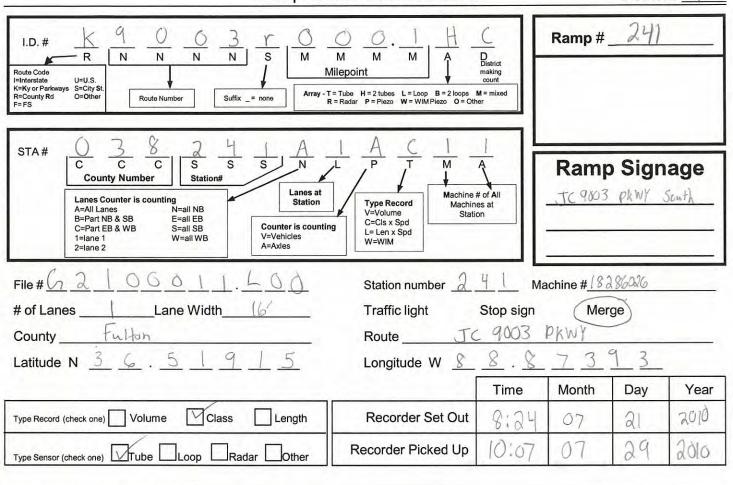
LATEST ADT COUNT: 365 IN 2010

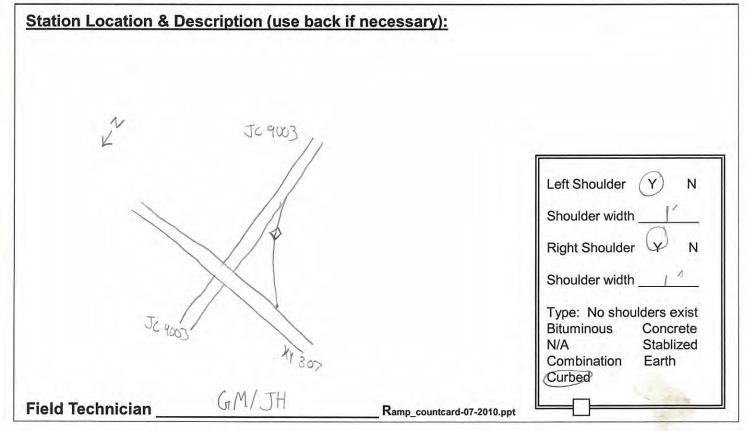
LOCATION INFORMATION: RAMP FROM PURCHASE PARKWAY NB ONTO KY 307 STATION: 231 BOTH N-S
ROUTE: KY9003R
TYPE COUNT: AUTOMATIC
FUNCTIONAL CLASS: 2

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**
2010
2010

Kentucky Transportation Cabinet - Division of Planning Ramp Traffic Count Record

District #





MONTHLY FACTO AXLE FACTOR TOTAL HOURS AM HIGH HOUR: PM HIGH HOUR:		TOTALS:	DATE: 12-7 1-2-1 23 34 35 45 45 45 45 45 4		ROUTE: KY9003R	
OR: 97 : 100 : 168 : 57		526	112 122 133 134 135 135 136 137 137 137 137 137 137 137 137 137 137		ž	
BETWEEN	AVERAG	508	FRI 111240000000000000000000000000000000000	WEEK OF		*PORTABLE
7- 8 AM	E DAILY	426	3222307335535555555555555555555555555555	JULY	2	
99	AVERAGE DAILY TRAFFIC	311	30 S S S S S S S S S S S S S S S S S S S	22 10	FULTON COUNTY	TRAFFIC RE
MONDAY	: 459	529	M	JULY	NTY	RECORDER
MILE POI	•	521	TER TO THE TOTAL TOTAL TO THE TO	28 2010		REPORT*
POINT ED BY SOURCE		498	EEB 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		**	
CEN OFF		3319	TOTALS 157 157 157 157 157 157 157 157 157 15		STATION 241	

* NOTE: RECOMMENDED OTHERS DISTRIBUTION IS CARS 00 OTHER 2 AXLE 4 TIRE VEHICLES 00 4 OR LESS AXLE SINGLE TRAILER TRUCKS 00 %%%

% HEA AXLES % TRA % TRA PEAK	PERCENT OF	TOTAL VEHICLES	07-08AM 07-08AM 08-09AM 09-10AM 11-11AM 11-01PM 01-02PM 01-02PM 03-04PM 03-04PM 04-06PM 07-08PM 08-09PM 09-10PM 01-11PM 01-12P	TIME PERIOD	
ILER HOUR	F TOTAL	ICLES	7/22/10 7/22/10 7/22/10 7/22/10 7/22/10 7/22/10 7/22/10 7/22/10 7/22/10 7/22/10	DATE	
	0.6	W	000000000000000000000000000000000000000	MOTOR- CYCLES	
31/ 120/ 21/ 21/ 2/ 17-08AM	69.0	331	120000000000000000000000000000000000000	PASSENGER CARS	
480 = 6.5 % 31 = 3.871 480 = 4.4 % 39 = 5.1 % 39 VEHICLE	24.0	115	0000-1500000000000000000000000000000000	OTHER 2 AXLE 4 TIRE VEHICLES	
71% CLES	0.0	0	000000000000000000000000000000000000000	BUSSES 6	SING
	1.9	9	00000000000	AXLE	SINGLE UNIT
%%% >	0.2	_	000000000000000000000000000000000000000	3 AXLE	T TRUCKS
AXLE CORF % SINGLE % SINGLE % TRUCKS	0.0	0		4 OR MORE AXLE	KS
AT F	1.2	6	000000000000000000000000000000000000000	4 OR LESS AXLE	SINGLE
TRUCK	3.1	15	-0-00-000-0N-0	5 AXLE	LE TRAILER
HR S	0.0	0	000000000000000000000000000000000000000		LER
10/ 2/ 4/	0.0	0	000000000000000000000000000000000000000	5 OR LESS AXLE	MULT
480 = 39 = 39 =	0.0	0	000000000000000000000000000000000000000	6 AXLE	TRUCKS
105.1 10.3	0.0	0	000000000000000000000000000000000000000	7 OR MORE AXLE	LER
%%%	0.0	0*	000000000000000000000000000000000000000	OTHERS	
		480	1822257771131677828332222233 9822257777339	TOTAL VEHICLES	

COUNTY: FULTON

STATION: 24
DISTRICT: 1

ROUTE: KYS
MILEPOST: 0.100

ROAD DESIGNATION: R

LATEST ADT COUNT: 459 IN 2010

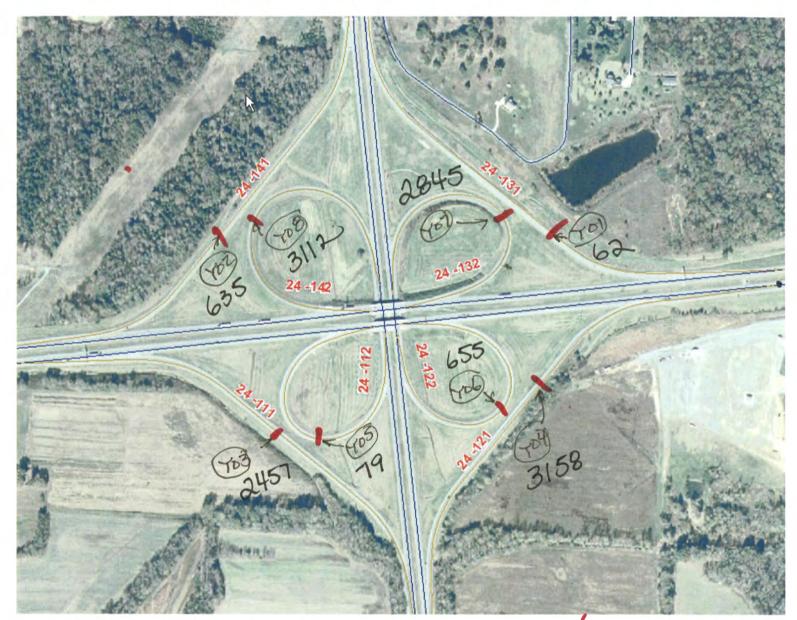
LOCATION INFORMATION: RAMP FROM KY 307 ONTO PURCHASE PARKWAY NB

STATION: 241 BOTH N-S ROUTE: KY9003R TYPE COUNT: AUTOMATIC FUNCTIONAL CLASS: 2

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF PLANNING-TRAFFIC SURVEY AND ANALYSIS SECTION **VEHICLE CLASSIFICATION COUNT**

2010

2010



Morshau Country I 24
I 24 & Julian Cardro U Pkwy
app. mpt 25.0
Ramps III | 141 | Yol-Yob
II2 | 142 |
I21 | 122 |
I31 |
I32

Y02		17 17 17 17 17 17 17 17 17 17 17 17 17 1	1784		OFF OFF JME LANES
STATION YOZ					CEN
		SUN SUN			POINT : SOURCE : COUNTED:
	28 2007	SAT SAT			MILE POINT COUNTED BY DATA SOURC ARRAY LANES COUN
MARSHALL COUNTY	TO OCTOBER 28 2007	FRI FRI		10: 635	TUESDAY
HALL	22	25 THU		RAFF	8 8
MARSI	WEEK OF OCTOBER	28500-01	28	AVERAGE DAILY TRAFFIC:	8- 9 AM 5- 6 PM
R9003	WEEK OF	27 27 27 27 27 27 27 27 27 27 27 27 27 2	891	AVERAG	BETWEEN
10024 RAMPaTR9003		Mos 258 258 258 258 258 258 258 258 258 258	865		TOR: 99 : 72 : 48 R: 90 R: 116
ROUTE: 10		DATE: DAY: 12-1 AM 2-3 AM 3-4 AM 3-4 S AM 6-7 AM 7-8 AM 10-11 AM 11-12 AM 11-12 AM 12-1 PM 12-1 PM 12-1 PM 12-1 PM 12-1 PM 12-1 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM	TOTALS:		MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR: PM HIGH HOUR:

6894 1574	JME E LANES
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DINT BY	SOURCE
	ARRAY ARRAY
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21	
TRA	NO
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SAGE	
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99	194
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
FA	HOUR:
1-2 AM 3-3 AM 4-5 4 AM 4-5 4 AM 6-7 AM 6-7 AM 8-9 AM 10-11 AM 11-12 AM 11-12 AM 11-2 PM 11-3 PM 11-4 P	TOTAL HO AM HIGH PM HIGH

PORTABLE TRAFFIC RECORDER REPORT

STATION Y04		1010 1001 1001 1001 1001 1001 1001 100	8863		01NT : 0.0 D BY : CEN OFF OURCE : VOLUME : TUBE
· ·	200	SUN SUN			MILE POINT : COUNTED BY : DATA SOURCE : ARRAY :
	28 2	SAT			COUNTE DATA S
NTY	TO OCTOBER 28 2007	FRI FRI		3158	MONDAY
000				: 10:	- W
ALL	22	THU THU		RAF	NO
MARSHALL COUNTY	OCTOBER	7000 44 2000 44 2000 44	319	AVERAGE DAILY TRAFFIC:	10-11 AM
R9003	WEEK OF	7 LUE 459 453 459 459 459 459 459 459 459 459 459 459	4410	AVERAG	BETWEEN
10024 RAMPATR9003		NON 22 200 200 200 200 200 200 200 200 200	4134		TOR: 99 : 72 : 48 R: 313
ROUTE: 10		DAYE: 12-1 AM 12-1 AM 5-2 AM 6-2 AM 6	TOTALS:		MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR:

ROUTE: 10024	10024 RAMPATR9003	TR9003		MARSHALL	ALL	3 MARSHALL COUNTY			STATION YOS	V05
		WEEK	OF.	OCTOBER		TO OCTOBER 28 2007	R 28	2007		
DATE: DAY: 12-1 AM 2-2 AM 3-4 AM 3-4 AM 4-5 AM 8-9 AM 11-12 AM 11-2 PM 11-2 PM 6-7 PM 7-6 PM 11-12 PM	28 - 20 20 20 20 20 20 20 20 20 20 20 20 20	TUS 000000000000000000000000000000000000		29g0 - x0 x x	7HD	PRI FRI	SAT SAT	SUN SUN		101 001 001 001 001 001 001 001 001 001
TOTALS:	96	116		12						224
		AVER	AGE	AVERAGE DAILY TRAFFIC:	RAFF		62			
MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR: PM HIGH HOUR:	8: 72 : 78 : 48 26	BETWEEN		10-11 AM 3- 4 PM	NO	MONDAY	LARBO	MILE POINT : COUNTED BY : DATA SOURCE : ARRAY : LANES COUNTED:	: CEN : VOLU : TUBE D: ALL	O OFF ME LANES

Y06		TOTALS 18 11 12 8	26484 24462 2462 2662 2462 266	1840		O OFF ME Lanes
STATION Y06						CEN VOLU TUBE
		SUN SUN				OINT :
	28 2007	27 SAT				MILE POINT COUNTED BY DATA SOURCE ARRAY LANES COUNTI
MARSHALL COUNTY	TO OCTOBER 28 2007	26 FR1			10: 655	MONDAY
ALL	22	25日			RAFF	NO
MARSH	OCTOBER	MED 13	00	33	AVERAGE DAILY TRAFFIC:	8- 9 AM 6- 7 PM
R9003	WEEK OF	TE 2	008524448485444848885 0088544484888	968	AVERAG	BETWEEN
10024 RAMPatr9003		22 MON	2824444444446894568 28341444468468914689	911		99 72 72 72 72 90
ROUTE: 1002		DATE: DAY: 12- 1 AM 1- 2 AM 2- 3 AM 3- 4 AM 4- 5 AM	5-65-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-	TOTALS:		MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS: AM HIGH HOUR: PM HIGH HOUR:

		WEEK OF	OF OCTOBER	22 1	TO OCTOBER 28 2007	28 2007		
ANAMAMAMAMAMAMAMAMAMAMAMAMAMAMAMAMAMAMA	MON 22 2244422 244444444444444444444444444	TUE 23 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2388 3388 4355 4355 4355 4355 4355 4355 4	THU THU	PR1	27 SAT 8	SUN SUN	TOTALS 33 33 34 22 34 37 48 48 48 48 48 48 48 48 48 48 48 48 48
PM PM	33	57						140
TOTALS: 4	6005	3836	139					7984
		AVER	AVERAGE DAILY TRAFFIC:	RAFFI	C: 2845			
MONTHLY FACTOR: AXLE FACTOR: TOTAL HOURS:	R: 99 : 72 : 48	1	0	i		MILE POINT COUNTED BY DATA SOURCE	T : 0.0	

KENTUCKY TRANSPORTATION CABINET-DEPARTMENT OF HIGHWAYS DIVISION OF TRANSPORTATION PLANNING

PORTABLE TRAFFIC RECORDER REPORT

Y08		101ALS 170 1104 1044 1104	8734	
STATION Y08				
		SUN SUN		
	28 2007	SAT SAT		
COUNTY	TO OCTOBER	26 FRI		
	22 T	Z5 THU		
MARSHALL	WEEK OF OCTOBER		435	The state of the s
	O.F			
al Ryous	WEEK	105 105 105 105 105 105 105 105	4451	
10024 KAMPalkyuus		MON 202 203 203 247 202 202 202 233 1126 1236 1238	3848	
KOOLE: 10		DATE: DAY: 12-1 AM 2-3 AM 3-4 AM 4-5 AM 6-7 AM 7-8 AM 8-9 AM 11-12 AM 11-12 AM 12-1 PM 12-1 PM 12-1 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM 11-12 PM	TOTALS:	

MILE POINT : 0.0
COUNTED BY : CEN OFF
DATA SOURCE : VOLUME
ARRAY : TUBE
LANES COUNTED: ALL LANES

MONTHLY FACTOR: 99
AXLE FACTOR: 72
TOTAL HOURS: 48
AM HIGH HOUR: 282 BETWEEN 11-12 AM ON TUESDAY
PM HIGH HOUR: 428 BETWEEN 5- 6 PM ON TUESDAY

Appendix F Highway Information System Summary of Parkway Data

- 1. Geometric Characteristics
- 2. Highway Systems
- 3. Bridge Characteristics
- 4. Horizontal and Vertical Curve Data
- 5. Interchange Characteristics
- 6. Weaving Analysis

Geometric Characteristics

Purchase Parkway and I-24 Fulton to Eddyville

COUNTY	BEGIN MP	END MP	LENGTH (MILES)	I OF I	LANE WIDTH (FT)	INSIDE SHOULDER WIDTH (FT)	OUTSIDE PAVED SHOULDER WIDTH (FT)		SPEED LIMIT	ROADWAY TYPE	PAVEMENT TYPE
Purchase	Parkway,	MP 0.00	00 to MP 5	1.398							
Fulton	0.000	0.625	0.6	4	12	3	10	Paved w/ Bituminous Material	55	Divided Highway	High Flexible
Fullon	0.625	3.434	2.8	4	12	3	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible
Hickman	3.434	8.352	4.9	4	12	3	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible
	8.352	21.887	13.5	4	12	3	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible
Graves	21.887	24.901	3.0	4	12	0	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible
	24.901	34.487	9.6	4	12	3	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible
Marshall	34.487	51.398	16.9	4	12	3	10	Paved w/ Bituminous Material	70	Divided Highway	Composite;Flexible over Rigid
Interstate	24, MP 2	4.941 to	MP 41.250)							
Marshall	24.941	29.352	4.4	4	12	ı	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible
Livingston	29.352	33.880	4.5	4	12	-	10	Paved w/ Bituminous Material	70	Divided Highway	Composite;Flexible over Rigid
Lyon	33.880	41.250	7.4	4	12	-	10	Paved w/ Bituminous Material	70	Divided Highway	High Flexible

Highway Systems

Julian M. Carroll (Purchase) Parkway and I-24 Fulton to Eddyville

COUNTY	BEGIN MP	END MP	STATE SYSTEM	NATIONAL TRUCK NETWORK	NATIONAL HIGHWAY SYSTEM	FUNCTIONAL CLASSIFICATION	TRUCK WEIGHT CLASS
Iulian M	Carroll (Pur	chase) Pa	l Irkway, MP 0.000 to MP 51.39		313121		CLASS
						T	
Fulton	0.000	3.430	State Primary (Parkway)	Yes	Yes	Rural Principal Arterial	AAA
Hickman	3.430	8.352	State Primary (Parkway)	Yes	Yes	Rural Principal Arterial	AAA
	8.350	21.305	State Primary (Parkway)	Yes	Yes	Rural Principal Arterial	AAA
Graves	21.305	24.747	State Primary (Parkway)	Yes	Yes	Urban Freeways & Expressways	AAA
	24.747	34.487	State Primary (Parkway)	Yes	Yes	Rural Principal Arterial	AAA
Marshall	34.487	51.398	State Primary (Parkway)	Yes	Yes	Rural Principal Arterial	AAA
Interstate	24, MP 24.9	41 to MP	41.250				
Marshall	24.941	29.352	State Primary (Interstate)	Yes	Yes	Rural Interstate	AAA
Livingston	29.352	33.880	State Primary (Interstate)	Yes	Yes	Rural Interstate	AAA
Lyon	33.880	41.25	State Primary (Interstate)	Yes	Yes	Rural Interstate	AAA

Bridge Characteristics

Julian M. Carroll (Purchase) Parkway and I-24 Fulton to Eddyville

Mainline Bridges - Julian M. (Purchase) Parkway WIDTH (GUTTER SUFFICIENCY COUNTY MP BRIDGE NO. DIRECTION FEATURES INTERSECTED Structural Function DESIGN LOAD INVENTORY RATING OPERATING RATING Load I (lbs) Load II (lbs) Load III (lbs) Load IV (lbs) (FT) TO GUTTER)(FT) Fulton 0.004 038B00053 SB Pkwy Over KY 116 (W. State Line St.) 153 38'-0' 96.6 HS 20-44 or Alt HS 33.3 128,000 134,000 150,000 204,000 Fulton 0.004 038B00053F NB Pkwy Over KY 116 (W. State Line St.) 152.9 38'-0' 99.6 HS 20-44 or Alt. HS 33.3 128,000 134,000 150,000 204,000 Fulton 0.924 038B00054L SB Pkwy Over KY 166 (Middle Rd) 142.25 38'-0" 99.7 HS 20-44 or Alt HS 31.7 122,000 130,000 142.000 200,000 NB 99.7 HS 31.7 Fulton 0.924 038B00054F Pkwv Over KY 166 (Middle Rd) 142.25 38'-0" HS 20-44 or Alt. 122.00 130,000 142,000 200,000 Fulton 1.781 038B00055L SB Pkwy Over I.C.R.R. (ILL. Central railroad) 539.00 30'-0" 81.6 HS 20-44 or Alt. HS 31.1 120,000 126,000 142,000 202,000 1.781 038B00055F NB 485.42 30'-0" 81.6 HS 20-44 or Alt. HS 31.1 120,00 126,000 Fulton Pkwy Over I.C.R.R. (ILL. Central railroad) 142,000 202,000 Graves 9.082 042B00170L SB Pkwy over Bayou Du Chien Creek 310.25 30'-0' 82.6 HS 20-44 or Alt. HS 36.1 134,000 140,000 154,000 208,000 Graves 9.082 042B00170F NB Pkwy over Bayou Du Chien Creek 310.25 30'-0" 82.6 HS 20-44 or Alt HS 36.1 134,000 140,000 154,000 208,000 Graves 12.788 042B00173L SB Pkwv over Bush Creek 127.17 38'-0" 98.7 HS 20-44 or Alt. HS 32.2 122.000 130.000 144.000 212,000 12.788 042B00173R NB 38'-0" 98.7 HS 20-44 or Alt. HS 32.2 130,000 144,000 Graves Pkwy over Bush Creek 127.17 122,000 212,000 16.751 042B00176L SB Pkwy over Obion Creek 208.25 38'-0' 98.6 HS 20-44 or Alt HS 34.4 138.000 144,000 156,000 208,000 Graves NB 38'-0" 99.6 138.00 Graves 16.751 042B00176B Pkwy over Obion Creek 208 25 HS 20-44 or Alt HS 34.4 144,000 156,000 208,000 Graves 17.777 042B00177L SB Pkwy over Opossum Creek 211.06 38'-0" 99.6 HS 20-44 or Alt. HS 34.4 152,000 162,000 174,000 232,000 Graves 17.777 042B00177R NB Pkwy over Opossum Creek 211.06 38'-0" 99.6 HS 20-44 or Alt HS 34.4 152,000 162,000 174,000 232,000 21.285 042B00154L SB Pkwy over US 45 (Mayfield Bypass) 208.31 24'-0" 92.6 HS 20-44 or Alt. HS 28.4 94,000 100,000 112,000 162,000 Graves NB Graves 21.285 042B00154R Pkwv over US 45 (Mayfield Bypass) 201.17 24'-0" 78.2 Functionally Obsolete HS 20-44 or Alt. HS 28.4 94.000 100,000 112.000 162,000 Graves 24.726 042B00155I SB Pkwy over US 45 (Paducah Rd) 238 00 38'-0" 97.4 HS 20-44 or Alt. HS 33.3 134.000 140.000 150,000 192,000 24.726 042B00155R NB Pkwy over US 45 (Paducah Rd) 238.00 38'-0" 97.6 HS 20-44 or Alt. HS 33.3 134,000 140,000 150,000 192,000 Graves 25.068 042B00156L SB Pkwy Over I.C.R.R. (ILL. Central railroad) 38'-0" 99.6 HS 20-44 or Alt. HS 35.6 144,000 150,000 162,000 208,000 Graves 172.17 Graves 25.068 042B00156F NB Pkwv Over I.C.R.R. (ILL. Central railroad) 172.17 38'-0" 99.6 HS 20-44 or Alt. HS 35.6 144.00 150.000 162,000 208,000 Graves 25.405 042B00157L SB Pkwv over Mavfield Creek 208.42 38'-0" 98.6 HS 20-44 or Alt HS 30.0 116.000 124.000 134,000 182,000 Graves 25.405 042B00157F NB Pkwv over Mavfield Creek 208.42 38'-0" 99.6 HS 20-44 or Alt. HS 30.0 116,000 124,000 134.000 182,000 25.637 042B00158L SB Pkwy over Mayfield Creek Overflow 1 97.17 38'-0" 96.6 HS 20-44 or Alt. HS 29.5 102,000 112,000 128,000 214,000 Graves 25.637 042B00158F NB Pkwy over Mayfield Creek Overflow 1 97 17 38'-0' 96.6 HS 20-44 or Alt HS 29.5 102.000 112,000 128,000 214.000 Graves Graves 25.863 042B00159L SB Pkwy over Mayfield Creek Overflow 2 97.17 38'-0" 99.6 HS 20-44 or Alt. HS 32.2 110,00 120,000 140,000 234,000 25.863 042B00159R NB Pkwy over Mayfield Creek Overflow 2 97.17 38'-0" 99.6 HS 20-44 or Alt. HS 32.2 110,000 120,000 140,000 234,000 Graves Graves 31.402 042B00162L SB Pkwy over Panther Creek 188.65 38'-0" 99.7 HS 20-44 or Alt. HS 30.0 128,000 136,000 148,000 208,000 31,402 042B00162R NB 38'-0" 99.7 HS 30.0 128.000 148.000 Graves Pkwv over Panther Creek 188.65 HS 20-44 or Alt. 136,000 208,000 Graves 31.573 042B00163L SB Pkwy over Panther Creek Overflow 97.17 38'-0" 99.7 HS 20-44 or Alt. HS 29.5 102.000 112,000 128,000 156,000 31.573 042B00163R NB Pkwy over Panther Creek Overflow 97.17 38'-0" 99.7 HS 20-44 or Alt. HS 29.5 102,000 112,000 156,000 Graves 128,000 33.524 042B00165L SB Pkwy over West Fork Clarks River Overflow 1 97.17 38'-0" 99.7 HS 20-44 or Alt HS 30.0 102,000 114,000 130,000 216,000 Graves NB 99.7 Graves 33.524 042B00165F Pkwv over West Fork Clarks River Overflow 1 97.17 38'-0' HS 20-44 or Alt HS 30.0 102.00 114.000 130.000 216,000 Graves 33.686 042B00166L SB Pkwy over West Fork Clarks River 208.42 38'-0" 99.7 HS 20-44 or Alt HS 30.0 132 00 138.000 150.000 204,000 HS 30.0 150,000 Graves 33.686 042B00166F NB Pkwy over West Fork Clarks River 208.42 38'-0" 99.7 HS 20-44 or Alt. 132,000 138,000 204,000 Graves 34.012 042B00167L SB Pkwy over West Fork Clarks River Overflow 2 107.68 38'-0" 96.7 HS 20-44 or Alt. HS 29.5 102,000 112,000 128,000 214,000 34.012 042B00167B NB 38'-0" 96.7 HS 20-44 or Alt. HS 29.5 102.000 112.000 214.000 Pkwv over West Fork Clarks River Overflow 2 107 68 128,000 Graves Graves 34.330 042B00168L SB Pkwv over KY 564 (Wayne Freeman Rd / Wadesboro 132.29 38'-0" 96.7 HS 20-44 or Alt. HS 31.7 120,000 130,000 142,000 210,000 34.330 042B00168R NB Pkwy over KY 564 (Wayne Freeman Rd / Wadesboro 38'-0" 97.7 HS 20-44 or Alt HS 31.7 120,000 130,000 210,000 Graves 132.29 142,000 SB 98.2 Marshall 42.748 079B00074L Pkwv over NC & St. Louis Railroads 157.70 38'-0" HS 20-44 (1961) HS 40.5 134.00 142,000 154.000 204,000 Marshall 42.748 079B00074R NB Pkwy over NC & St. Louis Railroads 157 70 38'-0" 98.2 HS 20-44 (1961) HS 40.5 134.000 142,000 154,000 204.000 43.277 079B00075L SB Pkwy over Clarks River Relief No. 1 81 HS 20-44 (1961) HS 26.6 122,000 130,000 158,000 Marshall 291.08 142,000 Marshall 43.277 079B00075R NB Pkwy over Clarks River Relief No. 1 291.08 30'-0" 81 HS 20-44 (1961) HS 26.6 122,000 130,000 142,000 158,000 SB HS 30.6 Marshall 43.614 079B00076L Pkwv over East Fork Clarks River 518.92 30'-0" 79.7 HS 20-44 (1961) HS 18.3 84.00 86,000 92.000 110,000 Marshall 43.614 079B00076R NR Pkwv over East Fork Clarks River 518.92 30'-0" 77 1 HS 20-44 (1961) HS 16.7 HS 30.0 90.000 96.000 102,000 132,000

Marshall 43.872 079B00064L

Marshall 43.872 079B00064B

SB

Pkwy over Clarks River Relief No. 2

Pkwy over Clarks River Relief No. 2

387.17

387.17

30'-0"

30'-0"

81

81

HS 20-44 (1961)

HS 20-44 (1961)

HS 33.9

HS 33.9

142,000

124,000

124.00

130,000

130.000

196,000

196,000

Bridge Characteristics (continued)
Julian M. Carroll (Purchase) Parkway and I-24
Fulton to Eddyville

Overpass Bridges - Julian M. Carroll (Purchase) Parkway

		_						MINIMUM H	ORZ CLEAR TO ABUT.		
COUNTY	MP	BRIDGE NO.	FEATURES INTERSECTED	NB MINIMUM VERTICAL CLEARANCE (FT)	SB MINIMUM VERTICAL CLEARANCE (FT)	SUFFICIENCY RATING	STRUCTURAL FUNCTION	CLEARANCE (FT)	MEASURED FROM OUTSIDE EDGE OF LANES (OEL) TO ABUTMENT FACE	DESIGN LOAD	INVENTORY RATING
Fulton	1.424	038B00012N	US 51 over Pkwy	16.45 (CL)	16.96 (CL)	89.1		36.0	both abuts	HS 20-44 or Alt.	HS 27.3
Fulton	2.442	038B00015N	KY 307 (Fulgham Rd) over Pkwy	16.38 (CL)	16.05 (CL)	96.6		36.0	SB abut	HS 20-44 or Alt.	HS 17.2
Hickman	4.146	053B00068N	KY 2569 (Holland Rd) over Pkwy	16.07 (IEL)	16.71 (CL)	98		35.0	SB abut	H20-44	HS 26.6
Hickman	5.122	053B00050N	KY 94 over Pkwy	16.38 (CL)	16.75 (CL)	95.8		36.0	both abuts	H20-44	HS 23.9
Hickman	6.533	053B00056N	KY 1529 over Pkwy	16.25 (OEL)	16.89 (IEL)	95.2		34.5	both abuts	H20-44	HS 18.9
Hickman	8.352	053B00102N	KY 1283 over Pkwy	16.16 (CL)	16.86 (CL)	97		35.0	both abuts	H20-44	HS 27.8
Graves	10.186	042B00171N	KY 1763 over Pkwy	17.04 (CL)	16.61 (CL)	79.3		36.0	both abuts	H 20 (inspect rpt)	HS 11.1
Graves	11.428	042B00172N	Grissom Rd over Pkwy	16.80 (CL)	16.37 (OEL)	78.4		35.5	NB abut	H20-44	HS 11.1
Graves	12.607	042B00180N	KY 944 over Pkwy	16.96 (CL)	16.38 (CL)	69.4		33.5	SB abut	H20-44	HS 8.3
Graves	13.653	042B00143N	KY 339/Relocated KY 58 over Pkwy	16.84 (CL)	16.33 (CL)	94.5		34.0	both abuts	H20-44	HS 18.9
Graves	15.302	042B00175N	Tater Rd over Pkwy	15.88 (OEL) 15.98 (OES)	16.74 (CL)	82.2		35.0	both abuts	H20-44	HS 11.7
Graves	16.526	042B00096N	KY 58 over Pkwy	16.68 (IEL)	15.94 (MOS) 16.13 (OEL)	91.3		36.0	both abuts	H20-44	HS 27.8
Graves	17.334	042B00128N	KY 1748 over Pkwy	16.34 (CL)	16.46 (CL)	62.6		35.0	SB abut	H20-44	HS 8.3
Graves	20.229	042B00153N	Cardinal Rd (Pryorsburg-Macedonia Rd) over Pkwy	16.66 (CL)	16.06 (OEL)	78.4		36.0	both abuts	H20-44	HS 11.1
Graves	22.267	042B00106N	KY 80 (Fancy Farm Rd) over Pkwy	15.30 (CL)	15.12 (IEL)	82.2		n/a	n/a	H20-S16	HS 33.9
Graves	23.701	042B00274N	KY 121 over Pkwy	17.34 (IEL)	16.77 (IEL)	98.5		30.1	both abuts	HS 25 or Alt.	HS 25.0
Graves	26.576	042B00160N	Hopewell Rd over Pkwy	16.61 (OEL)	17.68 (IEL)	76.3		35.5	both abuts	H20-44	HS 11.1
Graves	27.461	042B00009N	KY 131 over Pkwy	16.80 (IEL)	16.35 (OEL)	79.8		35.0	both abuts	H20-S16 or Alt.	HS 17.8
Graves	28.235	042B00161N	Twin Hill Rd (Spence Chapel Rd) over Pkwy	16.35 (CL)	16.29 (CL)	80.3		36.0	both abuts	H20-44	HS 11.1
Graves	31.129	042B00028N	KY 301 over Pkwy	16.22 (CL)	16.53 (CL)	94.2		33.0	both abuts	H20-44	HS 19.5
Graves	32.734	042B00164N	Panther Creek (School) Rd over Pkwy	16.24 (CL)	16.38 (CL)	79.4		35.0	NB abut	H20-44	HS 11.1
Marshall	36.197	079B00068N	KY 2603 / Vanzora Church Rd (Hale Springs Rd) over Pkwy	16.26 (CL)	16.54 (CL)	80.9		37.0	NB abut	H20-44	HS 13.3
Marshall	37.868	079B00071N	Bondurant Ln / KY 2604 (Marvin Jones Rd) over Pkwy	16.33 (CL)	16.40 (CL)	85.7		37.0	both abuts	H20-44	HS 13.3
Marshall	40.054	079B00073N	Jackson School Rd / KY 2606 (KY 299) over Pkwy	16.77 (CL)	16.42 (CL)	86		37.0	both abuts	H20-44	HS 20.6
Marshall	40.809	079B00126L	US 641 SB / Benton Bypass over Pkwy	16.98 (OEL)	17.45 (IEL)	94.5		> 30.0	NB abut	HS 20-44 or Alt.	HS 22.2
Marshall	40.809	079B00144R	US 641 NB / Benton Bypass over Pkwy	16.45 (OEL)	17.05 (IEL)	91.4		> 30.0	NB abut	HS 25 or Alt. Mil.	HS 38.6
Marshall	42.017	079B00103N	KY 408 / Oak Level Rd over Pkwy	16.89 (CL)	16.20 (CL)	71.4		36.0	both abuts	H20-44 (1961)	HS 11.1
Marshall	42.555	079B00102N	KY 348 / Symsonia Rd over Pkwy	16.88 (CL)	16.43 (CL)	72		33.0	both abuts	Inspect rpt has H20.	HS 11.1
Marshall	45.024	079B00012N	KY 795 / Scale Rd (Scale-Briensburg Rd) over Pkwy	16.38 (OEL)	16.88 (IEL)	97.8		35.0	SB abut	H20-44 (1961)	HS 26.6
Marshall	46.942		US 68 EB over Pkwy	16.29 (CL)	16.07 (CL)	66.2		36.0	both abuts	HS 20-44 (1961)	HS 27.8
Marshall Marshall	46.942 48.979	079B00001L 079B00050N	US 68 WB over Pkwy Palma Rd (Palma-Birmingham Rd Relocation) over	16.84 (CL) 16.14 (OEL)	16.61 (CL) 16.58 (CL)	66.2 86.4		36.0 36.0	both abuts NB abut	HS 20-44 (1961) H20-44 (1961)	HS 27.8 HS 26.1
			Pkwy	` ′	` ′					` ′	
Marshall	49.84 51.398 /	079B00066N	KY 2595 / Lakeview Church Rd over Pkwy	16.67 (CL)	16.32 (CL)	93.2 97		36.0	NB abut	H20-44 (1961)	HS 20.6
Marshall	24.941 51.398 /	079B00114R	I-24 EB over Pkwy	18.26 (IEL)	17.25 (CL)			25.0	both abuts	HS 20-44A or Alt.	HS 22.2
Marshall Overpass	24.941	079B00114L	I-24 WB over Pkwy	17.27 (CL)	16.27 (CL)	97		25.0	both abuts	HS 20-44A or Alt.	HS 22.2
Livingston	30.696	- 1-24 n/a	KY 453 over I 24	EB - 19.48 (EOS)	WB - 16.51 (OEL)	_	I -		_	_	-
Lyon	35.293	n/a n/a	KY 453 over I 24 KY 6008 (Hopewell Church Rd) over I 24	EB - 19.48 (EOS)	WB - 16.51 (OEL)	-	-	-		-	-
Lyon	36.413	n/a	KY 810 (Martins Chapel Rd) over I 24	EB - 16.27 (OEL)	WB - 17.46 (OEL)	-	-	-	<u>-</u>	-	-
Lyon	37.305	n/a	KY 6010 (Poplar Creek Rd) over I 24	EB - 16.46 (CL)	WB - 16.59 (OES)	-	-	-	- -	-	-
Lyon	40.744	n/a	KY 295 over I 24	EB - 16.66 (CL)	WB - 16.24 (CL)	_		<u> </u>	<u> </u>	_	-
Lyon	10.774	11/4	1(1 250 0VC1 124	_D 10.00 (OL)	10.27 (OL)				J	L	

Bridge Characteristics (continued)
Julian M. Carroll (Purchase) Parkway and I-24
Fulton to Eddyville

Culverts - Julian M.	Carroll (Purchase) F	Parkway						
COUNTY	CULVERT NO.	MP	SUFFICIENCY RATING	Structural Function	DESIGN LOAD	INVENTORY RATING	OPERATING RATING	LOCATION
Graves	042B00174N	14.151	83.1		HS 20-44 or Alt.	HS 20.0	-	Double 16' x 10' x 151' RCBC located at Cane Creek
Marshall	unknown	37.119	unknown		HS 20-44 or Alt.	unknown	-	Single 20' x 14' x 122' RCBC with KY 1949 / Wadesboro Rd N. running through it.
Marshall	079B00070N	37.135	68.9		HS20 or Alt. live load & 84 pcf earth dead load	HS 20.0	-	Double 14' x 10' x 163' RCBC located at Middle Fork Creek
Marshall	079B00072N	38.687	49.3		HS20-44 or Alt. live load & 84 pcf earth dead load	HS 11.1	HS 33.3	Double 12' x 10' x 238' located at Gibson Creek
Marshall	079B00065N	44.587	66.9		84 pcf earth DL (Inspect rpt has HS 20 for LL)	HS 61.1	-	Single 20' x 15' x 215' RCBC with Foust Sledd Rd (Old Benton Briensburg Rd) running through it. Should be 46' north of Clark River Relief Bridge 2 abutment.
Marshall	079B00067N	51.141	70		84 pcf earth DL (Inspect rpt has HS 20 for LL)	HS 20.0	-	Double 20' x 8' x 217' located at Little John Creek. Should be where Pkwy N. to I-24 E. ramp begins (exit 52A from Pkwy N.).

Horizontal and Vertical Curve Data

GRADE				Fulton to Calve		E HODIZONITAL CLIDVE				
			GR	ADE	VERTICAL LENGT	H OF CURVE	HORIZOI	NTAL CURVE	STOPPING SIGHT	DISTANCE
URBAN/RURAL	STATION	MP	IN %	OUT%	MINIMUM	ACTUAL (FT)	е	RADIUS 1	MINIMUM	ACTUAL
			,		CRITERIA (FT)	7.0.07.1		ACTUAL (FT)	CRITERIA (FT)	(FT)
Fulton County - I	MP 0.00									
RURAL	32+00	0.606	-0.50	2.46	536	800	-	-	730	1141
RURAL	48+00	0.909	2.46	-1.87	1071	1800	-	-	730	947
RURAL	53+98	1.022	-	-	ı	-	0.088	1910	-	-
RURAL	68+00	1.288	-1.87	0.53	435	1000	-	-	730	2149
RURAL	83+00	1.572	0.53	1.21	124	800	-	-	730	4211
RURAL	97+00	1.837	1.21	-1.43	654	1100	-	-	730	947
RURAL	119+50	2.257	-1.43	-0.55	159	600	-	-	730	2493
RURAL	127+00	2.399	-0.55	-1.00	111	600	-	-	730	2708
RURAL	145+00	2.740	-1.00	0.70	307	1000	-	-	730	2171
RURAL	168+18	3.179	-	-	-	-	0.024	6611	-	-
RURAL	180+00	3.403	0.70	0.50	49	700	-	-	730	5827
Hickman County	- MP 3.434					•				
RURAL	215+00	4.084	0.50	2.00	272	800	-	-	730	1975
RURAL	238+00	4.520	2.00	-1.00	741	1500	-	-	730	1039
RURAL	262+00	4.974	-1.00	1.10	379	1000	-	-	730	3616
RURAL	296+00	5.618	1.10	-1.75	703	1200	-	-	730	954
RURAL	324+00	6.149	-1.75	-0.50	226	800	-	-	730	2349
RURAL	337+09	6.397	-	-	ı	-	RC	22918	-	-
RURAL	370+00	6.980	-0.50	0.77	229	1000	-	-	730	2872
RURAL	401+00	7.567	0.77	-2.50	807	2000	-	-	730	1149
RURAL	417+00	7.870	-2.50	-0.50	362	800	-	-	730	4000
RURAL	424+58	8.014	-	-	-	-	0.028	5730	-	-
RURAL	434+00	8.192	-0.50	-1.81	325	1200	-	-	730	1421
Graves County -	MP 8.352									
RURAL	451+50	8.524	1.81	-0.50	572	800	-	-	730	866
RURAL	473+00	8.250	-0.50	0.50	181	500	-	-	730	1858
RURAL	483+50	9.130	0.50	-0.55	259	500	-	-	730	1281
RURAL	493+00	9.310	-0.55	0.52	192	500	-	-	730	1754
RURAL	507+00	9.575	0.52	3.00	449	1000	-	-	730	1971
RURAL	520+19	9.825	-	-	-	-	0.015	11459	-	-
RURAL	545+00	10.295	3.00	-1.00	987	2000	-	-	730	1039
RURAL	566+00	10.693	-1.00	3.00	724	1000	-	-	730	977
RURAL	581+00	10.977	3.00	-1.00	988	1800	-	-	730	985
RURAL	611+50	11.554	-1.00	1.68	486	800	-	-	730	1364
RURAL	633+00	11.962	1.68	-2.42	1015	1800	-	-	730	972
RURAL	672+50	12.710	-2.42	0.54	536	800	-	-	730	1141
RURAL	688+50	13.013	0.54	3.00	445	800	-	-	730	1668
RURAL	690+00	13.041	-	-	-	-	RC	11459	-	-
RURAL	695+00	13.136	3.00	-1.35	1075	1800	-	-	730	945
RURAL	713+00	13.696	-1.35	0.50	336	400	-	-	730	5488
RURAL	723+00	13.885	0.50	-2.61	767	1300	-	-	730	950
RURAL	734+00	14.094	-2.61	1.84	805	900	-	-	730	808
RURAL	743+19	14.268	-	-	-	-	RC	11459	-	-
RURAL	764+00	14.662	1.84	-1.30	776	1300	-	-	730	945
RURAL	780+00	14.965	-1.30	2.55	696	500	-	-	730	554
RURAL	794+00	15.230	2.55	-1.75	1062	1800	-	-	730	950
RURAL	810+00	15.533	-1.75	-0.50	227	400	-	-	730	1222
RURAL	821+00	15.742	-0.50	-1.06	138	400	-	-	730	2127
RURAL	843+00	16.158	-1.06	-2.40	332	400	-	-	730	1003
RURAL	862+00	16.518	-2.40	0.45	517	750	-	-	730	1151
RURAL	867+62	16.598	-	-	=	-	RC	11459	-	-
RURAL	875+75	16.752	0.45	-0.69	282	400	-	-	730	1146
RURAL	883+00	16.889	-0.69	0.00	125	400	-	-	730	2137
RURAL	897+50	17.164	0.00	0.74	134	400	-	-	730	2005
RURAL	910+50	17.410	0.74	0.00	182	400	-	-	730	1662

Horizontal and Vertical Curve Data (continued)

			GR	ADE	VERTICAL LENGT	•	HORIZOI	NTAL CURVE	STOPPING SIGHT	DISTANCE
URBAN/RURAL	STATION	MP			MINIMUM			RADIUS 1	MINIMUM	ACTUAL
			IN %	OUT%	CRITERIA (FT)	ACTUAL (FT)	е	ACTUAL (FT)	CRITERIA (FT)	(FT)
RURAL	922+00	17.628	0.00	0.70	127	400	-	-	730	2108
RURAL	929+25	17.765	0.70	-0.70	346	600	-	-	730	1071
RURAL	936+50	17.903	-0.70	0.92	293	400	-	-	730	967
RURAL	949+50	18.149	0.92	3.00	377	400	-	-	730	1862
RURAL	966+50	18.471	3.00	-1.97	1227	2100	-	-	730	955
RURAL	980+00	18.727	-1.97	1.48	624	600	-	-	730	727
RURAL	1000+00	19.105	1.48	2.20	131	400	-	-	730	2050
RURAL	1018+00	19.446	2.20	-2.32	1115	1800	-	-	730	927
RURAL	1030+75	19.688	-2.32	1.74	733	750	-	-	730	747
RURAL	1053+00	20.109	1.74	-0.50	553	900	-	-	730	932
RURAL	1062+00	20.280	-0.50	0.74	224	400	-	-	730	1237
RURAL	1075+00	20.526	0.74	-2.14	711	1200	-	-	730	949
URBAN	1081+25	20.606	ı	-	-	-	0.037	4297.19	-	-
URBAN	1088+00	20.734	-2.14	-0.53	155	400	-	-	425	969
URBAN	1106+00	21.075	-0.53	3.84	419	500	-	-	425	494
URBAN	1115+00	21.245	3.84	-3.87	648	1300	-	-	425	603
URBAN	1126+50	21.463	-3.87	1.07	475	1000	-	-	425	808
URBAN	1132+93	21.585	-	-	-	-	0.06	1145.92	-	-
URBAN	1143+00	21.775	1.07	-0.17	104	300	-	-	425	1021
URBAN	1143+93	21.793	-	-	-	-	0.06	1145.92	-	-
URBAN	86+00	22.245	0.80	-0.85	139	600	-	-	425	954
URBAN	96+00	22.434	-0.85	0.76	155	400	-	-	425	972
URBAN	97+00	22.453	-	-	-	-	-	-	-	-
URBAN	97+00	22.453	-	-	-	-	-	-	-	-
URBAN	105+50	22.645	0.76	-1.89	223	800	-	-	425	806
URBAN	117+97	22.881	-	-	-	-	0.083	1909.86	-	-
URBAN	118+50	22.891	-1.89	-1.25	62	400	-	-	425	2279
URBAN	136+00	23.223	-1.25	2.02	313	500	-	-	425	671
URBAN	144+00	23.374	2.02	0.50	127	600	-	-	425	1011
URBAN	151+52	23.517	0.50	-1.50	168	900	-	-	425	989
URBAN	164+00	23.753	-1.50	0.87	227	400	-	-	425	1089
URBAN	183+50	24.122	0.87	-0.51	115	400	-	-	425	985
URBAN	200+50	24.444	-0.51	-2.47	165	800	-	-	425	951
URBAN	207+00	24.567	-2.47	0.50	285	500	-	-	425	774
URBAN	216+00	24.738	0.50	-1.99	209	1200	-	-	425	1019
URBAN	224+50	24.890	-1.99	1.12	299	450	-	-	425	661
URBAN	225+47	24.917	-	-	-	-	0.083	1909.86	-	-
URBAN	235+00	25.089	1.12	-2.93	340	1650	-	-	425	938
RURAL	1231+48	25.320	-2.93	0.30	584	536	-	-	730	721
RURAL	1236+16		0.30	-0.30	148	400	-	-	730	1998
RURAL	1243+00	25.538	-0.30	0.00	54	400	-	-	730	4778
RURAL	1268+50	26.021	0.00	2.20	398	400	-	-	730	1422
RURAL	1282+00	26.277	2.20	0.80	346	600	-	-	730	1071
RURAL	1295+63	26.535	-	-	-	-	0.0156	11459	-	-
RURAL	1297+50		0.80	2.20	253	500	-	-	730	1355
RURAL	1313+50	26.873	2.20	-0.50	667	1300	-	-	730	1019
RURAL	1338+00	27.337	-0.50	-2.00	371	600	-	-	730	1019
RURAL	1347+50		-2.00	1.00	543	500	-	-	730	760
RURAL	1354+52	27.650	-	-	-	-	0.0156	11459		-
RURAL	1367+00	27.887	1.00	0.70	74	400	-	-	730	3797
RURAL	1386+00	28.247	0.70	-0.92	400	800	-	-	730	1066
RURAL	1406+00	28.625	-0.92	1.50	438	400	-	-	730	1021
RURAL	1416+50	28.824	1.50	-1.00	618	1100	-	-	730	974
RURAL	1432+50	29.127	-1.00	1.00	362	400	-	-	730	2400
RURAL	1442+00	29.307	1.00	-0.50	371	800	-	-	730	1119

Horizontal and Vertical Curve Data (continued)

			GR	ADE	VERTICAL LENGT	H OF CURVE	HORIZO	NTAL CURVE	STOPPING SIGHT	DISTANCE
URBAN/RURAL	STATION	MP	IN %	OUT %	MINIMUM	ACTUAL (FT)		RADIUS 1	MINIMUM	ACTUAL
			IIN 76	001%	CRITERIA (FT)	ACTUAL (FT)	е	ACTUAL (FT)	CRITERIA (FT)	(FT)
RURAL	1450+48	29.468	-	-	-	-	0.0156	11459	-	-
RURAL	1466+00	29.762	-0.50	-1.70	296	600	-	-	730	1199
RURAL	1477+00	29.970	-1.70	0.60	416	400	-	-	730	1200
RURAL	1489+00	30.197	0.60	-2.20	692	1200	-	-	730	962
RURAL	1507+79	30.553	-	-	-	-	0.0156	11459	-	-
RURAL	1508+00	30.557	-2.20	-0.75	262	400	-	-	730	1071
RURAL	1522+00	30.822	-0.75	-2.36	397	800	-	-	730	1072
RURAL	1539+00	31.144	-2.36	0.22	467	400	-	-	730	863
RURAL	1555+00	31.447	0.22	-0.36	143	400	-	-	730	2070
RURAL	1565+50	31.646	-0.36	3.00	608	600	-	-	730	751
RURAL	1572+57	31.780	- 2.00	- 0.52	-	-	0.02	7639	- 722	-
RURAL	1580+00	31.921	3.00	0.52	613	1000	-	-	730	933
RURAL	1597+00	32.243	0.52	1.34	148	400	-	-	730	1819
RURAL	1606+00	32.413	1.34	-1.32	657	1100	-	-	730	945
RURAL	1644+00 1650+35	33.132 33.252	-1.32	-2.67	334	600	0.02	7639	730	1098
RURAL	1654+67	33.391	-2.67	0.29	536	800	- 0.02	- 7039	730	1142
RURAL	1664+50	33.520	0.29	-0.30	146	400	-	-	730	2024
RURAL	1670+00	33.624	-0.30	0.30	109	400	_	-	730	2443
RURAL	1675+90	33.736	0.30	-0.30	148	400	_	_	730	1998
RURAL	1683+50	33.880	-0.30	0.30	109	400	_	_	730	2443
RURAL	1690+50	34.013	0.30	-0.30	148	400	_	-	730	1998
RURAL	1699+89	34.190	-0.30	3.00	597	600	-	-	730	768
Marshall County							!			
RURAL	1724+50		3.00	0.70	568	1000	-	-	730	969
RURAL	1727+02	34.661	-	-	-	-	0.028	5730	-	-
RURAL	1745+00	35.002	0.70	1.05	64	400	-	-	730	4086
RURAL	1768+00	35.438	1.05	-0.50	383	800	-	-	730	1095
RURAL	1815+00	36.328	-0.50	-0.80	73	400	-	-	730	3833
RURAL	1855+00	37.085	-0.80	-1.36	139	400	-	-	730	2123
RURAL	1866+00	37.294	-1.36	0.50	336	400	-	-	730	5293
RURAL	1897+00	37.881	0.50	2.29	324	400	-	-	730	14674
RURAL	1912+50	38.174	2.29	-1.52	940	1700	-	-	730	982
RURAL	1929+05	38.488	-	-	-	-	0.0156	11459	-	-
RURAL	1942+00	38.733	-1.52	2.42	712	800	-	-	730	812
RURAL	1971+50	39.292	2.42	-0.52	726	2100	-	-	730	1242
RURAL	1977+39	39.403	-	-	-	-	0.028	5730	-	-
RURAL	1997+02		-0.52	0.50	185	1000	-	-	730	3542
RURAL	2011+00	40.247	0.50	1.50	181	600	-	-	730	2209
RURAL	2023+00	40.474	1.50	-0.50	494	1600	-	-	730	1314
RURAL	2035+00	40.701	-0.50	0.50	181	600	-	-	730	2209
RURAL	2053+00	41.042	0.50	-0.50	247 -	2200	- 0.025	- 4504	730	2179
RURAL	2071+79 2094+00	41.398	-0.50	-2 00		1000	0.035	4584	720	929
RURAL RURAL	2094+00	41.819 42.179	-0.50 -3.00	-3.00 -1.86	618 206	1000 400	-	-	730 730	1333
RURAL	2113+00	42.179	-3.00	-1.80	-	-	0.028	5730	- 730	1333
RURAL	2135+00	42.431	-1.86	1.50	608	800	0.028	-	730	959
RURAL	2148+00	42.841	1.50	-3.00	1112	ILLEGIBLE	_	_	730	ILLEGIBLE
RURAL	2161+75	43.102	-3.00	0.00	543	600	-	-	730	880
RURAL	2208+25	43.983	0.00	3.00	543	600	-	-	730	880
RURAL	2232+00	44.432	3.00	1.10	469	800	-	-	730	968
RURAL	2256+50	44.896	-	-	-	-	0.028	5730	-	-
RURAL	2257+00	44.906	1.10	-0.68	440	1000	-	-	730	1106
RURAL	2269+00	45.133	-0.68	0.80	268	800	-	-	730	2000
RURAL	2293+00	45.587	0.80	-2.40	790	2000	-	-	730	1161

Horizontal and Vertical Curve Data (continued)

Julian M. Carroll (Purchase) Parkway
Fulton to Calvert City

			GRA	ADE	VERTICAL LENGT	H OF CURVE	HORIZON	NTAL CURVE	STOPPING SIGHT	DISTANCE
URBAN/RURAL	STATION	MP	IN %	OUT%	MINIMUM CRITERIA (FT)	ACTUAL (FT)	е	RADIUS ¹ ACTUAL (FT)	MINIMUM CRITERIA (FT)	ACTUAL (FT)
RURAL	2322+00	46.137	-2.40	2.20	833	1000	-	-	730	862
RURAL	2337+34	46.427	ı	-	-	-	0.055	2865	-	-
RURAL	2340+00	46.478	2.20	0.90	321	800	-	1	730	1230
RURAL	2355+00	46.762	0.90	-0.80	420	800	-	1	730	1035
RURAL	2389+61	47.417	ı	-	1	-	0.083	1910	-	-
RURAL	2415+00	47.898	-0.80	2.18	539	600	-	-	730	889
RURAL	2433+00	48.239	2.18	0.50	415	800	-	-	730	1042
RURAL	2447+00	48.504	0.50	-1.60	519	900	-	-	730	964
RURAL	2450+72	48.575	-	-	-	-	0.028	5730	-	-
RURAL	2470+00	48.940	-1.60	2.82	800	800	-	-	730	732
RURAL	2485+00	49.224	2.82	-1.60	1092	1800	-	-	730	937
RURAL	2501+00	49.527	-1.60	1.60	579	800	-	1	730	1021
RURAL	2515+00	49.792	1.60	-1.00	642	1200	-	-	730	998
RURAL	2544+00	50.309	-1.00	-3.00	494	800	-	-	730	940
RURAL	2558+51	50.584	-	-	-	-	ILLEGIBLE	11459	-	-
RURAL	2568+53	50.774	-3.00	0.00	543	800	-	-	730	1120
RURAL	2597+00	51.313	0.00	0.70	127	600	-	-	730	3110

Notes: 1) The minimum horizontal radius of the curve is 1810 feet for rural areas and 758 feet for urban areas.

Interchange Ramp Data
Julian M. Carroll (Purchase) Parkway and I-24
Fulton to Eddyville

									SHOU	JLDER				ENTRANCE/		PARALLEL	TAPER		
COUNTY	MP	PLAN YEAR	INTERSECTING ROUTE	EXIT NUMBER	ENTRANCE	EXIT	TYPE	LANE WIDTH	LT	RT	ROLLED CURB	DIVERGENCE ANGLE ¹	ENTRANCE/ EXIT RADIUS (FEET)	EXIT RADIUS DESIGN SPEED (MPH) ²	RAMP/ LOOP RADIUS (FEET)	LENGTH MEASURED (FEET)	LENGTH MEASURED (FEET)	TAPER LENGTH (FEET)	SUPER- ELEVATION
Interchange	es - Julian	M. Carro	ll (Purchase) Park	way															
						A (RAMP "A")	Taper	18	6'-11"			4° 19' 19"	881	50			354	459	0.95
FULTON	0.300	1966	FRONTAGE	0	B (RAMP "B")		Taper	18	6'-11"	_		-	716	50			409	900	0.1
I OLION	0.500	1300	ROADS A & B	"		C (Ramp "C")	Taper	18	6'-11"	_	Υ	2° 56' 5"	955	50			367	561	0.093
					D (Ramp "D")		Parallel	18	_	8'-11"	Υ	-	881	50		228	366	900	0.095
						A (Ramp "SW")	Taper	18		8'-11"	Υ	3° 13' 36"	1206	50			246	400	0.08
FULTON	1.424	1966	US 51	1	B (Ramp "SE")		Parallel	18	6'-11"	_	Υ	-	764	49		277	287	900	0.098
I OLION	1.727	1000	0001	l '		C (Ramp "NE")	Taper	18	6'-11"			4° 10' 52"	1432	49			370	459	0.07
					D (Ramp "NW")		Taper	18		8'-11"	Y	-	2122	51	1907		370	900	0.054
						A (Ramp "B")	Taper	18	6'-11"			6° 26' 53"	1138				165	459	Unknown
FULTON	2.478	1966	KY 307	2	B (RAMP "D")		Parallel	18		8'-11"	Y	-	9291	63	3626	202	610	900	0.027
						C (Ramp "C")	Taper	18	6'-11"		Y	5° 50' 59"	2190	61	1432		184	480	0.07
					D (RAMP "A")		Parallel	18	6'-11"		Υ	-	1637	49		358	584	900	0.063
						A (Ramp "D")	Loop	18	6'-11"		Υ	6° 38' 12"	150		2083/251		N/A		Variable
Graves	13.653	?	KY339	14	B (Ramp "C")		Loop	18	_	8'-11"	Y	-	150		1557/130		N/A		Variable
						C (Ramp "A")	Loop	18	6'-11"			9° 54' 42"	150		1432/237		N/A		Variable
					D (RAMP "B")		Loop	18		8'-11"	Υ	-	150		477/235		N/A		Variable
						A (RAMP "D")	Taper	18	6'-11"			4° 0' 44"			500		845	459	0.097
Graves	21.285	1966	US 45	21	C (RAMP "C")		Taper	18		8'-11"	Y	-			1146		190	459	0.083
					D (RAMP "B")		Loop	18		8'-11"	Υ	-			294		250	459	0.1
						A (RAMP 1)	N/A	16	6'	6'	N	-	N/A				N/A		Unknown
Graves	22.267	1961	KY 80	22	B (RAMP 4)		Taper	16	6'	6'	N	-	716				330	400 ³	Unknown
J.a.ioo	LL.LO.		11. 00			C (RAMP 3)	Taper	16	6'	6'	N	3° 7' 40"	716				315	480 ³	Unknown
					D (RAMP 2)		Taper	16	6'	6'	N	-	716				330	480 ³	Unknown
						A (RAMP "A")	Taper	18	6'	6'	N	4° 12' 27"	984	50 ⁴			215	520 ³	0.076
					B (RAMP "C")	,	Parallel	18	6'	6'	N	_	984	50 ⁴	984	925	415	1050 ³	0.076
Graves	23.701	2000	KY 121	24	2 ()	C (RAMP "D")	Taper	18	6'	6'	N	3° 39' 21"	984	50 ⁴	30.	323	275	520 ³	0.076
					D (DAME IIDII)	C (KAWE D)			6'	6'	N	3 39 21	984	50 ⁴	004		1250	1050 ³	0.076
				1	D (RAMP "B")	A (DAMP IICII)	Taper	18 18	6'-11"			4° 35' 6"	TAPER	50	984		200	459	
						A (RAMP "C")	Taper						t		716				Variable
Graves	24.726	1966	US 45	25	B (RAMP "D")		Parallel	18	6'-11"			-	714	51		400	400	940 3	0.1
						C (RAMP "B")	Taper	18	6'-11"			-	Tangent				245	450 ³	-
					D (RAMP "A")		Taper	18	6'-11"	8'-11"	Υ	-	725	51			585	900 ³	0.1
						A (Ramp "SW")	Taper	15	6'	8'	N	3° 51' 39"	764	32			206	450 ³	0.06
Graves	27.461	1966/	KY 131	27	B (Ramp "SE")		Parallel	18	6'-11"	8'-11"	Υ		716	51		375	656	900	0.1
Ciavos	27.401	1978(?)	101			C (Ramp "NE")	Taper	18	6'-11"	8'-11"	Υ	-	2865	47			294	458	0.036
					D (Ramp "NW")		Parallel	15	6'	8'	N	-	758	32		393	478	900	0.06

Interchange Ramp Data (continued)
Julian M. Carroll (Purchase) Parkway and I-24
Fulton to Eddyville

COUNTY	MP	PLAN YEAR	INTERSECTING ROUTE	EXIT NUMBER	ENTRANCE	EXIT	TYPE	LANE WIDTH	SHOU	ILDER RT	ROLLED CURB	DIVERGENCE ANGLE 1	ENTRANCE/ EXIT RADIUS (FEET)	ENTRANCE/ EXIT RADIUS DESIGN SPEED (MPH)	RAMP/ LOOP RADIUS (FEET)	PARALLEL LENGTH MEASURED (FEET)	TAPER LENGTH MEASURED (FEET)	TAPER LENGTH (FEET)	SUPER- ELEVATION
						A (Ramp "B")	Taper	15	6'	10'	N	3° 11' 42"	759	52			233	530 ³	0.1
Marshall	40.809	1998	US 641	41	B (Ramp "A")		Parallel	18			N	-	819	50		243	811	675/300	0.098
Maisilaii	40.000	1330	00 041	7.		C (Ramp "D")	Taper	18			N	6° 27' 11"	764	50	637		230	440	0.099/0.1
					D (Loop "D")		Loop	15	6'	10'	N	-	4530		549/549/4523	360	940	900/295	?/?/0.1/?
						A (Ramp "D")	Loop	18	6'-11"	8'-11"	Υ	3° 57' 15"	135		230		0		Unknown
Marshall	42.555	1966	KY 348	43	B (RAMP "B")		Loop	18	6'-11"	8'-11"	Υ	-	135		230		0		Unknown
warsnan	42.000	1300	1010	40		C (Ramp "A")	Loop	18	_	8'-11"	Y	3° 49' 12"	135		230		0		Unknown
					D (Ramp "C")		Loop	18	_	8'-11"	Y	-	135		230		0		Unknown
						A (Ramp "B")	Taper	18	6'-11"	8'-11"	Υ	4° 49' 19"	1910	74			246	459	0.1
Marshall	46.942	1966	US 68	47	B (RAMP "D")		Parallel	18			N			no drawing	found	322	502	no	o dwg
maronan	10.012	.000	00 00			C (Ramp "C")	Taper	18			N	4° 54' 15"					236		, u., g
					D (RAMP "A")		Parallel	18		8'-11"	Y		955	57		200	960	900	0.1
						A1 (Ramp "C")	Taper	18	6'-11"		Y		716	51	716		244	459	0.1
						A2 (Ramp "H")	Loop	18	6'-11"	8'-11"	Υ		716	51	358/230/358/716/1432		0		0.1
					B1 (Ramp "G")		Parallel	18		8'-11"	Υ		716	51	716	400	570		0.1
Marshall	51.398	1966	I-24	52	B2 (Ramp "F")		Loop	18		8'-11"	Y		716	51	1432/358/230/358		0		0.1
Marorian	01.000	.000		02		C1 (Ramp "E")	Taper	18		8'-11"	Υ		716	51	716		343	645	0.1
						C2 (Ramp "B")	Loop	18	_	8'-11"	Y		716	51	356/422/358/716		0		0.1
					D1 (Ramp "A")		Parallel	18		8'-11"	Y		716	51	716	355	695	900	0.1
					D2 (Ramp "D")		Loop	18	6'-11"	8'-11"	Y		716	51	358/230/358/716		0		0.1
Interchange	s - I-24		1			1		1					T	ı		ı			1
						A1	Taper										240		
						A2	Loop	4									0		
					B1		Parallel									390	420		
Marshall	-	-		25	B2		Loop	-	-	-	-	-	-		-		0		-
						C1	Taper										240		
						C2	Loop	_									0		
					D1		Parallel	_								325	590		
					D2		Loop										0		
						Α	Taper										240		
Marshall	-	-		27	В		Parallel	-	-	-	-	-	-		-	270	520		-
					-	С	Taper	-								255	200		
					D		Parallel									365	505		
						Α	Taper Parallel	-								220	260 510		
Livingston	-	-		31	В	С		-	-	-	-	-	-		-	220	290		-
					-	C	Taper	-								245			
					D		Parallel									245	775		
					D	Α	Taper Parallel	-								205	245 490		
Lyon	-	-		40	В	С		-	-	-	-	-	-		-	285			-
					-	L L	Taper	-								450	215		
			1		D		Parallel	-				-	-			450	415	-	
					D.	Α	Taper	-								200	240		
Lyon	-	-		42	В	С	Parallel	-	-	-	-	-	-		-	260	550 240		-
					<u> </u>	L L	Taper	1	1							210			
					D		Parallel									310	330		

¹ Divergence angle not provided in as built plans. Measurement from aerial drawing.

² Design Speed calculated from current AASHTO 10% max superlevation table.

³ Measured from as built plans.

⁴ Design Speed calculated from current AASHTO 8% max superlevation table.

Interchange Ramp Data
Julian M. Carroll (Purchase) Parkway
Fulton to Calvert City

		1		F	ulton to Ca	alvert City	/				
	EXIT					GF	RADE	OF C	L LENGTH URVE	STOPPII SIGHT DIST	
INTERCHANGE	NUMBER	MP	DWG RAMP	TYPE	SPEED ¹	IN %	OUT %	MINIMUM CRITERIA (FT) ¹	ACTUAL (FT)	MINIMUM CRITERIA (FT) ¹	ACTUAL (FT)
Fulton County											
			RAMP A	RAMP	35	-0.80	-0.09	35	200	250	1092
			RAMP B	RAMP	35	-0.09	0.38	23	200	250	1590
Frontage Road A;			RAMP C	RAMP	35	-2.88	-1.46	70	300	250	840
Frontage Road B	0	0.300	IVAIVIF C	IXAIVII	35	-1.46	0.69	105	900	250	2931
1 Tomago Hoda B					35	-1.40	0.53	94	200	250	2243
			RAMP D	RAMP	35	0.53	-0.50	30	200	250	1153
					35	-0.50	0.50	49	200	250	800
			RAMP NW	RAMP	35	0.70	-3.30	116	200	250	370
US 51					35	-3.30	2.36	277	300	250	265
(Profile information			RAMP NE	RAMP	35	-1.01	2.08	151	150	250	323
almost entirely	1	1.424			35	2.08	-2.43	131	200	250	339
illegible. Values represent best			RAMP SW	RAMP	35	-0.29	2.28	126	150	250	481
guesses.)					35	2.28	-1.54	111	200	250	382
gaddada			RAMP SE	RAMP	35	2.00	-3.61	163	300	250	342
					35	-3.61	1.63	257	250	250	245
			RAMP A	RAMP	35	2.65	-1.42	118	200	250	365
					35	-1.42	1.80	158	200	250	355
			RAMP B	RAMP	35	-1.00	2.52	172	200	250	312
KY 307	2	2.478			35	2.52	-0.75	95	200	250	430
			RAMP C	RAMP	35	Varies	4.48	-	150	250	-
					35	4.48	-4.05	247	400	250	318
			RAMP D	RAMP	35	0.55	-4.57	148	300	250	361
Crove a County					35	-4.57	-0.86	182	300	250	386
Graves County					25	Illegible	1.84	_			
			RAMP A	LOOP	25	1.84	-1.00	34	200	155	480
					25	-5.35	0.60	155	300	155	255
			RAMP B	LOOP	25	0.60	2.62	53	200	155	1489
KY 339	14	13.65			25	1.00	-2.04	36	200	155	455
			RAMP C	LOOP	25	-2.04	-0.51	40	200	155	553
					25	-1.04	2.66	96	300	155	387
			RAMP D	LOOP	25	2.66	1.00	20	200	155	750
					35	0.50	4.00	172	200	250	314
			RAMP 1	RAMP	35	4.00	-2.30	183	150	250	246
					35	2.82	-4.00	198	200	250	258
			RAMP 2	RAMP	35	-4.00	-0.50	172	200	250	314
KY 80	22	22.267			35	0.50	3.60	152	200	250	378
			RAMP 3	RAMP	35	3.60	-2.56	179	200	250	275
					35	3.13	-3.40	189	200	250	265
			RAMP 4	RAMP	35	-3.40	-0.50	142	200	250	426
			DAME A	DA115	35	-1.00	1.76	135	328	250	645
			RAMP A	RAMP	35	1.76	0.49	37	262	250	975
KY 121	24	24.726	RAMP B	RAMP	35	-1.45	0.55	98	328	250	2095
			RAMP C	RAMP	35	-2.17	2.00	204	426	250	450
			RAMP D	RAMP	35	1.54	0.49	31	196	250	1118
			DAME 1	DAME	35	0.57	3.66	152	250	250	437
			RAMP A	RAMP	35	3.66	0.63	88	300	250	506
			DAME	DAME	35	0.22	-4.35	132	350	250	411
110.45	0.5	25 000	RAMP B	RAMP	35	-4.35	-1.44	143	200	250	422
US 45	25	25.068	DAMP C	DAME	35	-2.55	-3.63	31	200	250	1100
			RAMP C	RAMP	35	-3.63	0.82	218	300	250	321
			DAMES	DAME	35	-1.79	2.33	202	300	250	346
			RAMP D	RAMP	35	2.33	-0.78	90	300	250	497

Interchange Ramp Data (continued)

	EXIT					GF	RADE		L LENGTH URVE	STOPPING SIGHT DISTANCE										
INTERCHANGE	NUMBER	MP	DWG RAMP	TYPE	SPEED ¹	IN %	OUT %	MINIMUM CRITERIA (FT) ¹	ACTUAL (FT)	MINIMUM CRITERIA (FT) ¹	ACTUAL (FT)									
			RAMP NE	RAMP	35	-1.32	3.69	245	350	250	330									
					35	3.69	0.50	92	300	250	489									
			RAMP SE	RAMP	35 35	-0.25 -3.68	-3.68 1.32	99 245	300 400	250 250	465 367									
KY 131 27	27.461	55		35	-0.91	0.30	59	300	250	972										
			RAMP NW	RAMP	35	0.30	0.60	15	200	250	2443									
			RAMP SW	RAMP	35	-1.30	2.26	174	350	250	455									
					35	2.26	0.80	42	300	250	889									
Marshall County			ı	Г	25	1.61	2 27	144	650	250	F21									
			RAMP A	RAMP	35 35	1.61 -3.37	-3.37 0.50	190	650 400	250 250	531 459									
					35	0.33	3.70	165	400	250	539									
			RAMP B	RAMP	35	3.70	-2.06	167	800	250	547									
					35	-2.06	-0.27	88	300	250	10428									
US 641	41	40.809		.	35	0.22	-2.66	84	400	250	574									
			RAMP D	RAMP	35	-2.66	6.00	425	1000	250	497									
					35 25	6.00	2.61	98 44	300 280	250 155	468 437									
			LOOP D	LOOP	25	-2.37 -6.00	-6.00 1.95	207	800	155	443									
			200. 2	200.	25	1.95	-0.24	26	350	155	668									
		55		25	0.32	2.21	49	200	155	2779										
KY 348 43		RAMP A	LOOP	25	2.21	1.56	8	200	155	1760										
			RAMP C	LOOP	25	-1.55	-2.99	17	200	155	849									
	43	42.555	10 1011	2001	25	-2.99	1.18	108	200	155	255									
		RAMP B	LOOP	25	-1.56	-3.66	25	200	155	614										
					25 25	-3.66 -1.18	-0.54 4.93	81 159	200	155 155	374 185									
			RAMP D	LOOP	25 25	4.93	1.56	40	200	155	420									
			RAMP A	RAMP	35	-1.89	1.29	156	200	250	362									
			RAMP B	RAMP	35	-0.40	2.03	119	200	250	648									
US 68	47	46.942	RAIVIP B	KAIVIP	35	2.03	1.56	14	200	250	2376									
			RAMP C	RAMP	35			No drawing found												
			RAMP D	RAMP	35															
			RAMP A	RAMP	35 35	-0.84 0.80	0.80 -0.99	80 52	400 600	250 250	956 903									
												I IVANIE A	1 ACMIVIT	35	-0.99	0.40	68	400	250	1109
					25	-2.36	1.28	95	400	250	492									
		RAMP B		LOOP	25	1.28	4.48	83	400	250	578									
					35	-0.86	0.74	78	200	250	532									
			RAMP C	RAMP	35	0.74	3.00	111	400	250	1278									
					35	3.00	0.72	66	300	250	623									
I-24	52	51.398	RAMP D	LOOP	25	-1.20	-3.06	22	300	250	730									
					25 35	-3.06 -1.65	2.38 -0.79	141 42	300	250 250	274 1326									
			RAMP E	RAMP	35	-0.79	1.33	104	300	250	1400									
			DAME 5	1005	25	-4.08	-0.51	93	400	250	502									
			RAMP F	LOOP	25	-0.51	1.16	43	400	250	940									
			RAMP H	LOOP	25	-1.70	2.42	107	400	250	432									
			10.00	2001	25	2.42	0.80	19	400	250	867									
			RAMP G	RAMP	35	0.80	-3.20	116	1000	250	735									
			L		35	-3.20	Illegible	-	400	250	-									

			FREEWA	Y WEA	/ING WOR	KSHEE	T .		
Genera	ıl Informat	tion			Site Info	rmation			
Analyst Agency/Co Date Perfo Analysis Ti	rmed		ase Parkway / er Engineering /2010		Freeway/Dir o Weaving Seg Jurisdiction Analysis Yea	Location		ase Parkway) and Purchas	
Inputs									
Weaving n Weaving so Terrain	ee-flow speed, umber of lanes eg length, L (ft)	, N	50 3 111 Lev	el	Weaving type Volume ratio, Weaving ratio	VR		A 0.7 0.7	
Conver	rsions to p	pc/h Unde	er Base C	ondition					ĝ .
(pc/h)	V	PHF	Truck %	RV %	E _T	ER	$f_{\rm HV}$	fp	Ň
V _{o1}	367	0.90	25	0	1.5	1.2	0.889	1.00	458
V ₆₂	0	0.90	0	0	1.5	1.2	1.000	1.00	0
V _{w1}	229	0.90	25	0	1.5	1.2	0.889	1.00	286
V _{w2}	610	0.90	25	0	1.5	1.2	0.889	1.00	762
V _w				1048	V _{nw}				458
V									1506
Weavin	ng and No	n-Weavir	g Speeds	3					
			Unconstr		Soul			trained	
		Weaving	(i = w)	Non-Wea	ving (i = nw)		ng (i = w)		ving (= nw)
a (Exhibit 2							.35 .20		020 00
b (Exhibit 2 c (Exhibit 2		8 8			- 4		.97		30
d (Exhibit 2							.80		75
Weaving inten							.70		28
Weaving and r speeds, Si (mi	non-weaving					29.81		46.30	
Number of	lanes required number of lane:	s, Nw (max)			1.89 1.40	if Nw > N	w (max) const	rained operati	on
Weavin	ng Segme	nt Speed,	Density,	Level o	f Service,	and Cap	oacity		
Weaving s	egment speed,	S (mi/h)	7	33.43					
Weaving s	egment density	, D (pc/mi/ln)		15.02					
Delta de Australia									

Notes

Level of service, LOS

Capacity of base condition, c_b (pc/h)

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp

Capacity as a 15-minute flow rate, c (veh/h)

Capacity as a full-hour volume, c_h (veh/h)

- Capacity constrained by basic freeway capacity.
 Capacity occurs under constrained operating conditions.
 Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in

В

4249

3777 3399

- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C). g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such
- . Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such

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MEETING REPORT Kick Off Meeting I-69 Strategic Planning Study – Fulton to Eddyville MAY 26, 2010

A Project "Kick-Off" Meeting was conducted on May 26, 2010 for this project at the District 1 Office in Paducah, Kentucky.

Attendees were:

Tim Choate Bill Gulick Ted Merryman David Martin Steve Ross Jill Asher Tom Hines Chris Kuntz Randy Williams Blake Beyer Christa Turner Mike McGregor Bryan Black Michael Oliver Susan Oatman Kyle Poat David Isley Lee Klieman Gary Sharpe	KYTC District 1 KYTC – Central Office KYTC District 1 KYTC District 1 KYTC District 1 KYTC – Environmental KYTC – Environmental KYTC – TEBM KYTC District 1 RYTC District 1 BLA BLA Palmer Engineering	Tim.choate@ky.gov Bill.gulick@ky.gov Edward.merryman@ky.gov Charles.martin@ky.gov Steve.ross@ky.gov Jill.asher@ky.gov Thomas.hines@ky.gov Chris.kuntz@ky.gov Randy.williams@ky.gov Blake.beyer@ky.gov Christa.turner@ky.gov Mike.mcgregor@ky.gov Bryan.black@ky.gov Michael.oliver@ky.gov Susan.oatman@ky.gov Kyle.poat@ky.gov Disley@blainc.com Lklieman@blainc.com Gsharpe@palmernet.com
Gary Sharpe	Palmer Engineering	Gsharpe@palmernet.com
Will Conkin	Palmer Engineering	Wconkin@palmernet.com

Following introductions, Tim Choate provided a brief overview of the project. Mr. Choate noted that this "Kick-Off" Meeting followed and earlier "Pre-Kick-Off" Meeting held April 29, 2010 with Tim Choate and Ted Merryman. The earlier meeting was held to facilitate the consultant getting started with initial evaluations of existing conditions.

Following opening remarks by Mr. Choate, Gary Sharpe summarized discussions from the "Pre-Kick-Off" Meeting held on April 29, 2010 at the District 1 Office. The meeting notes from this meeting are attached for reference (Attachment A) and resulted in the additional discussion points.

Key References for Applicable Geometric and Engineering Criteria:

- AASHTO Policy on Geometric Design of Highways and Streets, 2004 Edition
- AASHTO Policy on Design Standards Interstate Standards, 2005
- AASHTO Roadside Design Guide, 2006

Other reference sources will be the study reports for Section of Independent Utility No. 5 from Eddyville to Henderson, the various Corridor 18 Study Reports that may have application to this project, and the Division of Highway Design Manual, current edition.

The negotiated scope of services was used as a guide for discussions. In general, it was noted that work on the existing I 24 segment of the corridor would involve a more reduced scope of services. More specifically, for purposes of this study, the assessment of existing conditions on I 24 will be limited to the following:

- Existing vertical clearances
- Existing ramp taper lengths
- Crash history analyses
- Level of Service (LOS) calculations

In regard to Level of Service Calculations, it was noted that Level of Service is a matter of choice for the highway designer and is not a strict guideline for compliance in meeting requirements for designation of a route as an interstate highway. Agency policies for Level of Service may be a consideration for development of future improvement concepts. Thus Level of Service will be determined for roadway segments within the corridor and will be reported but will not be a consideration for developing recommendations for this study.

The extent that safety hardware and more specifically guardrail end treatments meet current criteria will be evaluated for this project. The consultant was directed to identify any second generation or older guardrail end treatments that do not meet current standards. Length of need will not be evaluated except for locations identified with substandard guardrail end treatments. Where substandard guardrail end treatments are identified, an estimate of length of need meeting current criteria will be developed and included in recommended improvement scenarios.

It was noted that guardrail end treatments on I 24 between MP 27 and 31 will be updated to current standards as a part of a pavement rehabilitation project during the summer 2010.

Earthen mounds used for pier protection in the median were discussed. It was noted that depending upon the width of median and side slopes, these may not be consistent with criteria in the AASHTO Roadside Design Guide, 2006 edition. District 1 staff noted that where the pier footings were below the elevation of the median, these could be removed and had been removed at some locations during earlier rehabilitation projects. The consultant was directed to review as-built structure plans and to validate locations in the field where modifications to existing pier protection may be needed.

Traffic forecasting assumptions were discussed. After discussion, it was ultimately decided that the annual growth rates for traffic analyses used for this segment of I 69 should be similar to those used with the previously completed section from Eddyville to Henderson. For the study from Eddyville to Henderson, the following parameters were used:

Average Annual Growth Rates Without I 69:
Average Annual Growth Rates With I 69 & I 66 (Ford Parkway):
Average Annual Growth Rates with I 69 (Breathitt Parkway)
2.2% to 2.3%

Based on this information, the consultant was directed to use a 2% annual growth rate for the corridor without I 69. It can be seen from the above information that the relative increase in annual growth rate from the Eddyville to Henderson Study is 0.5% to 0.6% for the segments of the study that did not also include I 66. Thus, the *consultant recommends* (on the basis of the adjacent section) that an annual growth rate of 2.5% for the corridor with an I 69 designation be used for this study.

Treatment of roadside signs was discussed. The consultant was advised during the assessment of existing conditions that roadside signs that (1) were not shielded by guardrail, and (2) did not include *break-away posts* should be identified and addressed in the report. Signs that are obviously outside the clear zone should be noted as such.

The importance of identifying substandard vertical clearances was discussed as a critical aspect for the report and an ultimate designation of the corridor as a segment of I 69. The consultant advised that initial screening for substandard vertical clearances will be done using vertical clearance maps provided by the district, as-built plans, plans from more recent pavement rehabilitation projects, and data from pavement management records. The consultant concurred that vertical clearances will be verified in the field. Ted Merryman provided the consultant with a listing of structures that appeared to be less than the minimum 16.0 feet required for interstate designation.

Mr. Merryman also briefed the group on the status of recent meetings with the FHWA concerning concepts for upgrading segments of the Wendell H. Ford Western Kentucky Parkway and Edward T. Breathitt Pennyrile Parkway (SIU 5 between Eddyville and Henderson) to meet interstate standards. Mr. Merryman advised that many of the concepts and agreements evolving from these discussions for SIU 5 were likely to have application for this segment (SIU 6 Fulton to Eddyville).

Crashworthy bridge railing was discussed. The consultant was advised that substandard bridge railing not meeting NCHRP standards for crashworthy bridge railing should be replaced or upgraded to meet current standards. In situations where shoulder widths do not meet interstate criteria, shoulders should be widened to meet current criteria and bridge railing installed that meets interstate criteria. In situations where the existing shoulder width satisfies current standards for interstate routes but the bridge railing is substandard, bridge railings should be reconstructed or modified to meet current interstate standards. Discussions for modifying bridge railing included (1) removal of the existing railing and reconstruction with a bridge railing meeting current criteria, (2) adding either a constant slope wall or other approved shape wall on top of the existing curbs (if curb widths are sufficient), or (3) securely fastening thrie-beam guardrail to the existing bridge railing. The consultant was advised that the assessment of existing conditions would include identifying all locations with substandard bridge railing and substandard shoulder widths. It was further agreed that the consultant would work with Division of Highway Design staff to determine a "reasonable cost" for modifying bridge railing to meet current

interstate criteria and would use this as a basis for inclusion in cost estimates for needed improvements to meet interstate criteria.

Paved ditched in the median was discussed. It was noted that paved ditch in the median should not have an effect on potential designation as an interstate route unless median and ditch slopes were outside allowable criteria per the AASHTO Roadside Design Manual. The extent of pavement ditches in the median will be discussed in the report but will only be addressed recommendations if re-grading of the median is needed to meet interstate criteria.

Pier protection and crash attenuators for existing bridges were discussed. Crash attenuators that do not meet current criteria will be identified and addressed in recommendations for improvement scenarios. Of particular significance are those locations where bridge piers in the median are protected by earthen mounds. As-built bridge plans will be reviewed to determine the location of pier footings. Median slopes and distances from the driving lanes to the piers will be evaluated for compliance with the AASHTO Roadside Design Guide. Improvement scenarios for locations not meeting Roadside Design Guide criteria will be addressed in the report and recommendations.

Crash history analyses were discussed. Crash history data will be collected for the 2005-2009 period. Crash analysis will be conducted utilizing the Kentucky Transportation Center's Analysis of Traffic Accident Data in Kentucky methodology. Fatal crashes will be distinguished in the crash analysis. The consultant was requested to evaluate crash history data in combination with non-compliant design features to determine if there is a direct relationship between crash history and non-compliant design features.

During contract negotiations, it was requested that the format for reporting follow a similar format to the earlier study completed for SIU 5 between Eddyville and Henderson. The consultant requested and received the "Project Development Analysis Tool and Users Guide" used with SIU 5 Study and will use during preparation of the report for SIU 6 from Fulton to Eddyville.

Tim Choate led a discussion with the group concerning the section of the project involving the Mayfield Bypass. Mr. Choate distributed Attachment C and discussed the noted items in the context of a review of conditions observed during a recent visit to the site. The following summarizes the results of these discussions:

It was agreed that the items identified in the handout were valid concerns and should be addressed in some context. It was agreed that the Mayfield Bypass should be evaluated on the basis of urban interstate standards. It was specifically noted that since this project did not involve adding capacity, that noise walls would not be required. Finally, it was agreed that roll curbs should be eliminated for all ramps and mainline situations but could be left in place for cross-roads.

The meeting concluded with a brief discussion of the proposed public Meeting for this project. It was agreed that the location of the public meeting would be in the vicinity of Mayfield, Kentucky. It is anticipated that the public meeting will be held in mid October. District 1 will make arrangements for the location, date, and time for the meeting. It also

	-	d officials day of the		included	the	project

Prepared By: Will Conkin, PE

Will Conkin, PE Gary W. Sharpe, PE, PLS

MEETING NOTES

Project: I-69 (Purchase Parkway/I-24) Strategic Planning Study

Attendees: Tim Choate, KYTC Project Manager

Ted Merryman, KYTC I 69 Coordinator

Lee Klieman, Bernardin, Lochmueller & Associates

Will Conkin, Palmer Engineering Gary W. Sharpe, Palmer Engineering

Purpose: Pre-kickoff meeting—getting started

Location: District 1 Office, Paducah

Meeting Date: April 29, 2010

The following items were discussed:

- A formalized Kickoff Meeting will be scheduled Tim Choate will coordinate
- Key References:
 - o 2005 Policy on Design Standards Interstate System
 - o The 2004 AASHTO Policy on Geometric Design of Highways and Streets
 - o 2006 AASHTO Roadside Design Guide
- Because of scope reductions, only the following work will be done on I 24:
 - Crash Analyses
 - o Level of Service (LOS) Analyses
 - o ADT
 - o Ramp Taper Lengths
 - Mainline Vertical Clearances
- The following were noted as information to be included in the Assessment of Existing Conditions for comparison with current AASHTO Standards.
 - o Vertical Clearances
 - o Ramp Taper lengths
 - o Bridge Widths including shoulder and brush block widths
- Information from the Corridor 18 studies will be reviewed for relevant information such as traffic forecast assumptions, truck percentages and annual growth rates, etc that were used
- Crash histories will be obtained from the beginning of the project to the I-24 Interchange with Western Kentucky Parkway
- The weaving section for the intersection of the Purchase Parkway and I-24 was noted as a potential concern
- Paved ditches with headwalls within the clear zone were discussed in the context of potential repairs needed for future rehabilitation projects
- The sign inventory was requested by Palmer Engineering and was provided by District 1

- End Treatments not meeting current standards should be identified
- Cost estimates will be included as part of the study
- Identify potential problems for interchanges within the corridor
- HIS may be another source of data in addition to As-built plans
- The Kentucky State Police website will be the source of crash data
- Potential access for the Graves Industrial Park should be mentioned in the final report
- For comparison with AASHTO standards, the Mayfield Bypass segment will be treated as an Urban interstate
- Questions and requests for information should be made through Tim Choate and copied to appropriate persons
- The Project Team will wear safety vests while collecting data in the field.
- KYTC will provide information on Pavement Rehabilitation contracts
- KYTC provided the maps of Bridge Vertical Clearances within the project area Vertical clearances that are near 16 feet +/- will be validated
- Cross slopes for the Mayfield Bypass will be reviewed

Prepared by: Will Conkin

		-1 1
Vertical	Clearance-	Check

MP 2.5

MP 4.1

MP 8.3

MP15.2(low)

MP16.5

MP17.7

MP22.2(low)

MP37.9

MP46.95

Check Mainline Ramp Tapers for AASHTO standards



Determine whether any correlation exists between noncompliant features and crash history.

Bridge Curbs

Guardrail End Treatments

HAMPOUT PROVIDED BY TIM Choate

- 1. The need for auxiliary lanes Northbound and Southbound between exit 24 and exit 25.
- 2. Treatment of median. Barrier wall or cable barrier. High wall preferred due to curvature.
- 3. The need for auxiliary lanes between KY 80 exit and Trumpet Conflict between weaves and southbound departures. Also future extension of Pky. to southside of town 1-181.50 and 181.40
- 4. Can existing Trumpet function as system interchange without modifications
- 5. The need for 2-lane on-ramps and departure ramps in Trumpet
- 6. Elimination of roll curbs
- 7. Is noise walls required
- 8. Clearance at KY 80 overpass. Can middle span be modified with box beams to get 16 feet. Also widen bridge for sight distance with new barriers similar to what was done at Exit 27 (KY 131)
- 9. Can Additional overhead signage help with this system?

MEETING REPORT I-69 Strategic Planning Study – Fulton to Eddyville June 8, 2010

A Pre-Interdisciplinary Team Meeting for this project was held on June 8, 2011 at the Transportation Cabinet Central Office in Frankfort, Kentucky. The purpose of this meeting was to identify preliminary issues and concerns regarding the draft project study report prior to holding an Interdisciplinary Team Meeting for formal review of the draft report for this study. Initial draft copies of the study report were distributed to a small group (including attendees) for initial comments regarding the format and presentation of information included in the report.

Attendees were:

Ted Merryman David Martin KYTC – Central Office Keith Damron Steve Ross KYTC – Central Office Falmer Engineering Palmer Engineering Will Conkin Palmer Engineering	Edward.merryman@ky.gov Charles.martin@ky.gov Keith.damron@ky.gov Steve.ross@ky.gov Jill.asher@ky.gov Dlindeman@palmernet.com Gsharpe@palmernet.com Wconkin@palmernet.com
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Attendees via Video Teleconference from the District 1 Office in Paducah were:

Jim LeFevre	KYTC – Central Office	<u>James.lefevre@ky.gov</u>
Mike McGregor	KYTC – District 1	Mike.mcgregor@ky.gov
Jessica Herring	KYTC – District 1	<u>Jessica.herring@ky.gov</u>

Gary Sharpe opened the meeting with a brief discussion of the status of the project and more specifically summarized information included in the draft report.

Ted Merryman, State Highway Engineer's Office and I-69 Coordinator, discussed the current status of a draft agreement between KYTC and FHWA for design variances and design exceptions associated with designating Section of Independent Utility (SIU) 5 as I-69. SIU 5 includes a segment of the Western Kentucky Parkway from I-24 near Eddyville and portions of the Pennyrile Parkway from the Western Kentucky Parkway to Henderson. Mr. Merryman further noted that initial emphasis for designation of I-69 in SIU 5 would be the 38 mile segment of the Western Kentucky Parkway from I-24 to the Pennyrile Parkway.

It was further discussed that it was anticipated that a similar agreement would ultimately be developed for the section of I-69 covered by this study — SIU 6 from Fulton at the Tennessee State Line along the Purchase Parkway to I 24 near Calvert City and then with I 24 to the Western Kentucky Parkway (beginning of SIU 5). It was further noted that since there are many similar issues and considerations for design variances and design exceptions among SIU 5 and SIU 6, some aspects of the draft agreement with the FHWA also may have application for SIU 6. An updated copy of the *Draft Interstate* 69

Agreement Between Commonwealth of Kentucky Transportation Cabinet and Federal Highway Administration was provided to the consultant for their information in finalizing the draft study report for SIU 6.

It was further noted in the meeting that it was the KYTC's intent to request design exceptions and design variances for design elements not meeting current interstate standards where there were no indications of crash histories with a critical rate factor exceeding 1.0. Mr. Merryman emphasized in his comments that if there were crash history data associated with any substandard roadway or structure element, these should be identified and an improvement strategy should be recommended for addressing the roadway or structure element in question. It was further emphasized that design exceptions and design variances could be treated as either permanent or temporary, depending on the specific conditions.

In the draft study report, a crash history analysis has been provided for roadway and structures deficiencies that do not meet current interstate standards. For example, a crash history analysis will be presented to describe crash history data associated with the narrow mainline bridges on the Purchase Parkway where mainline bridge width is not consistent with roadway and shoulder approach width.

Discussions continued regarding acceleration and deceleration taper lengths commonly used by the KYTC and presented in the Division of Highway Design Manual as opposed to minimum acceleration and deceleration taper lengths per AASHTO standards. It was agreed that determination of whether or not a ramp met criteria would be in accordance with AASHTO standards.

There also was discussion concerning parameters for developing cost estimates. It was agreed that cost estimates for spot improvements at interchanges would be developed on the basis of spot improvements (with design variances and design exceptions) at specific locations but would be summarized for the entire interchange so as to allow for a direct comparison of spot improvements as compared to a fully reconstructed interchange. Cost estimates will be developed for (1) spot improvement concepts with design exceptions and variances as appropriate and (2) more extensive improvement strategies without design exceptions and variances.

Bridge peir protection that does not meet interstate standards will be recommended for improvement. Currently some overpass bridge piers have earthen mound protection. An estimate will be provided in the report for improving these locations.

Jill Asher will coordinate with FHWA, but tentatively, an IDT meeting is scheduled for the last week of July 2011 at the Central Office in Frankfort. This meeting may be scheduled in conjunction with a Lake Bridges status meeting. The IDT Meeting has since been scheduled for July 26, 2011 from 1:00 pm to 3:00 pm in Conference Room C122, Transportation Building, Frankfort, Kentucky

Prepared By: Will Conkin, PE, PTOE

Gary W. Sharpe, PE, PLS

MEETING REPORT I-69 Strategic Planning Study – Fulton to Eddyville July 26, 2011

An Interdisciplinary Team Meeting for this project was held on July 26, 2011 at the Transportation Cabinet Central Office in Frankfort, Kentucky. The purpose of this meeting was to review the draft report for this study. Draft copies of the study report were distributed to the project team.

Attendees were:

Mike McGregor	KYTC – District 1	Mike.mcgregor@ky.gov
Kevin Damron	KYTC – Central Office	Kevin.Damron@ky.gov
Ted Merryman	KYTC – Central Office	Edward.merryman@ky.gov
David Martin	KYTC – Central Office	Charles.martin@ky.gov
Keith Damron	KYTC – Central Office	Keith.damron@ky.gov
Bill Gulick	KYTC – Central Office	Bgulick@ky.gov
Steve Ross	KYTC – Central Office	Steve.ross@ky.gov
Jill Asher	KYTC – Central Office	<u>Jill.asher@ky.gov</u>
Ryan Tenges	FHWA	ryan.tenges@dot.gov
John Ballantyne	FHWA	<u>John.ballantyne@dot.gov</u>
Steve Mills	FHWA	Steve.mills@dot.gov
David Lindeman	Palmer Engineering	Dlindeman@palmernet.com
Gary Sharpe	Palmer Engineering	Gsharpe@palmernet.com
Will Conkin	Palmer Engineering	Wconkin@palmernet.com
Lee Klieman	BLA	Lklieman@blainc.com
Ben Quinn	AEI	Beng@aei.com

Attendees via Video Teleconference from the District 1 Office in Paducah were:

Susan Oatman	KYTC – District 1
Randy Williams	KYTC – District 1
Jessica Herring	KYTC – District 1
Chris Kuntz	KYTC – District 1
Mike Oliver	KYTC – District 1
Craig Morris	Pennyrile ADD
Stacey Courtney	Purchase ADD

Gary Sharpe opened the meeting with a brief discussion of the status of the project and more specifically summarized information included in the draft report. A power point presentation was presented covering the findings of the report. During the presentation, the following discussions occurred referring to the content of the report.

Design Exception / Design Variance: In the report, design exceptions will specifically refer to the FHWA referenced controlling 13 design criteria which are the following:

- 1. Design Speed
- 2. Lane Width
- 3. Shoulder Widths
- 4. Bridge Width
- 5. Horizontal Alignment
- 6. Superelevation
- 7. Vertical Alignment

- 8. Grade
- 9. Stopping Sight Distance
- 10. Cross Slope
- 11. Vertical Clearance
- 12. Lateral offset to obstruction
- 13. Structural Capacity

In the report deficient design elements not listed in the controlling 13 design criteria but that are deviations from typical practices for design of interstate highways will be referenced as design variances.

Vertical Curve / K value: Bill Gulick asked a question about the K value used to calculate the stopping sight distance. Mr. Gulick referenced the maximum K value (167 ft) provided on pages 270 and 274 of the AASHTO *A Policy Geometric Design of Highways and Streets* (Green Book) for crest and sag vertical curves and the relationship to pavement drainage. The passage from the Green Book is "It is not intended that K of 167 ft per percent grade be considered a design maximum, but merely a value beyond which drainage should be more carefully designed." The minimum length of curve and stopping sight distance was calculated in the report using the following K values (Exhibits 3-72 and 3-75 of the Green Book).

Sag Vertical Curves Rural 70 mph – K= 181 Urban 50 mph – K = 96

Crest Vertical Curves

Rural 70 mph - K = 247Urban 50 mph - K = 84

Mr. Gulick further described conditions on other interstate routes where K values had exceeded K = 167 and where there were significant crash histories. It was noted that vertical curves with insufficient length or with less than the required calculated stopping sight distance would be recommended for improvement if there was a significant crash history at that location (Critical Rate Factor > 1.0).

Interchange Control of Access: Measurement of the interchange control access was discussed. For the draft report, the interchange control of access was measured in the field from the ramp radius to the radius of the closest entrance. According to the KYTC Highway Design Manual, access control should have been measured from the end of the interchange ramp radius to the center line of the closest access point. It was agreed that control of access measurements presented in the report would be updated according to this standard.

Mainline Bridge Width: According to *A Policy on Design Standards Interstate System* the offset to the face of parapet or bridge rail on both the left and right side is 3.5 feet for bridges longer than 200 feet. Therefore, the minimum lateral clearance for mainline bridges is 31 feet (offsets plus 2-12 foot lanes). For bridges less than 200 feet, the lateral clearance shall, at a minimum, equal the paved approach roadway width. Therefore at a minimum, mainline bridges less than 200 feet long shall have a lateral clearance of 38 feet

(4 foot inside shoulder, 10 foot outside shoulder, and 2 -12 foot travel lanes). All bridges on the Purchase Parkway with a lateral clearance less than 38 feet longer than 200 feet.

Mainline Bridge Side Railing/Barriers: All mainline bridges have side railings/barriers with a 10" brush block that are inconsistent with current standards. It will be clarified in the report that the brush block is the deficient element of the side railing/barrier and not the railing. It was noted in the discussion that side railings/barriers with brush blocks can be retrofitted with thrie beam or improved with a sloped face barrier.

Mayfield Bypass Design Speed: The Mayfield Bypass meets the minimum horizontal alignment criteria for a 50 mph design speed, but not a 70 mph design speed. During discussions of the Mayfield Bypass segment of the project, the consultant was requested to back-calculate the design speed of the Mayfield Bypass based on the horizontal alignment and include this information in the report. The smallest radius for a curve on the Mayfield Bypass is 1146 feet (located at MP 21.585 and MP 21.793). Based on the 8% maximum superelevation tables, the 1146 radius results in a 59 mph design speed. This is based on a 8% superelevation. According to the as-built plans, both of these curves have a superelevation of 8%.

Superelevation: A superelevation rate of 8% is the maximum superelevation rate recommended in the current edition for the AASHTO Policy on the Geometric Design of Highways and Streets (Green Book) for areas with snow and ice. Thus, for interstate highways in Kentucky, the maximum rate for superelevation currently recommended is 8%. However, it was noted that when the Purchase Parkway was constructed, it was common to use superelevation rates up to 10% on high speed facilities. As a result of this practice, there were four (4) curves identified that exceed 8% superelevation (8.3%). With further discussion it was noted that although these curves do not meet the current standard, the greater superelevation does not necessarily result in an unsafe situation unless there was a significant crash history identified at that specific location. Thus it was agreed that recommendations for addressing areas with superelevation rates greater than 8% will be to measure actual superelevation rates in the field whenever the next pavement rehabilitation projects were scheduled and to make appropriate modifications in pavement rehabilitation project.

Earthen Mound / Pier Protection: Currently there are 8 overpass bridges that have an earthen mound pier protection that does not meet current standards. It was discussed that there are earthen mound pier protection at these overpass bridges because the pier footers may protrude above the ground line. As-built plans for bridges will be reviewed and the locations at which the footers are higher than the existing ground will be identified and included in the report.

SIU 5 / SIU 6 Connection: It was noted in the discussions that there had been some difficulties in coordinating with Tennessee Department of Transportation (TDOT) and FHWA-Tennessee concerning the connection of SIU 5 and SIU6 at the Tennessee and Kentucky state line. Mr. John Ballentyne and Mr. Steve Mills advised that they would facilitate a meeting with KYTC, TDOT, and FHWA concerning the connection of I-69 at the border.

I-69 / I-24 Interchange: The Project Team requested a fully directional interchange be evaluated and presented in the report as a potential alternative at the I-24 and Purchase Parkway Interchange. The interchange should include 70 mph design speed for the I-69 ramp through movements. As presented, the draft report included a fully directional interchange but with 50 mph design speed ramps for the I-69 through movements. The report also will include a lower cost partially reconstructed interchange at this location to address operational concerns (weaving lengths, etc) with construction of improvements staged based on traffic demand. It was initially envisioned that this concept would involve providing an I 24 westbound to I 69 southbound flyover ramp and an I 24 eastbound to I 69 southbound ramp improvement as the initial construction for this concept.

Cost: The construction costs of the alternatives should be based on geographical unit cost to achieve utmost accuracy. The unit costs for the estimate will be evaluated by the District 1 staff to verify local construction costs versus statewide average. Consultant will send District 1 unit cost from draft report.

Potential Alternatives: During the meeting the project team decided to present four potential alternatives for recommendations. The following alternatives provide brief description of desired alternatives:

- 1. No Build This alternative would leave a gap in the nationally proposed I-69 route. However, the Purchase Parkway would provide the connectivity for the I-69 traffic to travel from Tennessee to I-24.
- 2. Necessary Upgrade and Spot Safety Improvements Key safety and operational concerns would be addressed. In addition, design exceptions and variances would be obtained for the existing conditions that do not meet current AASHTO guidelines and are deemed appropriate by the KYTC and the FHWA.
- 3. Partially Compliant with Design Exceptions This alternative would involve improvements within existing right of way or with minimum right of way acquisition necessary for making the existing parkway meet minimum AASHTO criteria for interstate routes with minimal design exceptions and variances.
- 4. Fully Compliant without Design Exceptions This alternative would involve improvements within existing right of way or with minimum right of way acquisition necessary for making the existing parkway meet minimum AASHTO criteria for interstate routes without any design exceptions and variances.

Prepared By: Will Conkin, PE, PTOE

Gary W. Sharpe, PE, PLS

AGENDA

IDT Meeting

I-69 Strategic Corridor Planning Study

Overview of Existing Conditions of Julian M. Carroll (Purchase) Parkway and I-24 Fulton to Eddyville

Fulton/Hickman/Graves/Marshall/Livingston/Lyon Counties Central Office, Frankfort July 26, 2011

- A. Introductions
- B. Opening Comments Ted Merryman / Mike McGregor
 - I. Purpose of Meeting
 - II. Previous Studies (Corridor 18 studies, SIU 5)
 - III. SIU 6 Study Philosophy
- C. Report Review Power Point
 - I. Early Coordination and Public Involvement
 - II. Operational Considerations
 - III. Mainline Geometry / Typical Sections
 - IV. Bridges and Overpasses
 - V. Interchanges and Ramps
 - VI. Potential Improvement Alternatives and Development Costs
 - VII. Recommendations
 - VIII. Appendices
- D. Questions / Discussions
- E. Final Steps / Completion
 - I. I-69 Project Development Analysis Tool
 - II. Public Meeting Summary
 - III. Final Submittal
- F. Coordination with ongoing projects
- G. Adjourn

Overview of Existing Conditions of Julian M. Carroll (Purchase) Parkway and I-24 July 26, 2011 I-69 STRATEGIC CORRIDOR PLANNING STUDY IDT MEETING

	Name	Organization	Telephone	E-mail Address
H	David Lindeman	Palmer Engineering	859-744-1218	dlindeman@palmernet.com
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က်	Will Conkin	Palmer Engineering	859-744-1218	wconkin@palmernet.com
4	Lee Klieman	BCA	43-479-6200	Iklieman @ Haine. ram
r,	Jill Asher	KYTC C.O. Daning	2812 H95 ZOC	Jill, asher@ ky, gov
6.	6. DALO MARIN	MYTE-CO DESIGN	502-5/4-3280	charles. mantin elyo. sol
7.	MIKE MEGREGON	KYTC DI PROS. Drev.	270-898-2431	MILESMCGREGORE KY. 600
∞ં	Kevin Dancen	KYTC &SHE	502.564.3730	Kesin, Damron @ Ky-500
်တ	TED MERRYMAN	HYTC SHE	0528 495205	ESWARD. WEARYMAN @ My. GOV
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Overview of Existing Conditions of Julian M. Carroll (Purchase) Parkway and I-24 I-69 STRATEGIC CORRIDOR PLANNING STUDY IDT MEETING July 26, 2011

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1-69 STRATEGIC CORRIDOR PLANNING STUDY IDT MEETING

Overview of Existing Conditions of Julian M. Carroll (Purchase) Parkway and I-24 July 26, 2011

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FOLLOW UP TO IDT MEETING REPORT I-69 Strategic Planning Study – Fulton to Eddyville AUGUST 24, 2011

A follow up meeting to the Interdisciplinary Team Meeting for this project was held on August 24, 2011 at the Transportation Cabinet Central Office in Frankfort, Kentucky. The purpose of this meeting was to review edits made to the draft report, previously reviewed at the IDT meeting on July 26, 2011. The edits were distributed to the project team.

Attendees were:

Kevin Damron Ted Merryman David Martin	KYTC – Central Office KYTC – Central Office KYTC – Central Office	Kevin.Damron@ky.gov Edward.merryman@ky.gov Charles.martin@ky.gov
Bill Gulick Steve Ross Jill Asher David Lindeman	KYTC – Central Office KYTC – Central Office KYTC – Central Office Palmer Engineering	Bgulick@ky.gov Steve.ross@ky.gov Jill.asher@ky.gov Dlindeman@palmernet.com
Gary Sharpe Will Conkin	Palmer Engineering Palmer Engineering	Gsharpe@palmernet.com Wconkin@palmernet.com

Attendees via Video Teleconference from the District 1 Office in Paducah were:

Jim LeFevre	KYTC – District 1	James.LeFevre@ky.gov
Mike McGregor	KYTC – District 1	Mike.mcgregor@ky.gov
Jessica Herring	KYTC – District 1	
Stacey Courtney	Purchase ADD	

Gary Sharpe opened the meeting with a brief discussion of the status of the project and draft report. The purpose of the meeting was to review edits made to the draft report based on comments during and following the IDT meeting held on July 26, 2011. During the review of these edits, the following discussions occurred referring to the content and recommendations of the report. Other minor editorial changes (not mentioned in this summary) were made during the meeting.

Design Exception / Design Variance: In the report, design variances were more clearly defined. There are two categories for design variances discussed in the report--a design feature that (1) varies from the current AASHTO criteria but not part of the *13 controlling criteria* or (2) a design feature that varies from common practice but not part of the *13 controlling criteria*.

Superelevation Crash Analysis: A crash analysis was done on the Purchase Parkway horizontal curves with a superelevation greater than 8%. There was one horizontal curve (MP 47.417) with a critical crash rate factor greater than one. The crashes were reviewed by the project team. The project team agreed that the crash history did not appear to be directly related to superelevation. Therefore improvements to the superelevation at this location will not be recommended in the report.

Superelevation: The report was edited to according to the KYTC policy and AASHTO recommendations for superelevation on freeways and expressways. According to the AASHTO Policy on Geometric Design of Highways and Streets, current edition (commonly referred to as the Green Book), the maximum superelevation rate is controlled by climate conditions, terrain conditions, type of area, and frequency of slow-moving vehicles that may be affected by high superelevation rates. A specific maximum superelevation is not recommended for an Interstate facility by AASHTO. It is left to the user agencies to make specific policy decisions concerning allowable rates of superelevation. The KYTC policy references the Green Book for freeway geometric design. The Green Book provides superelevation rate tables for 4%, 6%, 8%, 10%, and 12% maximum superelevations.

Referencing the Federal Highway Administration *Mitigation Strategies for Design Exceptions*, "A formal design exception is required if the State's superelevation policy cannot be met in design of any curve on the NHS." This document advises, "A design exception is also required if a superelevation rate is proposed that is different from the published rate per the State's policy for that curve, regardless of whether the curve is a controlling one (minimum radius for a design speed) or not." From review of as-built plans and field inspections, it appears that the Purchase Parkway was constructed on the basis of 10% maximum superelevation. Since the Purchase Parkway appears to have been constructed with a maximum superelevation of 10% which is compliant with AASHTO and KYTC policies and there no apparent crash histories related to superelevation, a design exception for superelevation does not appear warranted.

Crash Analysis: In the draft report, segments of the Purchase Parkway and I-24 that have a critical crash rate factor between 0.9 and 0.99 were defined as Potential High Crash Segments. The term "Potential" was considered misleading and was removed from the report. The report will identify these segments as crash segments having a critical crash rate factor between 0.9 and 0.99.

Mayfield Bypass: The project team discussed evaluating the Mayfield Bypass as an urban interstate. The project team reviewed the existing roadway geometry/cross section and determined it was designed with the intention to serve the City of Mayfield as an urban expressway. The interchanges are spaced at one mile or farther. The traffic volumes are significantly higher along the Mayfield Bypass than the rural sections of the Purchase Parkway to the north and south of Mayfield. The crash analysis shows the Mayfield Bypass operates safer than most of the Purchase Parkway. According to KYTC, the Mayfield Bypass is functionally classified as an Urban Freeway & Expressway. Based on this information, the Mayfield Bypass is currently performing as an urban expressway and should be evaluated as an urban interstate. It was also mentioned that currently the legal speed limit along the Mayfield Bypass is signed 70 mph. Once the Mayfield Bypass is designated I-69, KYTC would take the appropriate steps to insure the legal speed limit is changed from 70 mph to 50 mph. The report will be edited to only compare the Mayfield Bypass to urban interstate criteria (50 mph design speed).

Potential Alternatives: During the IDT meeting (July 24, 2011) the project team decided to present four potential alternatives with a range of improvements. With additional discussion, the project team decided to present just three alternatives in the report. The following alternatives are presented in the report.

- 1. **No Build** This alternate would leave a gap in the nationally proposed I-69 route. However, the Purchase Parkway would provide the connectivity for the I-69 traffic to travel from Tennessee to I-24.
- 2. **Necessary Upgrades and Spot Safety Improvements** Key safety and operational concerns would be addressed. Design exceptions or variances would be obtained for the existing conditions that do not meet current AASHTO or KYTC guidelines that are deemed appropriate by the KYTC and the FHWA.
- 3. Fully Compliant Reconstruction This alternate would involve improvements within existing right of way or with minimum right of acquisitions necessary for making the existing Purchase Parkway meet minimum AASHTO criteria for interstate routes.

Necessary Upgrades and Spot Safety Improvements: Discussion of the alternative resulted in the following recommendations and edits:

- Mainline Structures (Widen Deficient Bridges): The project team chose to seek a design exception for the deficient bridges. All of the deficient bridges are longer than 200 feet and have a horizontal lateral clearance 30 feet. The minimum horizontal lateral clearance for a mainline bridge on an interstate over 200 feet in length is 31 feet. Based on the crash analyses, it is not apparent that the crash history is directly related to narrow bridge width. Therefore, it is not recommended to widen the deficient bridges by one foot, but seek a design exception for lateral horizontal clearance for the deficient mainline bridges.
- Mainline Structures (Upgrade Guardrail/Approaches/Railings): The project team decided that the bridge railing/barriers will be retrofitted rather than replaced. The cost of attaching thrie-beam guardrail to the existing barrier will be used in the estimate for retrofitting the existing barrier. This retrofit meets current crash worthy standards.
- I-24 and Purchase Parkway Interchange: The project team reviewed the previous interchange options presented in the draft report. Additional interchange options were presented based on comments during the IDT meeting. Also presented to the team were projected 2040 ramp design hourly volumes with I-69 and without I-69 traffic for the existing interchange configuration. These volumes were calculated from a 2007 ramp traffic count. Based on the ramp traffic volumes and capacity of the interchange, the project team recommends to improve the eastbound I-24 to southbound I-69 ramp and construct a new southbound I-69 flyover ramp from westbound I-24. The following existing ramps will be eliminated with this recommendation:
 - Westbound I-24 to northbound Purchase Parkway ramp
 - Westbound I-24 to southbound Purchase Parkway loop ramp
 - o Eastbound I-24 to northbound Purchase Parkway loop ramp.

The existing northbound Purchase Parkway to eastbound I-24 ramp will serve as the I-69 northbound movement. This ramp will accommodate the projected I-69

traffic in the near future. It is recommended to improve the ramp to meet interstate criteria once traffic volumes exceed capacity. It also is recommended to construct a new northbound I-69 to westbound I-24 flyover ramp once the traffic volumes exceed the existing loop ramp capacity.

Previous Toll Plazas: The interchanges located at Exit 14 and Exit 43 will be referenced as previous toll plazas versus flopped diamond.

Regional and Local Opportunities: It was decided to eliminate narrative referencing impacts to employment opportunities or specific locations as a result of designating the Purchase Parkway as I-69.

Cost Estimate: It should be noted in the report that the cost estimate for the presented alternatives does not include connecting Segment of Independent Utility (SIU) 6 to SIU 7 (Exits 0, 1, 2 at the Tennessee/Kentucky border) or to SIU 5 (I-24 at the Western Kentucky Parkway).

Prepared By: Will Conkin, PE, PTOE

Gary W. Sharpe, PE, PLS

AGENDA

Follow Up to IDT Meeting

I-69 Strategic Corridor Planning Study

Overview of Existing Conditions of Julian M. Carroll (Purchase) Parkway and I-24 Fulton to Eddyville

Fulton/Hickman/Graves/Marshall/Livingston/Lyon Counties Central Office, Frankfort August 24, 2011

- A. Introductions
- **B.** Opening Comments
 - I. Purpose of Meeting
- C. Report Review Updates
 - I. Chapter I Project Introduction
 - 13 Controlling Criteria (1-5)
 - II. Chapter IV Mainline Geometry/Typical Section
 - Superelevation Rate (4-7); Mayfield Bypass Design Speed (4-8)
 - III. Chapter V Bridges and Overpasses
 - Minimum Bridge Width (5-1); Bridge Side Railing/Barrier (5-2)
 - IV. Chapter VI Interchanges and Ramps
 - Superelevation Rate (6-8); Interchange Control of Access (6-11)
 - V. Chapter VII Key Findings of Existing Conditions Overview
 - AASHTO Minimum Guidelines (7-1); Superelevation (7-10, 7-11); Lateral Clearance (7-10); Narrow Bridge Crash Analysis (7-13); Interchange Control of Access (7-14)
 - VI. Chapter VIII Potential Improvement Alternatives and Development Costs
 - Interchange Alternatives
 - VII. Chapter IX Recommendations
 - VIII. Appendix G Project Meeting Minutes
 - IX. Crash Analysis
- D. Questions / Discussions
- E. Adjourn

FOLLOW UP TO IDT MEETING

I-69 STRATEGIC CORRIDOR PLANNING STUDY

Overview of Existing Conditions of Julian M. Carroll (Purchase) Parkway and I-24 AUGUST 24, 2011

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