# VALUE ENGINEERING SUMMARY OF <br> US 119/ZEBULON TO BENT MT. WPI NO. 12-308.1 \& 308.2 <br> PIKE COUNTY, KENTUCKY 

JANUARY 6-14, 1997

Prepared by:
Ventry Engineering
In Association With:

## Kentucky Transportation Cabinet, Department of Highways

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## I. LOCATION OF PROJECT




## II. TEAM MEMBERS AND PROJECT DESCRIPTION

TEAM MEMBERS

| NAME | AFFILIATION | EXPERTISE | PHONE |
| :--- | :--- | :--- | :--- |
| Jack Trickey, P.E. <br> C.V.S. | Ventry Engineering | Team Leader | $904 / 627-3900$ |
| Don Keenan, P.E. | Ventry Engineering | Structural <br> Team Member | $904 / 627-3900$ |
| Ron Whichel, P.E. | Ventry Engineering | Cost Estimating <br> Team Member | $904 / 627-3900$ |
| Dallas Gray | Ventry Engineering | Right of Way <br> Team Member | $904 / 627-3900$ |
| Ken Sperry, P.E. | KY Transportation <br> Cabinet | Highway Design | $502 / 564-3280$ |
| Steve Halloran, <br> P.E. | KY Transportation <br> Cabinct | Construction | $502 / 564-4780$ |
| Daryl Greer, P.E. | KY Transportation <br> Cabinet | Value Engineering | $502 / 564-3280$ |
| Jeff Jasper, E.I.T. | KY Transportation <br> Cabinet | Highway Design | $502 / 564-3280$ |

## PROJECT DESCRIPTION

The project provides for the relocation of 14.3 kilometers ( 8.9 miles) of U.S. 119 in Pike County, Kentucky. The proposed new alignment is approximately 12 kilometers ( 7.5 miles) in length. The project relocates existing U.S. 119 from Burning Fork Road to near Bent Mountain.

Four new mainline bridges cross Burning Fork Road, Racoon Creek, Johns Creek and existing U.S. 119. Winn Branch and Scott Fork are crossed with culverts with mainline access provided only to the southeast portion of Winn Branch. Overpasses are called for on the approaches at Raccoon Creek and Johns Creek to eliminate left turning vehicles across the median.

The project is functionally classified as a rural arterial in mountainous terrain.
The proposed typical section provides a 12 meter depressed median with two 7.2 meter roadways and 3.6 meter outside shoulders, with 3.0 meters paved.

The proposed project will displace approximately 116 families, 5 businesses and 213 graves.
This existing and proposed facility serves local traffic, major coal operations and is a major intrastate route.

## COST ESTIMATE

| Roadway Excavation | $\$ \mathbf{6 4 , 6 2 3 , 0 0 0}$ |
| :--- | ---: |
| Drainage | $3,129,000$ |
| Pavement and Base | $8,241,000$ |
| Bridges | $47,459,000$ |
| Compression Station | $\mathbf{6 , 5 0 0 , 0 0 0}$ |
| Miscellaneous |  |
| (Silt checks, Guardrail, End treatments, | $1,164,000$ |
| Staking, R/W Fence, Traffic, Water) |  |
|  |  |
| Mobilization | $3.0 \%$ |
| Demobilization | $1.5 \%$ |

Eng. \& Conting. $20 \% \quad 27,403,244$
(Approaches)
Burning Fork $\quad 9,202,000$
Racoon Branch $\quad \mathbf{8 , 4 9 1 , 0 0 0}$
Winn Branch
Johns Creek
3,113,000
Bent Mountain
Subtotal
7,853,000

Total Construction
\$198,225,464

Right of Way
Utility Relocation

| $\$ 32,379,000$ |
| ---: |
| $\mathbf{5 , 5 7 0 , 0 0 0}$ |

Total Project Estimate
$\$ 236.174 .464$

Figure 3
Typical Sections US 119 Corridor US 119
Pike Co., 1996

TYPICAL SECTION WITH 12 m DEPRESSED MEDIAN
PROJECT PLANNING REPORT
TYPICAL SECTIONS

us 119 zebuldon to bent mountain
TYPICAL DECK SECTION



## 

 LTIng6II S
GJSOdOzd
III. INVESTIGATION PHASE

## V.E. STUDY BRIEFING

January 7, 1997

| NAME | AFFILIATION | PHONE |
| :--- | :--- | :--- |
| Jack Trickey | Ventry Engineering | $904 / 627-3900$ |
| Don Keenan | Ventry Engineering | $904 / 627-3900$ |
| Steve Halloran | KTC Construction | $502 / 564-4780$ |
| Steve Hoefler | KTC Highway Design | $502 / 564-3280$ |
| Randy Stephens | Palmer Engineering | $606 / 744-1218$ |
| David Lindeman | Palmer Engineering | $606 / 744-1218$ |
| Charles Reichenbach | KY D.O.H. Dist. \#12 | $606 / 433-7791$ |
| Denton Biliter | Chief Dist. Eng., <br> KY D.O.H. Dist. \#12 | $606 / 433-7791$ |
| James D. Wright | Dist. Const. Eng., | $606 / 433-7791$ |
| KY D.O.H. Dist. \#12 |  |  |
| Keith R. Damron | Dist. \#12 Design Engineer | $606 / 433-7791$ |
| Robin R. Justice | Dist. \#12 Design EIT | $606 / 433-7791$ |
| Dallas Gray | Ventry Engineering | $904 / 627-3900$ |
| Ron Whiche! | Ventry Engineering | $904 / 627-3900$ |
| Jeff Jasper | KTC Highway Design | $502 / 564-3280$ |
| Ken Sperry | KTC Highway Design | $502 / 564-3280$ |
| Daryl Greer | KTC Value Engineer | $502 / 564-3280$ |
| Janet R. Coffey | KTC Dist. \#12 Operations | $502 / 564-4556$ |
| Dexter Newman | KTC Dist. \#12 Const. | $606 / 433-7791$ |
|  |  |  |

## PERSONS CONTACTED

| NAME | AFFILIATION | PHONE |
| :--- | :--- | :--- |
| Randy Stephens | Palmer Engineering Co. | $606 / 744-1218$ |
| Robert Miller | Tensar, Atlanta, Ga. | $800 / 292-4459$ |
| Jerry Justice | Dist. \#12, R/W | $606 / 433-7765$ |
| Joe Emberson | Tensar, Atlanta, Ga. | $800 / 292-4459$ |

## INVESTIGATION

The following have been identified by the Value Engineering Team as areas of focus and investigation for the Value Engineering process:

Areas identified as high cost items during the investigation phase:

## ITEM

EXCAVATION
DRAINAGE
PAV'T \& BASE
RIGHT OF WAY
STRUCTURES
APPROACHES

COST
\$64,600,000
$\$ 3,100,000$
\$8,200,000
$\$ 25,000,000$
$\$ 47,500,000$
$\$ 33,800,000$

FUNCTION
ESTABLISH PROFILE
CONVEY WATER
SUPPORT VEHICLES
PROVIDE LAND

SEPARATE TRAFFIC
PROVIDE ACCESS

FUNCTIONAL ANALYSIS WORKSHEET, INFORMATION PHASE
PROJECT: US 119/ZEBULON TO BENT MOUNTAIN
DATE: JANUARY 6-14, 1997

| ITEM | $\frac{\text { FUNCT. }}{\text { VERB }}$ | $\frac{\text { FUNCT. }}{\text { NOUN }}$ | TYPE | COST | WORTH | VALUE INDEX |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EXCAVATION | establish establish accom. facilitate facilitate | profile <br> align. <br> typical <br> access <br> develop. | $\begin{aligned} & \text { B } \\ & \text { B } \\ & \text { B } \\ & \text { B } \\ & \text { S } \end{aligned}$ | \$64,600,000 | \$60,000,000 | 1.1 |
| DRAINAGE | convey minimize | water erosion | $\begin{aligned} & \mathbf{B} \\ & \mathbf{B} \end{aligned}$ | \$3,100,000 | \$3,100,000 | 1.0 |
| PAV'T AND BASE | support support protect remove increase reduce | vehicles <br> loads <br> base <br> water <br> traction <br> rutting | $\begin{aligned} & \text { B } \\ & \text { B } \\ & \text { B } \\ & \text { B } \\ & \text { B } \\ & \text { B } \end{aligned}$ | \$8,200,000 | \$8,200,000 | 1.0 |
| RIGHT OF WAY | provide accom. | land design | $\begin{aligned} & \text { B } \\ & \text { B } \end{aligned}$ | \$25,000,000 | \$23,000,000 | 1.1 |
| STRUCTURES | span separate convey | creek traffic water | $\begin{aligned} & \text { B } \\ & \text { B } \\ & \text { B } \end{aligned}$ | \$47,500,000 | \$44,000,000 | 1.1 |
| BURNING FORK APPROACH | provide <br> eliminate | access left turns | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~S} \end{aligned}$ | \$9,200,000 | \$8,200,000 | 1.1 |
| RACCOON CREEK APPROACH | provide eliminate separate | access <br> left turns traffic | $\begin{aligned} & \text { B } \\ & \mathrm{S} \\ & \mathrm{~S} \end{aligned}$ | \$8,500,000 | \$8,000,000 | 1.1 |
| WINN BRANCH APPROACH | provide | access | S | \$3,100,000 | \$3,100,000 | 1.0 |
| JOHNS CREEK APPROACH | provide eliminate separate | access <br> left turns traffic | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~S} \\ & \mathrm{~S} \\ & \hline \end{aligned}$ | \$7,900,000 | \$7,000.000 | 1.3 |
| BENT MOUNTAIN APPROACH | provide eliminate | access left turns | $\begin{aligned} & \mathbf{B} \\ & \mathbf{S} \end{aligned}$ | \$5,100,000 | \$5,100,000 | 1.0 |

## IV. SPECULATION PHASE

## SPECULATION

Ideas generated, utilizing the brainstorming method, for performing the functions of previously identified areas of focus.

## EXCAVATION

- Revise the median width from a 12 m depressed median to a 4.2 m paved median with barrier wall throughout except at the Winn Branch Approach
- Revise the alignment between station $505+800$ and station $507+300$ to turn easterly along the hollow then back to the proposed alignment
- Increase the grade between station 508+600 and station $509+750$
- Bifureate the-foadways-in-fill-areas

PAVEMENT AND BASE

- Use-conerete-parement instead-of-asphalt-pavement for the US-119-mainline roodways
- Construet the outside-laneto-be-4.3m(14') wide and-strip the edge-line at 3.6m(12') to provide- on-additional- 0.6 m (2') of full-depth roadway for-edge-of-pavement suppert
- Construet a full depthshoulder to reduee future maintenanee eost eaused by heary trueks


## STRUCTURES

- Revise the bridge typical section to only provide a 3.0 m outside shoulder instead of the 3.6 m shoulder proposed


## US 119 AT BURNING FORK APPROACH

- Eliminate Ramp D from station $40+000$ to station $40+535$ and utilize Ramp E with a 15 m radius turnlane to provide the WB to NB movement
- Revise the north side of the intersection to reflect a half diamond type interchange with the long radius currently proposed for the SB to WB movement for the heavy trucks and retaining the relocated US 119 configuration currently proposed for the south side of the intersection


## RACCOON CREEK APPROACH

- Reduce the number of graves to be removed by using a combination of slope reinforcement and retaining walls on Ramp $A$ and left of mainline station $502+900$


## WINN BRANCH APPROACH

- Construct a wagon box to maintain access to Winn Branch Drive and eliminate the proposed approach on the east side of the new mainline US 119


## JOHNS CREEK APPROACH

- Use the mainline structures to provide for the separation of traffic between KY 194 and relocated US 119 and eliminate the proposed overpass on the new approach


## V. EVALUATION PHASE

## V.(a) ALTERNATIVES

## ALTERNATIVES

The following alternatives were formulated during the "eliminate and combine" portion of the Evaluation Phase.

## A. EXCAVATION

Value Engineering Alternative No. 1-Reduce the median width from a 12 m depressed median to a 4.2 m paved median with barrier wall throughout except at the Winn Branch Approach

Value Engineering Alternative No. 2-Revise the alignment between station $505+800$ and station $507+300$ to turn easterly along the hollow then back to the proposed alignment

Value Engineering Alternative No. 3-Flatten the side slopes in long fill sections (stations $504+800$ to $506+200$ and $508+500$ to $509+100$
B. STRUCTURES

Value Engineering Alternative No. 1-Reduce the shoulder width of the bridge typical section to 3.0 m instead of the proposed 3.6 m
C. US 119 AT BURNING FORK APPROACH

Value Engineering Alternative No. 1-Revise the north side of the intersection eliminating Ramp D to reflect a half diamond type interchange with the same long radius currently proposed for the SB to WB movement for the heavy trucks and also retaining the relocated US 119 configuration currently proposed for the south side of the intersection

## D. RACCOON CREEK APPROACH

Value Engineering Alternative No. 1-Reduce the number of graves to be removed by using a combination of slope reinforcement and retaining walls on Ramp A and left of mainline station $502+900$

## E. WINN BRANCH APPROACH

Value Engineering Alternative No. 1-Construct a wagon box to maintain access to Winn Branch Drive and eliminate the proposed approach on the east of the new mainline US 119

## F. JOHNS CREEK APPROACH

Value Engineering Alternative No. 1-Use the mainline structures to provide for the separation of traffic between KY 194 and relocated US 119 and eliminate the proposed overpass on the new approach

## V.(b) ADVANTAGES AND DISADVANTAGES

## EVALUATION

The following Advantages and Disadvantages were developed for the Value Engineering Alternatives previously generated during the speculation phase. It also includes the Advantages and Disadvantages for the As Proposed.

## A. EXCAVATION

As Proposed Typical Section (12m depressed median)

## Advantages

- provides area for snow removal
- simplifies drainage
- reduces runoff on fill sections due to the crowned roadway section
- provides area wide enough to provide a refuge for smaller turning vehicles
- allows for provision of deceleration and acceleration lanes along the median
- a larger volume of excavated material would be utilized in fill sections
- eliminates all obstacles ( barrier wall, etc.) from the median
- does not require milling of curb lips and barrier wall in future resurfacing operations


## Disadvantages

- increases the amount of excavation required in cut areas
- increases the R/W requirements
- increases the cost of maintenance


## Conclusion

Carry Forward for Further Development

Value Engineering Alternative No. 1 - Revise the median width from a 12 m depressed median to a 4.2 m paved median with barrier wall throughout except at the Winn Branch Approach.

## Advantages

- reduces the amount of excavation required
- reduces the $\mathrm{R} / \mathrm{W}$ requirements
- reduces the amount of maintenance required to maintain the median
- still provides enough area for a left turn storage lane
- reduces the potential for head-on collisions due to the addition of the median barrier wall
- would reduce the bridge deck width by 1' $31 / 2^{\prime \prime}$


## Disadvantages

- does not provide enough width for refuge of smaller turning vehicles across the median
- barrier wall is considered a obstacle to vehicles
- eliminates some of the area that could be used for storage of snow
- complicates the drainage of the project due to the addition of median drainage boxes and loss of storage area
- requires the draining of pavement runoff across 2 lanes of traffic

Conclusion
Carry Forward for Further Evaluation
Value Engineering Alternative No. 2-Revise the alignment between station $505+800$ and station $507+300$ to turn easterly along the hollow then back to the proposed alignment

## Advantages

- will reduce the amount of excavation required
- may reduce the amount of $\mathrm{R} / \mathrm{W}$ required
- may avoid the gas well at station $506+940$
- allows a flatter profile grade along the mainline


## Disadvantages

- will slightly increase the length of the roadway
- adds additional curves (2) to the alignment
- increases the potential impacts to the designated mine area
- eliminates a potential waste site


## Conclusion

Carry Forward for Further Evaluation
Value Engineering Alternative No. 3-Flatten the side slopes in long fill sections (stations $504+800$ to $506+200$ and $508+500$ to $509+100$ )

## Advantages

- reduces the amount of excavated waste
- reduces the area needed for waste disposal
- more conducive to future development
- reduces the amount of guardrail required
- reduces the potential for fill slides


## Disadvantages

- may increase the demand for additional access to the mainline roadway
- will increase the amount of drainage structures required
- will increase the amount of backslope that will have to maintained (mowing, etc. )

Conclusion
Carry Forward for Further Evaluation

## B. STRUCTURES

Value Engineering Alternative No. 1 -Reduce the shoulder width of the bridge typical section to 3.0 m instead of the proposed 3.6 m

## Advantages

- reduces the width of the bridges by 0.6 m ( $2^{\prime}$ ) each
- discourages the use of the outside shoulder as a traffic lane
- conforms to Kentucky Bridge Standards


## Disadvantages

NONE

## Conclusion

Carry Forward for Further Evaluation

## C. US 119 AT BURNING FORK APPROACH

As Proposed Approach

## Advantages

- provides high operating speeds on Ramps C and D


## Disadvantages

- requires a larger amount of excavation to construct
- requires a larger amount of pavement to construct
- requires additional R/W to construct
- requires an increased amount of drainage to construct
- design is more complex normally required for this type intersection


## Conclusion

Carry Forward for Further Evaluation

Value Engineering Alternative No. 1-Revise the north side of the intersection eliminating Ramp $D$ to reflect a half diamond type interchange with the same long radius currently proposed for the SB to WB movement for the heavy trucks and also retaining the relocated US 119 configuration currently proposed for the south side of the intersection

## Advantages

- reduces the amount of excavation required to construct
- reduces the amount of pavement required to construct
- reduces the amount of $\mathrm{R} / \mathrm{W}$ required to construct
- reduces the amount of drainage required to construct
- design is similar to that normally used for a tight diamond intersection


## Disadvantages

- will require a longer acceleration lane
- will reduce the operating speed of the interchange when compared to the As Proposed design


## Conclusion

Carry Forward for Further Evaluation

## D. RACCOON CREEK APPROACH

Value Engineering Alternative No. 1 -Reduce the number of graves to be removed by using a combination of slope reinforcement and retaining walls on Ramp $A$ and left of mainline station $502+900$

## Advantages

- reduces the cost of grave relocation
- may help with public relations by reducing the social impacts of this project
- reduces the potential for project delay due to difficulties with grave relocation


## Disadvantages

- increase the amount of waste material that will have to be disposed of elsewhere
- adds an additional cost for slope reinforcement and retaining wall


## Conclusion

Carry Forward for Further Evaluation

## E. WINN BRANCH APPROACH

Value Engineering Alternative No. 1 - Construct a wagon box to maintain access to Winn Branch Drive and eliminate the proposed approach on the east of the new mainline US 119

## Advantages

- eliminates the only proposed at-grade crossing involving left turns in this project
- may decrease the amount of waste material
- retains the same access currently available to all the residents of Winn Branch Road


## Disadvantages

- does not provide direct access to mainline US 119
- may increase the cost of construction, including drainage


## Conclusion

Carry Forward for further Evaluation

## F. JOHNS CREEK APPROACH

Value Engineering Alternative No. 1 - Use the mainline structures to provide for the separation of traffic between KY 194 and relocated US 119 and eliminate the proposed overpass on the new approach

## Advantages

- eliminates the proposed overpass structure on the Johns Creek Approach
- may reduce the $R / W$ requirements


## Disadvantages

- may increase the length of the mainline structures

Conclusion
Drop From Further Consideration
VI. DEVELOPMENT PHASE

## VI.(a) EXCAVATION

## VI.(a)(1) AS PROPOSED

## MEDIAN WIDTH

## "AS PROPOSED" 12.0 m

The as proposed typical section incorporates four lanes at 3.6 m , two median shoulders of 1.2 m paved and .6 m unpaved, two exterior shoulders at 3.0 m paved and .6 m unpaved. The median is a 12 m depressed median. This section provides for drainage of both roadways and provides for snow storage, left turns and storage lanes.

agsodoud sy


## FILL SLOPES

## "AS PROPOSED"

The proposed alignment provides for a typical section with a maximum slope of 1:2 in fill sections. Guardrail are utilized in areas steeper than $1: 4$ slopes.


## ALIGNMENT

## "AS PROPOSED"

The alignment between station $505+913$ and station $507+225$ crosses near the top of the mountain. This requires high volume excavation and adds to the waste disposal on the project.

## VI.(a)(2) V.E. ALTERNATIVES

## MEDIAN WIDTH

## V.E. ALTERNATIVE NO. 1

The V.E. alternative typical section incorporates four lanes at 3.6 m , two exterior shoulders at 3.0 m paved and .6 m unpaved. The median is 4.2 m wide with a traffic barrier in the middle of the median. The barrier will be used throughout except where approach roads will have a left turn movement (Winn Branch only).

This typical provides for drainage, left turns and storage lanes.
7ヲSOdOぬd 9NIVヨヨNIONヨ ヨח7V＾


## COST COMPARISON

Revised Median Width ( 12 m vs. 4.2 m )

| DESCRIPTION | $\begin{aligned} & \text { UNIT } \\ & \text { COST } \end{aligned}$ | PROP'D QTY. | $\begin{aligned} & \text { PROP'D } \\ & \text { COST } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { QTY. } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { COST } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Pavement | \$110/m ${ }^{2}$ |  |  | 11,041 | \$1,214,510 |
| Median Barrier Type 300C | \$140/m ${ }^{3}$ |  |  | 10,616 | \$ 1,486,240 |
| Crash Cushions | \$20,000/ea |  |  | 4 ea. | \$ 80,000 |
| Conc. Median Barrier Box Inlet | \$ 9,800/ea |  |  | 43 ea. | \$ 421,400 |
| Excavation Section 1 | \$2.61/m ${ }^{3}$ | 4,934,235 | \$12,878,353 | 4,749,617 | \$12,396,500 |
| Section 2 | \$2.61/m ${ }^{3}$ | 8,194,128 | \$21,386,674 | 7,722,398 | \$20,155,458 |
| Section 3 | \$2.61/m ${ }^{3}$ | 6,950,220 | \$18,140,074 | 6,673,785 | \$17,418,579 |
| Section 4 | \$2.61/m ${ }^{3}$ | 4,468,968 | \$11,664,006 | 4,210,184 | \$10,988,580 |
| Subtotal |  | 24,547,551 | \$64,069,107 | 23,355,984 | \$64,161,267 |
| Bridge Conc. | \$3.50/CY |  |  | 148 C.Y. | \$ -51,800 |
| Bridge Rebars | \$ .55/LB |  |  | 35,900 LB | \$ -19,745 |
| TOTAL |  |  | \$64,069,107 |  | \$64,089,722 |

## FILL SLOPES

## V.E. ALTERNATIVE NO. 2

The V.E. team recommends a typical section to provide modified slopes that will allow for the utilization of additional excavated material from station $504+800$ to station 505 +260 , station $505+460$ to station $506+200$ and station $508+640$ to station $509+060$. This will reduce the amount of waste, the waste area required, and guardrail necessary.


Left Sa guadrack eliminated by waste usage or flattering slopes
=tat quaturer sos End

$$
\begin{aligned}
& 504+880 \\
& 505+220 \\
& 505+460>340 \mathrm{~m} \\
& 506+160 \\
& 508+640>300 \mathrm{~m} \\
& 509+020
\end{aligned}
$$

End

Right side guardrail einimatet
start flattened slope
End
=tent
End
Start

$$
\begin{align*}
& 504+800 \\
& 505+280 \\
& 505+460 \\
& 506+200 \\
& 508+640 \\
& 509+060
\end{align*}>420 \mathrm{~m}
$$

Total Saved

$$
\begin{array}{r}
3240 \mathrm{~m} \\
\approx 10,800 \mathrm{ft}
\end{array}
$$


cu．m

| $\begin{aligned} & \underline{\omega} \\ & \vdots \\ & \vdots \end{aligned}$ |  |  | $\begin{array}{ll} 8 \\ Q_{0} \\ \infty \\ \infty \\ \mathbb{N} \end{array}$ | 8 8 N N | $\begin{aligned} & \hline 8 \\ & 户 \\ & 0 \\ & \end{aligned}$ | O N N | $\begin{aligned} & O_{N} \\ & N \\ & N \\ & N \end{aligned}$ | 으N | O <br> N <br> N | $\begin{aligned} & \frac{8}{9} \\ & 8 \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \mathbf{8} \\ & \mathrm{g} \\ & \mathbf{N} \\ & \mathrm{~N} \end{aligned}$ | $\frac{Q}{\mathbf{C}}$ | $\begin{aligned} & \text { 品 } \\ & 0 \\ & 0 \\ & 寸 \\ & \hline \end{aligned}$ |  |  | 8 <br>  |  | $\begin{aligned} & 0 \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ |  |  | $\begin{aligned} & 0 \\ & 0 \\ & \hline 0 \end{aligned}$ | 8 <br> 0 <br> 0 <br> 0 <br> $\vdots$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| $\frac{\stackrel{\varangle}{\mathbf{~}}}{\frac{\mathbf{\alpha}}{\mathbf{\alpha}}}$ |  | $\begin{gathered} 9 \\ \% \\ \% \\ \% \end{gathered}$ | $\begin{aligned} & \infty \\ & \sim \\ & \sim \end{aligned}$ | $\frac{19}{T}$ | $\frac{\Psi}{\infty}$ | $\frac{\square}{2}$ | $\begin{aligned} & 0 \\ & \sim \\ & \sim \end{aligned}$ | $\begin{aligned} & 0 \\ & \underset{\sim}{0} \\ & \hline \end{aligned}$ | $\begin{aligned} & \substack{8 \\ N \\ \hline} \end{aligned}$ | 0 | $\stackrel{\oplus}{T}$ | $\begin{aligned} & \mathbf{N} \\ & \mathbf{0} \\ & \underset{\sim}{2} \end{aligned}$ | $\begin{aligned} & \boldsymbol{N} \\ & \mathbf{N} \\ & \underset{N}{2} \end{aligned}$ | $\stackrel{M}{N}$ | $\begin{aligned} & \infty \\ & \\ & \hline \end{aligned}$ | 옹 | \％ | $\underset{\sim}{\text { N}}$ | 品 | $\stackrel{\infty}{N}$ | 옹 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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|  | $0$ | $\infty$ | Iq |  | $\underset{N}{\sim}$ | $\left\|\begin{array}{c} n \\ \cdots \\ m \end{array}\right\|$ | $\left\lvert\, \begin{aligned} & n \\ & \infty \\ & 0 \end{aligned}\right.$ | $\begin{aligned} & \text { On } \\ & \text { N } \end{aligned}$ | $\infty$ | $\frac{N}{n}$ | $0$ | $\frac{\pi}{N}$ | $\bar{\infty}$ | $\mathbf{D}_{\mathbf{\circ}}^{\mathbf{8}}$ | $\frac{91}{5}$ | $\stackrel{\square}{\text { ¢ }}$ | $\frac{\square}{\square}$ |  | $\begin{aligned} & \mathbf{N} \\ & \mathbf{O} \\ & \mathbf{N} \end{aligned}$ | N： | $\begin{aligned} & \mathbf{N}_{1} \\ & \mathbf{N} \end{aligned}$ | 3 | $\begin{array}{\|c\|} \mathbf{N} \\ \mathbf{q} \\ \hline \end{array}$ | $\begin{aligned} & 8 \\ & 8 \\ & \hline \end{aligned}$ |  | $\pm$ |  |  |  |  |  |  |  |  |  |  |  |  |
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Flatten Fill Slopes

| DESCRIPTION | UNIT <br> COST | PROP'D <br> QTY. | PROP'D <br> COST | V.E. <br> QTY. | V.E. <br> COST |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Guardrail | $\$ 8.47 / \mathrm{ft}$. | $14,600 \mathrm{ft}$ | $\$ 123,662$ | 3800 ft | $\$ 32,186$ |

Possible Savings $\quad \$ 91,476$

## ALIGNMENT

## V.E. ALTERNATIVE NO. 3

The V.E. team recommends that the alignment be relocated through the saddle located southerly of the proposed alignment. This greatly reduces excavation and the volume of waste.

V.E. Alternative No. 3

Alignment Revision

| DESCRIPTION | UNIT COST | PROP'D QTY. | $\begin{aligned} & \text { PROP'D } \\ & \text { COST } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { QTY. } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { COST } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway Pavement | \$6.81 Lin.M | 1312 M | \$ 893,472 | 1376 M | \$ 937,056 |
| Excavation | \$2.61 Cu. M | 6,152,513 | \$16,058,000 | 4,015,143 | \$10,479,523 |
| Drainage | \$224 Lin.M | 1312 M | \$ 293,888 | 1376 M | \$ 308,224 |
| TOTAL |  |  | \$17,245,360 |  | \$11,724,803 |

Possible Savings $\quad \$ \mathbf{5 , 5 2 0 , 5 5 4}$

## VI.(b) STRUCTURES

## VI.(b)(1) AS PROPOSED

# BRIDGE TYPICAL <br> (EXTERIOR SHOULDER 3.6 VS. 3.0) 

## "AS PROPOSED"

The As Proposed Typical for the bridges incorporates 3.6 m exterior shoulders, two 3.6 m lanes, a 1.8 m interior shoulder and two barriers at .5 m each. This typical is for each bridge. The 3.6 m exterior shoulder could encourage people to use this as a travel lane. (See AASHTO Geometric Design Chap. IV, Pg. 338).
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## VI.(b)(2) V.E. ALTERNATIVES

V.E. ALTERNATIVE NO. 1

The V.E. Alternative incorporates 3.0 m exterior shoulders, two 3.6 m lanes, a 1.8 m interior shoulder and two barriers at .5 m each. This typical reduces each bridge by .6 m each. A 3.0 m shoulder is adequate for emergency use and is consistent with the typical section for the adjacent Bent Mountain project and with the Basic Geometric Design Standards (Exhibit 66-03-06).
$7 \forall S O d O y d$ ONIUヨヨNIONヨ ヨП7ヲ＾
13. Space is provided for bus stops.
14. Improved lateral placement of vehicles and space for occasional encroachment of vehicles is provided.

For further information on other uses of shoulders, refer to NCHRP 254 (3).

Urban highways generally have curbs along the outer lanes. A stalled vehicle during peak hours disturbs traffic flow in all lanes in that direction when the outer lane serves through traffic. Where on-street parking is permitted, the parking lane provides some of the same services listed above for shoulders. Parking lanes are discussed further in the section "On-street Parking."

## Width of Shoulders

Desirably, a vehicle stopped on the shoulder should clear the pavement edge by at least 0.3 m . preferably by 0.6 m . This preference has led to the adoption of 3.0 m as the normal shoulder width that should be provided along high-type facilities. In difficult terrain and on low-volume highways, shoulders of this width may not be feasible. A minimum shoulder width of 0.6 m should be considered for the lowest-type highway, and a 1.8 to 2.4 m width would be preferable. Heavily traveled and high-speed highwavs and those carrving large numbers of trucks should have usable shoulders at leass 3.0 m and preferably 3.6 m wide; however, widths greater than 3.0 m mav encourage unauthorized use as a travel lane. Where bicyclists are to be accommodated. a minimum shoulder width of 1.2 m should be utilized. Shoulder widths for specific classes of highways are enumerated as parts of the total cross sections discussed in following chapters.

Where roadside barriers, walls. or other vertical elements are used, it is desirable to have a graded shoulder wide enough that these vertical elements can be offset a minimum of 0.6 m from the outer edge of the usable shoulder. It may be necessary to provide a graded shouider wider than used elsewhere to provide lateral support for guardrail posts and/or clear space for lateral dynamic deflection required by the particular barrier in use. On low-volume roads, roadside barriers may be placed at the outer edge of the shoulder: however. a minimum of 1.2 m should be provided from the traveled way to the barrier.

Although it is desirable that a shoulder be wide enough for a vehicle to be drivencompletely off the traveled way, narrower shoulders are better than none at all. When a vehicle making an emergency stop can drive onto the shoulder to occupy only 0.3 to 1.2 m of a traveled way of adequate width, the remaining traveled way width can be used by passing vehicles. Partial shoulders are

multi-lane brides
OTHER THAN URBRN-MANOATOPY
URBAN-DESIRABLE


NOTE:
On frcervays exception is to be mode for major long-spen structures which warrant independent analyses for bridge-widm determination.

## STRUCTURES COST COMPARISON

V.E. Alternative No. 1

Revising Bridge Typical (Exterior Shoulder 3.6 vs. 3.0)

| DESCRIPTION | $\begin{array}{\|l\|} \hline \text { UNIT } \\ \text { COST } \\ \hline \end{array}$ | PROP'D QTY. | $\begin{aligned} & \text { PROP'D } \\ & \text { COST } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { QTY. } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { COST } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Burning Fork Conc. | \$3.50/CY |  |  | 93.2 CY | \$ 32,620 |
| Burning Fork Steel | \$ .55/LB |  |  | 22,600 LB | \$ 12,430 |
| Racoon Creek Conc. | \$3.50/CY |  |  | 160 CY | \$ 56,000 |
| Racoon Creek Steel | \$ .55/LB |  |  | 38,880 LB | \$ 21,380 |
| Johns Creek Conc. | \$3.50/CY |  |  | 154 CY | \$ 53,900 |
| Johns Creek Steel | \$ .55/LB |  |  | 37,400 LB | \$ 20,600 |
| Bent Mountain Conc. | \$3.50/CY |  |  | 44.6 CY | \$ 15,610 |
| Bent Mountain Steel | \$.55/LB |  |  | 10,800 LB | \$ 5,940 |
| TOTAL |  |  |  |  | \$218,480 |

VI.(c) U.S. 119 AT BURNING FORK APPROACH

## VI.(c)(1) AS PROPOSED

## BURNING FORK

## "AS PROPOSED"

The proposed plan provides an off ramp (ramp D) from West to North for old U.S. 119. There is also a West to West ramp (ramp E) for Burning Fork Road South.






## VI.(c)(2) V.E. ALTERNATIVES

## BURNING FORK

## V.E. ALTERNATIVE NO. I

The V.E. team recommends that Ramp D be eliminated from station $40+000$ to $40+535$. Ramp $E$ will be moved southerly toward the mainline and will intersect Ramp $C$ at a more westerly location and closely (southerly) toward the mainline. Access north and south to old U.S. 119 (Burning Fork Road) is provided for west bound traffic along Ramp E. This eliminates more than 400 M of ramp through a cut, reduces right of way requirements and waste.



## COST SAVINGS

V.E. Alternative No. 1

Burning Fork

| DESCRIPTION | UNIT <br> COST | PROP'D <br> QTY. | PROP'D <br> COST | V.E. <br> QTY. | V.E. <br> COST |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Right of Way | $\$ 30,985 /$ Ac. |  |  | (RED.) <br> 19.4 Ac | (SAVINGS) <br> $\$ 601,103$ |
|  | $\$ 110 / \mathrm{m}^{2}$ |  |  |  |  |
| Pavement | $\$ 2.61 / \mathrm{m}^{3}$ |  |  |  |  |
| Excavation |  |  |  |  |  |
|  |  |  |  |  |  |
|  |  |  |  |  |  |
| TOTAL |  |  |  |  |  |

Possible Savings $\$ 4,488,777$
VI.(d) RACCOON CREEK APPROACH
VI.(d)(1) AS PROPOSED

## GRAVE REMOVAL

## "AS PROPOSED"

The construction of fills for the proposed alignment and Ramp A at Raccoon Creek will force the relocation of three cemeteries. The cemetery right of station $502+500$ contains 56 graves. The two cemeteries left of station $502+900$ contain 4 and 68 graves respectively.


AS PROPOSED
VI.(d)(2) V.E. ALTERNATIVES

## GRAVE REMOVAL

## V.E. ALTERNATIVE NO. 1 - Use Retaining Walls \& Steepened Slopes

This alternative uses MSE walls and $1: 1.5$ slopes reinforced with geotextile to reduce the footprint of the fill and avoid grave relocation.


VALUE ENGINEERING ALTERNATIVE
WITH STEEPENED SLOPES AND RETAINING WALLS

$$
\begin{aligned}
& \square \\
& 5 \operatorname{sta} 5.52+700 \\
& \begin{aligned}
3240 \mathrm{~m}^{2} & =\text { Reinf Slope } \approx 35996 \mathrm{ft}^{2} \\
100 \mathrm{~m}^{2} & =\text { Wall } \approx 1111 \mathrm{t}^{3}
\end{aligned}
\end{aligned}
$$

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VALUE ANALYSIS
ALTERNATIVE

250


COST COMPARISON
Cemetery Relocation vs. Ret. Walls \& Steepened Slopes ( $1: 1^{1 / 2}$ )

| DESCRIPTION | UNIT COST | PROP'D <br> QTY. | $\begin{aligned} & \text { PROP'D } \\ & \text { COST } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { QTY. } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { COST } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Grave Relocation | \$3,000/Grave | 128 | \$384,000 |  |  |
| Reinforced Steepened Slope | \$375/m ${ }^{2}$ |  |  | $2600 \mathrm{~m}^{2}$ | \$ 975,000 |
| Reinforced Steepened Slope | \$375/m ${ }^{2}$ |  |  | $3240 \mathrm{~m}^{2}$ | \$1,218,000 |
| MSE Retaining Walls | \$430/m ${ }^{2}$ |  |  | $420 \mathrm{~m}^{2}$ | \$ 180,600 |
| MSE Retaining Walls | \$430/m ${ }^{2}$ |  |  | $100 \mathrm{~m}^{2}$ | \$ 43,000 |
| TOTAL |  |  | \$384,000 |  | \$2,413,600 |
| Conversion Factor $10.76 \mathrm{SF}=\mathrm{Im}^{2}$ |  |  |  |  |  |

Possible Additional Cost \$2,029,600

## GRAVE REMOVAL

## V.E. ALTERNATIVE NO. 2 - Steepened Slopes

This alternative uses $1: 1$ slopes reinforced with geotextile to reduce the footprint of the fill and avoid grave relocation.



VALUE ENGINEERING ALTERNATIVE
WITH STEEPENED SLOPES

## COST COMPARISON

Cemetery Relocation vs. Steepened Slopes (1:1)

| DESCRIPTION | UNIT COST | PROP'D <br> QTY. | PROP'D <br> COST | V.E. <br> QTY. | V.E. <br> COST |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Grave Relocation | $\$ 3,000 /$ Grave | 128 | $\$ 384,000$ |  |  |
|  |  |  |  |  |  |
| Reinforced Steepened Slope | $\$ 460 / \mathrm{m}^{2}$ |  |  | $302 \mathrm{~m}^{2}$ | $\$ 1,389,200$ |
| Reinforced Steepened Slope | $\$ 460 / \mathrm{m}^{2}$ |  |  |  |  |
|  |  |  |  |  |  |
| TOTAL |  |  | $\$ 384,000$ |  | $\$ 1,536,400$ |

Possible Additional Cost \$ 2,541,600

## VI.(c) WINN BRANCH APPROACH

## VI.(e)(1) AS PROPOSED

## WINN BRANCH

(505 + 300)

## "AS PROPOSED"

The mainline alignment intersects Winn Branch Road at Mainline Station $505+300( \pm)$. The as proposed solution is to cut off Winn Branch road on both side of the embankment. Those residences north of the mainline maintain their existing access to the North to old U.S. 119. They would have no direct access to neighbors south of the mainline. Residents south of the mainline would have access to the mainline only by way of a new access road that would intersect the mainline at station $505+575$. The mainline intersection would be at grade and would allow south to west turns across the median. This would be contrary to a project design criteria that stated there should be no median crossings. This intersection would be the only exception on the entire project.


## VI.(e)(2) V.E. ALTERNATIVES

## WINN BRANCH

## V.E. ALTERNATIVE NO. 1

The V.E. alternative provides no access to the mainline but does maintain existing access to old U.S. 119 for all residences on Winn Branch Road. The V.E. alternative eliminates the proposed access road and the at grade intersection on the mainline and utilizes a 8.5 m x 4.8 m Wagon Box through the embankment at station $505+300$. The primary advantages her would be:

1. Maintaining neighborhood integrity.
2. Equal access for all Winn Branch Rd. residences.
3. Elimination of at grade intersection and resultant median crossing.

The primary disadvantages would be:

1. Increased cost.

As proposed $\quad=\$ 2.898(2.124+* 775 \mathrm{R} / \mathrm{W})$
V.E. Alternative $=\$ 4.714$
2. No direct access to new facility.

* $\mathrm{R} / \mathrm{W}=25$ Acres @ $\$ 30,985 /$ Acre


COST COMPARISON

Winn Branch

| DESCRIPTION | $\begin{aligned} & \text { UNIT } \\ & \text { COST } \end{aligned}$ | PROP'D QTY. | $\begin{aligned} & \text { PROP'D } \\ & \text { COST } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { QTY. } \end{aligned}$ | $\begin{aligned} & \text { V.E. } \\ & \text { COST } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Access Road |  |  | \$2,123,375 | 0 | 0 |
| Wagon Box |  | 0 | 0 |  | \$4,714,000 |
| Right of Way | 30,985 Ac | 25 | \$ 774,625 | 0 | 0 |
| TOTAL |  |  | \$2,898,000 |  | \$4,714,000 |

Possible Additional Cost \$ 1,816,000

## VII. SUMMARY OF RECOMMENDATIONS

## SUMMARY OF RECOMMENDATIONS

It is the recommendation of the Value Engineering Team that the following Value Engineering Alternatives be carried into the Project Development process for further development.

## EXCAVATION

Recommendation No. 1

The Value Engineering Team recommends that Value Engineering Alternative No. 2 be implemented. This alternative is to flatten the fill slopes in areas with long fills to a 1:6 slope, reducing the amount of waste material.

If this recommendation can be implemented, there is a potential savings of $\$ 91,476$.
Recommendation No. 2
The Value Engineering Team recommends that Value Engineering Alternative No. 3 be implemented. This alternative is to revise the alignment between stations $505+800$ and $507+300$.

If this recommendation can be implemented, there is a potential savings of \$5,520,554.

## STRUCTURES

Recommendation No. 3

The Value Engineering Team recommends that Value Engineering Alternative No. 1 be implemented. This alternative is to reduce the bridge shoulder widths to 3.0 m ( 10 feet).

If this recommendation can be implemented, there is a potential savings of $\mathbf{\$ 2 1 8 , 4 8 0}$.

## US 119 AT BURNING FORK

Recommendation No. 4
The Value Engineering Team recommends that Value Engineering Alternative No. 1 be implemented. This alternative is to revise the design of the US 119 at Burning Fork Road interchange.

If this recommendation can be implemented, there is a potential savings of $\$ 4,488,777$.

## WINN BRANCH APPROACH

## Recommendation No. 5

The Value Engineering Team recommends that Value Engineering Alternative No. 1 be implemented. This alternative is to eliminate the proposed at-grade intersection and construct a wagon box along Winn Branch Road.

If this recommendation can be implemented, there is an additional cost of $\$ 1,816,000$.

If all these recommendations are implemented, there is a potential total savings of approximately $\$ 8,503,287$.

| NAME | AFFILIATION | PHONE |
| :--- | :--- | :--- |
| Jack Trickey | Ventry Engineering | $904 / 627-3900$ |
| Ron Whichel | Ventry Engineering | $904 / 627-3900$ |
| Dallas Gray | Ventry Engineering | $904 / 627-3900$ |
| Daryl Greer | KTC Co. Hwy. Design | $502 / 564-3280$ |
| Ken Sperry | KTC Co. Design | $502 / 564-3280$ |
| Don Keenan | Ventry Engineering | $904 / 627-3900$ |
| David Lindeman | Palmer Engineering | $606 / 744-1218$ |
| Randy Stephens | Palmer Engineering | $606 / 744-1218$ |
| John Sacksteder | KTC - Design | $502 / 564-3280$ |
| Bill Hornbeck | KTC - Bridge Design | $502 / 564-4560$ |
| Joette Fields | KTC - Design | $502-564-3280$ |
| Charles Briggs | Div. Operations | $506 / 564-4556$ |
| Keith R. Damron | Dist. Design Engineer | $606 / 433-7791$ |

## VIII. APPENDICES



PIKE COUNTY US 199

GRAND TOTAL
-

| CONSTAUCTION COST ESTIMATEALTEANATE BCONSTRUCTION SECTION 2STA. $503+480-306+320$LENGTH $=(2840 \mathrm{~m})(2.840 \mathrm{~km})-(9317 \mathrm{~m})(1.76 \mathrm{mi})$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | UNIT |  |
| DESCRIPTION | OUANTITY | UFIT | Cost | TOTAL |
| EXCAYATION | 7.739,105 | CU. METEA | 52.61 | \$20.199,064 |
| STTUATION SIZE CROSS ETAMINS | LS | LS | LS | 584,307 |
| MEOLAN CROSS GRAIMS | LS | LS | LS | \$101.049 |
| MEDIAN BOXS | 35. | EACH | 52,300.00 | \$80.500 |
| PEAFORAJED PIPE UNDERORAIN 4' | 37,289 | LIN. FT. | \$5,00 | \$186,340 |
| CHANNEL CHANGE |  | CU.YD. | 52.01 | 50 |
| CHANNEL LINING CLASS 4 | 3,550 | TON | \$3.58 | 312.736 |
| CLEAAING AND GAUEAINO |  | ACAES | 51,200.00 | 50 |
| SILT CHECKS | 20 | EA. | 540.47 | \$969 |
| GUARDRAIL | 3,200 | LIN.F.F. | 5.47 | 527.104 |
| END TREATMENTS | 14 | EA, | \$500,00 | \$7.000 |
| STAKINE | 1.76 | MILE | 545,000.00 | 379.200 |
| AW FENCE - | 11,434 | UIN.F. | \$3.06 | \$57,393 |
| MAINTAIN ANO CONTAOL TAAFFIC | 1 | LUMP | \$50,000.00 | 550,000 |
| WATER | 2,000 | MGAL | 32.40 | 54.880 |
| $4^{\circ}$ DGA | 18,871 | TON | \$12.00 | \$228,452 |
| 9: DRAINAGE BLANKET | 23,504 | TON | 521.00 | 5493,584 |
| $10^{\circ}$ BASE | 34,996 | TON | 525.37 | \$607, 240 |
| $1.5^{\text {a }}$ EITUMINOUS CONC. SUAFACE | 0,033 | TON | 524.83 | \$105,381 |
| FUUL DEPTH DESA | 14677 |  | \$11.36 | \$132.651 |
| EITUMINOUS MATENTAL POA TACK | 63 | TON | \$230.75 | \$15,017 |
| EMLLSIIFIED ASPHALT AS.2 | 40 | TON | \$291.19 | \$11,560 |
| GITUMINOU'S SEAL RGGREGATE | 331 | TON | \$25.97 | 58,506 |
| SEED AND PROTEECTİON |  | SO.YD. | \$0.18 | 50 |
| SUB TOTAL |  |  |  | \$22,831,713 |
| MOEILIZATION 3\% |  |  |  | S604.959 |
| OEMOPILIZATION 1.5\% |  |  |  | \$542. 476 |
| SUB TOTAL |  |  |  | \$23,859,140 |
| ENGINEER 8 CONTINO. $20 \%$ |  |  |  | 54,771,821 |
| BAINLINE TOTAL * |  |  |  | \$28,630,968 |
| - DOES NOT Include approachs | BAICGES |  |  |  |
|  |  |  |  |  |
| APPROACH | WINN BR | $\mathrm{NCH}$ |  |  |
| APPPROACH RT. STA. $503+575$ |  | \$2,12 | 4,121 | Z-7C4EI2 |
| APPROACM LI. STA. 505*575 |  | \$2,00 | 6,110 |  |
| TURN LANES (2) |  | 512 | , 966 | $12+76$ |
| BRIOGE |  | 510.9 | 7,775 | 10207-7 |
| CULVEAT |  | s981 | ,000 | 5044000 |
| GRAND TOTAL |  |  |  |  |





* Preferboder.


